PAVING, STORM DRAIN, WATER AND SANITARY SEWER CONSTRUCTION PLANS

THE RESERVES AT MAGNOLIA

1020 N. WILLOWWOOD STREET DENTON, TEXAS

OWNER

OPG MAGNOLIA PARTNER, LLC

254 N. SANTA FE AVE., SUITE A SALINAS, KS 67401 TEL: 78-212-0810 email: aengstrom@overlandpg.com CONTACT: APRIL ENGSTORM

DEVELOPER

OVERLAND PROPERTY GROUP

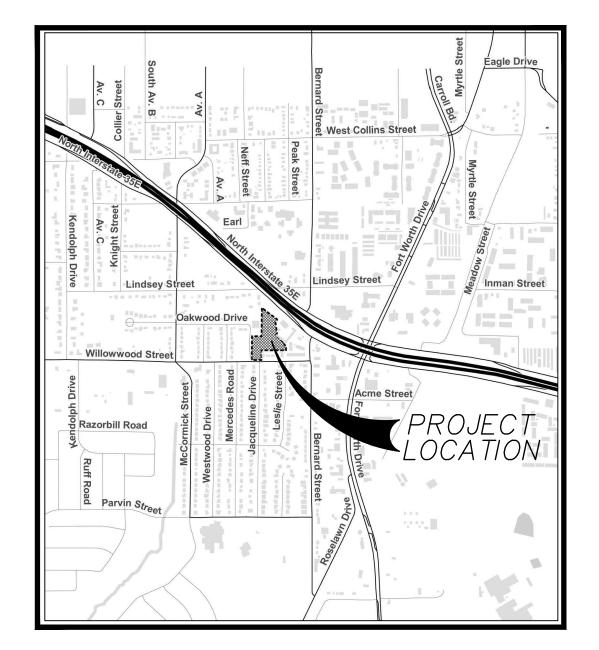
5341 W. 151st TERRACE LEAWOOD, KS 66224 TEL: 785-212-0810 email: aengstrom@overlandpg.com CONTACT: APRIL ENGSTROM

PREPARED BY:



tbpels registration number: f - 2759 tbpels registration/license number: 10088000 519 east border arlington, texas 76010 8 1 7 - 4 6 9 - 1 6 7 1 fax: 817-274-8757 www.mmatexas.com

CONTACT: ROB CRONIN, P.E.



VICINITY MAP NO SCALE

SOURCE BENCHMARK:

A: CITY OF DENTON MONUMENT #2011 -3.5" ALUMINUM CAP ON ROD "2011" LOCATED AT JUNCTION OF FRAME ROAD NORTH OF CONCRETE CREEK RUNNING PARALLEL WITH MCKINNEY APPROXIMATELY 37.7' FROM POWER POLE TO THE WEST ELEV: 611.24'

NAD83 TXNC (GRID) N: 7128266.50' E: 2389088.77'

BEARINGS & COORDINATES ARE GRID BASED ON THE "TEXAS COORDINATE SYSTEM OF 1983, NORTH CENTRAL ZONE" (2011), AS DETERMINED BY GPS OBSERVATIONS. THE CONVERGENCE ANGLE AT THIS POINT OF BEGINNING IS 0°44'22.0". ALL DISTANCES HAVE BEEN ADJUSTED TO SURFACE USING A COMBINED SCALE FACTOR OF 1.0001526521 (0.999847371200).

SITE BENCHMARK:

BM #1 - "X" CUT IN SQUARE LOCATED ON THE NORTHWEST CORNER OF AN INLET OFF THE EAST BOUND SIDE OF WILLOWWOOD STREET AND APPROXIMATELY 42' WEST FROM THE CENTERLINE OF JACQUELINE DRIVE

ELEV: 686.09' NAD83 TXNC (GRID) N: 7120793.43' E: 2383257.97'

BM #2 - "X" CUT IN SQUARE LOCATED APPROXIMATELY 272' EAST FROM THE CENTERLINE OF MERCEDES ROAD AT THE END OF OAKWOOD DRIVE ON THE SOUTH SIDE ON THE CURB APPROXIMATELY 50' WEST OF THE CHURCH

ELEV: 688.85' NAD83 TXNC (GRID) N: 7121272.19' E: 2383152.20'

DAT Approval

Development Services Department based upon recommendation from HR Green. All responsibility for the adequacy of these plans and for compliance with City of Denton specifications, standards, and codes remain with the engineer who prepared them. Review and approval of these plans by the City does not remove this responsibility. Deficiencies discovered prior to the final acceptance of public improvements will need to be corrected prior to Final Acceptance of the improvements.

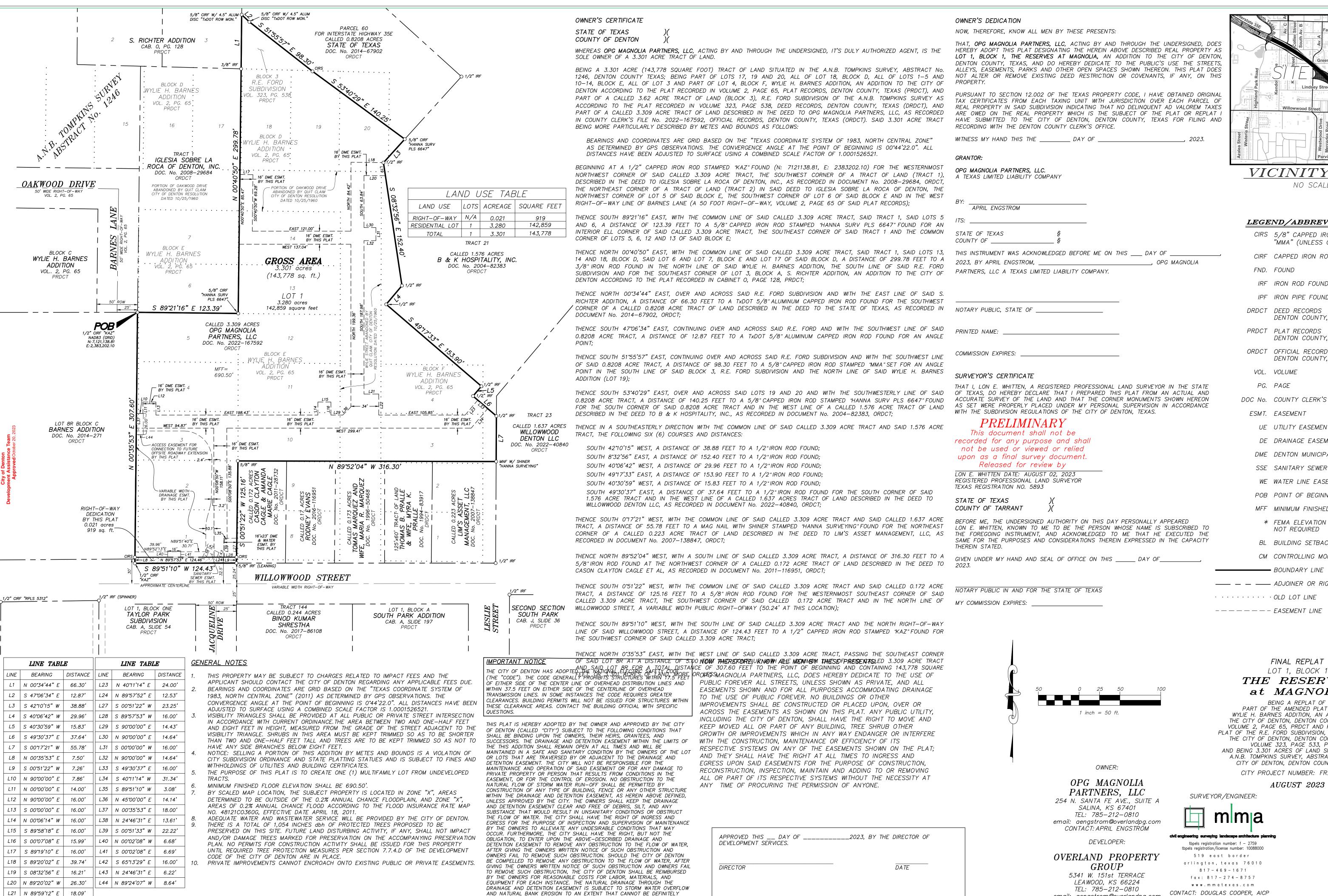
October 20, 2023

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C2.0	DEMOLITION PLAN
C3.0	SITE PLAN
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C4.0	DIMENSION CONTROL PLAN
C5.0	PAVING PLAN
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C12.8	PUBLIC STORM DRAINAGE DETAILS
C12.9	PUBLIC PAVING DETAILS
C12.10	PUBLIC PAVING DETAILS
C12.11	PUBLIC DUMPSTER PAD DETAILS
C12.12	PUBLIC EROSION CONTROL DETAILS

THE DETAIL SHEETS SPECIFICALLY IDENTIFIED ABOVE HAVE BEEN SELECTED BY ME UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.





DEFINED. THE CITY SHALL NOT BE HELD LIABLE FOR ANY DAMAGES OF ANY

THE EASEMENT OR OTHERWISE.

NATURE RESULTING FROM THE OCCURRENCE OF THESE NATURAL PHENOMENA

OR RESULTING FROM THE FAILURE OF ANY STRUCTURE OR STRUCTURES, WITHIN

CITY SECRETARY

OF THE CITY OF DENTON, TEXAS

L22 | S 00°00'00" E | 25.77'

VICINITY MAP

NO SCALE

LEGEND/ABBREVIATIONS

CIRS 5/8" CAPPED IRON ROD SET STAMPED "MMA" (UNLESS OTHERWISE NOTED)

CIRF CAPPED IRON ROD FOUND

IRF IRON ROD FOUND

IPF IRON PIPE FOUND

DRDCT DEED RECORDS DENTON COUNTY, TEXAS

PRDCT PLAT RECORDS

DENTON COUNTY, TEXAS

ORDCT OFFICIAL RECORDS DENTON COUNTY, TEXAS

DOC No. COUNTY CLERK'S DOCUMENT NUMBER

ESMT. EASEMENT

UE UTILITY EASEMENT

DE DRAINAGE EASEMENT

DME DENTON MUNICIPAL ELECTRIC

SSE SANITARY SEWER EASEMENT

WE WATER LINE EASEMENT

POB POINT OF BEGINNING

MFF MINIMUM FINISHED FLOOR ELEVATION

* FEMA ELEVATION CERTIFICATE NOT REQUIRED

BL BUILDING SETBACK LINE

CM CONTROLLING MONUMENT

BOUNDARY LINE

—— — — ADJOINER OR RIGHT-OF-WAY LINE

· · · · · · · · · · OLD LOT LINE

----EASEMENT LINE

FINAL REPLAT LOT 1, BLOCK 1 THE RESERVES at MAGNOLIA

PART OF THE AMENDED PLAT OF THE WYLIE H. BARNES ADDITION, AN ADDITION TO THE CITY OF DENTON, DENTON COUNTY, TEXAS VOLUME 2, PAGE 65, PRDCT AND PART OF THE PLAT OF THE R.E. FORD SUBDIVISION, AN ADDITION TO THE CITY OF DENTON, DENTON COUNTY, TEXAS VOLUME 323, PAGE 533, PRDCT AND BEING 3.301 ACRES OF LAND SITUATED IN THE A.N.B. TOMPKINS SURVEY, ABSTRACT NO. 1246 CITY OF DENTON, DENTON COUNTY, TEXAS

CITY PROJECT NUMBER: FR23-0012

CONTACT: DOUGLAS COOPER, AICP COPYRIGHT © 2023 MMA, INC. SHEET 1 OF 1

MMA PROJECT NO. 3597-00-02

DATE

email: aengstrom@overlandpg.com

CONTACT: APRIL ENGSTROM

- 1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE LATEST EDITION OF THE CITY OF **DENTON** STANDARD CONSTRUCTION SPECIFICATION DOCUMENTS.
- 2. THE LOCATIONS OF ALL EXISTING UTILITIES INDICATED ON THE PLANS ARE TAKEN FROM EXISTING PUBLIC RECORDS OR PROVIDED BY VARIOUS OWNERS OF THE FACILITIES. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFICATION OF VERTICAL AND HORIZONTAL LOCATIONS OF ALL UTILITIES PRIOR TO CONSTRUCTION. SUCH VERIFICATION SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS. NOTIFY THE PROJECT INSPECTOR PROMPTLY OF ALL CONFLICTS OF THE WORK WITH EXISTING UTILITIES.
- 3. THE CONTRACTOR SHALL NOTIFY ALL UTILITY COMPANIES WITH FACILITIES IN PROJECT AREA 48 HOURS PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL COORDINATE WITH THE PROPER UTILITY COMPANIES TO RELOCATE, BRACE, AND SUPPORT ANY UTILITY IN CONFLICT WITH THE PROPOSED STRUCTURE OR CONSTRUCTION. ALL ASSOCIATED WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS. ANY REPAIR TO THE DAMAGED EXISTING UTILITIES SHALL BE THE COST OF CONTRACTOR.
- 4. PUBLIC OR PRIVATE UTILITY LINES MAY EXIST THAT ARE NOT SHOWN ON THE CONSTRUCTION PLANS. THE CONTRACTOR IS RESPONSIBLE FOR PROTECTING THE INTEGRITY OF THESE LINES. SUCH WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS.
- 5. A TEXAS DEPARTMENT OF TRANSPORTATION (TXDOT) PERMIT WILL BE REQUIRED WHEN ENTERING/CROSSING RIGHT-OF-WAY OF TXDOT. CONTRACTOR IS RESPONSIBLE FOR NOTIFYING TXDOT 48 HOURS PRIOR TO CONSTRUCTION.
- 6. TRENCH SAFETY PLAN SHALL BE STAMPED, SIGNED, AND DATED BY A LICENSED PROFESSIONAL ENGINEER AND SUBMITTED TO THE PROJECT ENGINEER AND PUBLIC WORKS INSPECTOR BEFORE OR DURING PRE—CONSTRUCTION MEETING.
- 7. THREE COPIES OF "STORM WATER POLLUTION PREVENTION PLAN (SWPPP)" SEALED BY A LICENSED PROFESSIONAL ENGINEER SHALL BE SUBMITTED TO THE PROJECT ENGINEER AT LEAST 10 WORKING DAYS PRIOR TO CONSTRUCTION. A "NOTICE OF INTENT" (NOI) SHALL BE INCLUDED IN THE SUBMITTAL IF THE PROJECT IS 5 ACRES IN SIZE OR LARGER. THE CONTRACTOR IS RESPONSIBLE FOR SUBMITTING A NOTICE OF TERMINATION (NOT) WITHIN 30 DAYS OF THE PROJECT COMPLETION.
- 8. A TRAFFIC CONTROL PLAN SHALL BE SUBMITTED TO PUBLIC WORKS/TRANSPORTATION DEPARTMENT TRANSPORTATION DIVISION FOR APPROVAL FOR ANY UTILITY AND STREET WORK THAT WILL CAUSE LANE CLOSURES. AT LEAST ONE LANE TRAFFIC SHALL BE MAINTAINED AT ALL TIMES. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING AN APPROVED TRAFFIC CONTROL PLAN TO THE PROJECT INSPECTOR.
- 9. THE CONTRACTOR SHALL NOTIFY THE PROJECT INSPECTOR AT LEAST THREE (3) WORKING DAYS PRIOR TO CONSTRUCTION IN ORDER TO COORDINATE CONSTRUCTION, ADMINISTRATION, AND INSPECTION ACTIVITIES.
- 10. ALL WATER AND SANITARY SEWER WORK REQUIRED WITHIN THE PRIVATE PROPERTY SHALL BE PERFORMED BY A LICENSED PLUMBER. THE CONTRACTOR SHALL NOTIFY THE PROPERTY OWNER AND IMMEDIATELY COMMENCE SURFACE RESTORATION TO AN EQUAL OR BETTER CONDITION AFTER THE WATER AND SEWER WORK IS COMPLETE.
- 11. THE CONTRACTOR SHALL AVOID DAMAGING ANY EXISTING SPRINKLER SYSTEM THAT MAY BE IN THE CONSTRUCTION AREA. REMOVED OR DAMAGED SPRINKLER SYSTEM SHALL BE REPAIRED BY A LICENSED IRRIGATOR. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS.
- 12. THE CONTRACTOR SHALL AVOID DAMAGING ANY LANDSCAPING THAT MAY BE IN THE CONSTRUCTION AREA. REPLACING DAMAGED LANDSCAPING SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL.
- 13. REMOVAL OF TREES AND OTHER EXISTING STRUCTURES SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL OR APPROVED BY THE PROJECT INSPECTOR.
- 14. ALL TRENCHES SHALL BE BACKFILLED AT THE END OF EACH WORKING DAY. THE CONTRACTOR SHALL PROVIDE ALL FENCING AND/OR BARRICADES TO MAINTAIN PUBLIC SAFETY AROUND THE CONSTRUCTION AREA.
- 15. THE CONTRACTOR SHALL CLEAN UP DIRT AND DEBRIS IN THE PAVED AREAS TO ENSURE THE STREET IS IN DRIVABLE CONDITION AT THE END OF EACH WORKING
- 16. EXISTING MAIL BOXES SHALL BE PROTECTED AND MAIL SERVICE SHALL NOT BE INTERRUPTED.
- 17. ALL WATER AND SANITARY SEWER LINES USING DUCTILE IRON PIPE SHALL BE CLASS 350 UNLESS OTHERWISE SHOWN ON THE CONSTRUCTION PLANS OR BID ITEMS. ALL DUCTILE IRON PIPES SHALL BE WRAPPED BY POLYETHYLENE ENCASEMENT TO MEET ANSI/AWWA C105/A21.5 SPECIFICATIONS WITH A MINIMUM 8MIL THICKNESS.
- 18. ALL PIPE FITTINGS SHALL BE DUCTILE IRON AND RESTRAINED WITH BOTH CONCRETE BLOCKING AND MEGALUGS. ALL PIPE FITTINGS SHALL BE CONSIDERED SUBSIDIARY TO THE PIPE PRICES.
- 19. REMOVAL, SALVAGE, AND ABANDONMENT OF THE EXISTING WATER AND SEWER LINES SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS.
- 20. REMOVAL AND HAUL-OFF EXCESS CONSTRUCTION MATERIALS FROM THE CONSTRUCTION SITE SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS.
- 21. ALL EXISTING FENCES REMOVED OR DAMAGED BY CONSTRUCTION SHALL BE REPLACED OR RESTORED WITH SAME TYPE, STYLE, AND MATERIAL TO AN EQUAL OR BETTER CONDITION. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL.
- 22. ALL TEMPORARY FENCES SHALL BE INSTALLED TO PROTECT PRIVATE PROPERTIES WHEN THE CONSTRUCTION WORK IS PERFORMED WITHIN THE EASEMENTS. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL.

WATER NOTES:

- 1. ALL WATER MAINS USING PVC PIPE SHALL BE IN COMPLIANCE WITH AWWA C900 AND DR-14.
- 2. UTILITY SAND MEETING THE REQUIREMENTS OF CITY OF DENTON SPECIFICATIONS 33.05.05 SECTION 2.2.A.1 SHALL BE USED FOR ALL WATER MAINS.
- 3. THERE SHALL BE A MINIMUM COVER OF FORTY—TWO INCHES (42") OVER THE WATER PIPE AS MEASURED FROM THE TOP OF THE PIPE TO THE EXISTING GROUND, OR THE PROPOSED FINISHED GRADES, WHICHEVER IS GREATER.
- 4. FIRE HYDRANTS SHALL BE PLACED A MINIMUM DISTANCE OF AND AS CLOSE TO 2 FEET BEHIND BACK OF CURB AS POSSIBLE. ALTERNATE LOCATIONS MUST BE APPROVED BY CITY OF DENTON WATER DISTRIBUTION.
- 5. WHEN REPLACING WATER SERVICES, ALL EXISTING WATER METER BOXES SHALL BE REPLACED AND THE METERS SHALL BE RELOCATED TO THREE FEET (3') BACK OF THE CURB OR AS SHOWN ON THE CONSTRUCTION PLANS.
- 6. ALL VALVE BOXES SHALL BE ADJUSTED UP TO THE FINISHED PAVEMENT GRADE. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL.
- 7. NO WELDED OUTLET IS ALLOWED FOR THE DUCTILE IRON WATER LINE.

SANITARY SEWER NOTES:

- 1. ALL SANITARY SEWER MAINS USING PVC PIPE SHALL BE SDR-26, HEAVY WALL, IN COMPLIANCE WITH ASTM D-3034 LATEST REVISION.
- 2. CRUSHED ROCK MEETING THE REQUIREMENTS OF CITY OF DENTON SPECIFICATIONS 33.05.05 SECTION 2.2.A.2 SHALL BE USED FOR ALL SANITARY SEWER MAINS.
- 3. THE CONTRACTOR SHALL ENSURE ALL THE EXISTING SEWER SERVICES THAT CAN BE RECONNECTED OR REROUTED TO THE NEW MAINS. THE CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER ANY POTENTIAL CONFLICTS PRIOR TO CONSTRUCTION SO THE MODIFICATION CAN BE MADE IF NECESSARY.
- 4. THE CONTRACTOR SHALL CONDUCT A PRE—CONSTRUCTION TELEVISION INSPECTION OF ALL THE EXISTING SEWER LINES TO BE ABANDONED OR REHABILITATED BY OTHER THAN OPEN CUT TO VERIFY THE SERVICE LOCATIONS AND LINE CONDITIONS. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL.
- 5. MINIMUM LATERAL SIZE 6" IS REQUIRED FOR ANY DEVELOPMENT ABOVE SINGLE FAMILY RESIDENTIAL.
- 6. THE CONTRACTOR SHALL VERIFY THE EXISTING SANITARY SEWER SLOPE PRIOR TO MAKING CONNECTION. SLOPE OF THE NEW MAIN AND THE EXISTING MAIN SHALL MATCH UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN. A REDUCER SHALL BE USED WHEN A LARGER SIZE LINE CONNECTS TO A SMALLER SIZE LINE.
- 7. THE CONTRACTOR SHALL VERIFY THE SIZE, ELEVATION, AND MATERIAL OF THE EXISTING LINE PRIOR TO ORDERING PREFABRICATED MANHOLE.
- 8. REWORK THE INVERT WHERE TYING INTO AN EXISTING MANHOLE.
- 9. BYPASS PUMP REQUIRED FOR PERFORMING CONSTRUCTION WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL. A BYPASS PUMPING PLAN MUST BE SUBMITTED AND APPROVED BY THE CITY OF DENTON PRIOR TO ANY BYPASS PUMPING OPERATIONS MAY TAKE PLACE.
- 10. ALL MANHOLE AND CLEANOUT SHALL BE ADJUSTED UP TO THE FINISHED PAVEMENT GRADE. THIS WORK SHALL BE CONSIDERED SUBSIDIARY TO ALL OTHER BID ITEMS UNLESS OTHERWISE INDICATED ON THE CONSTRUCTION PLAN OR BID PROPOSAL.

THE RESERVES AT MADENTON, TEXAS

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GENERAL UTILITY NOTES:

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CALL TEXAS 811 OR OTHER
UTILITY LOCATING SERVICES 48
HOURS PRIOR TO
CONSTRUCTION ACTIVITY. MMA
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KNOWING ALL EXISTING UTILITIES
OR DEPICTING EXACT
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SOURCE BENCHMARK:

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3.5" ALUMINUM CAP ON ROD "2011" LOCATED AT JUNCTION
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A: 611.24'

NAD83 TXNC (GRID) N: 7128266.50' E: 2389088.77'

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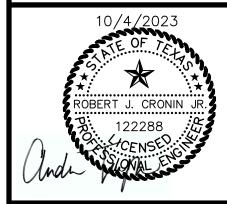
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BEARINGS & COORDINATE VALUES SHOWN HEREON ARE IN REFERENCE TO THE NAD83 — TEXAS COORDINATE SYSTEM — NORTH CENTRAL ZONE, 4202, BASED ON GPS OBSERVATIONS UTILIZING THE LEICA GPS REFERENCE NETWORK. ALL DISTANCES SHOWN HEREON WERE ADJUSTED TO SURFACE USING A COMBINED SCALE FACTOR OF 1.0001526521 (0.999847371200), BASE POINT OF 0,0,0.



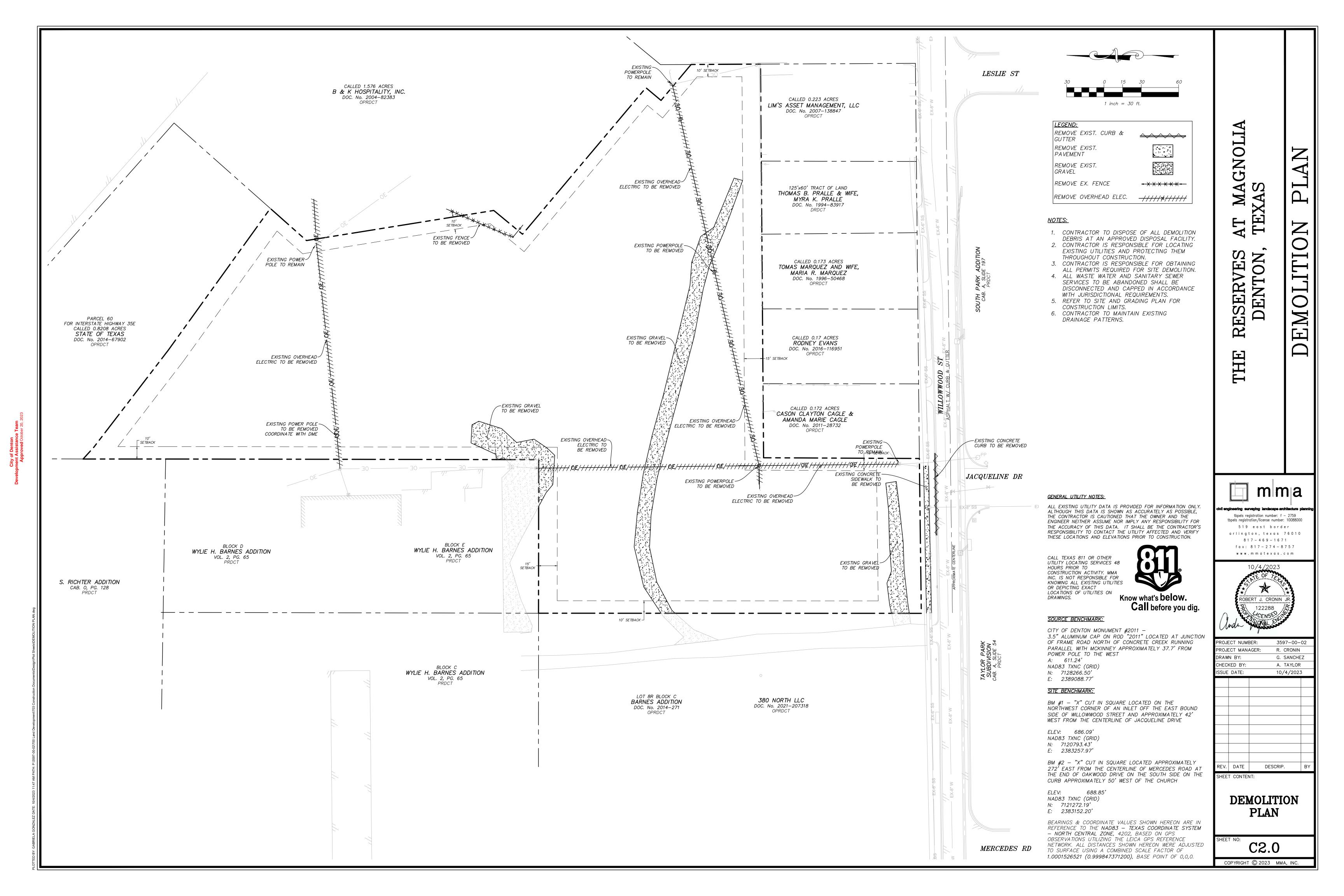
519 east border arlington, texas 76010 817-469-1671 fax: 817-274-8757 www.mmatexas.com

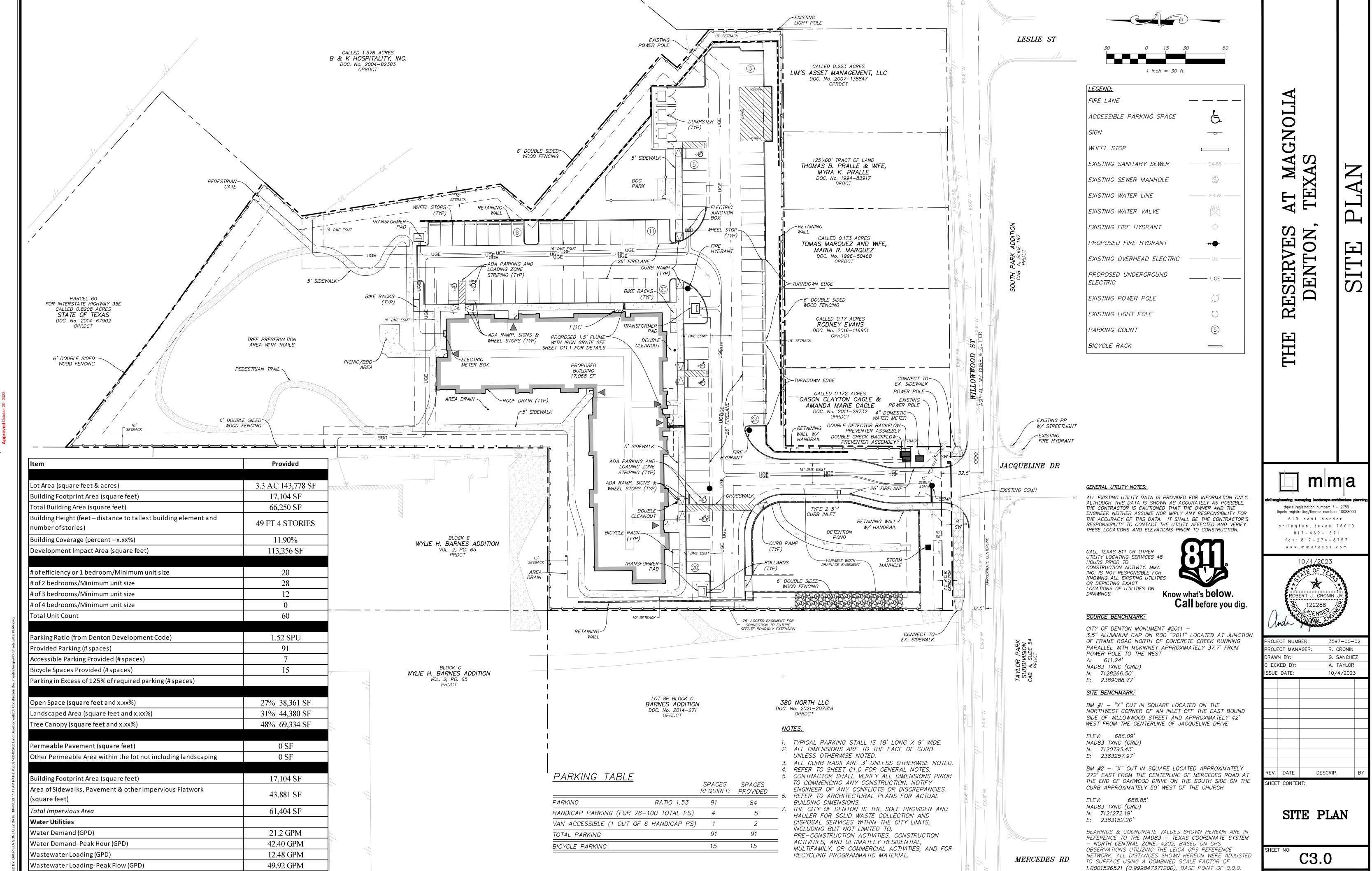


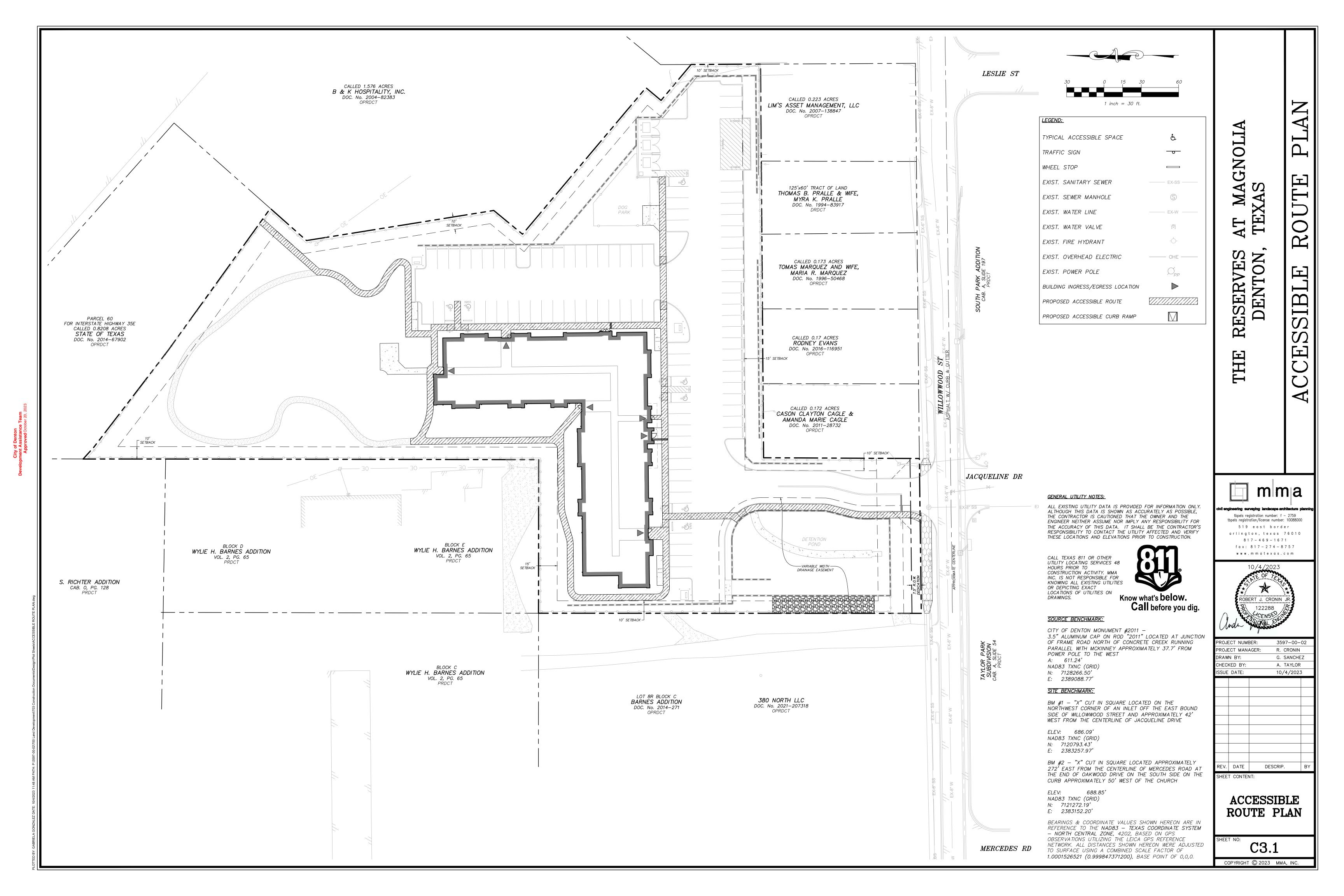
PROJE	ECT NUMI	BER: 3597-00-	02
PROJE	ECT MAN	AGER: R. CRONIN	
DRAW	N BY:	G. SANCH	ΞZ
CHEC	KED BY:	A. TAYLOR	
SSUE	DATE:	10/4/202	3
REV.	DATE	DESCRIP.	BY
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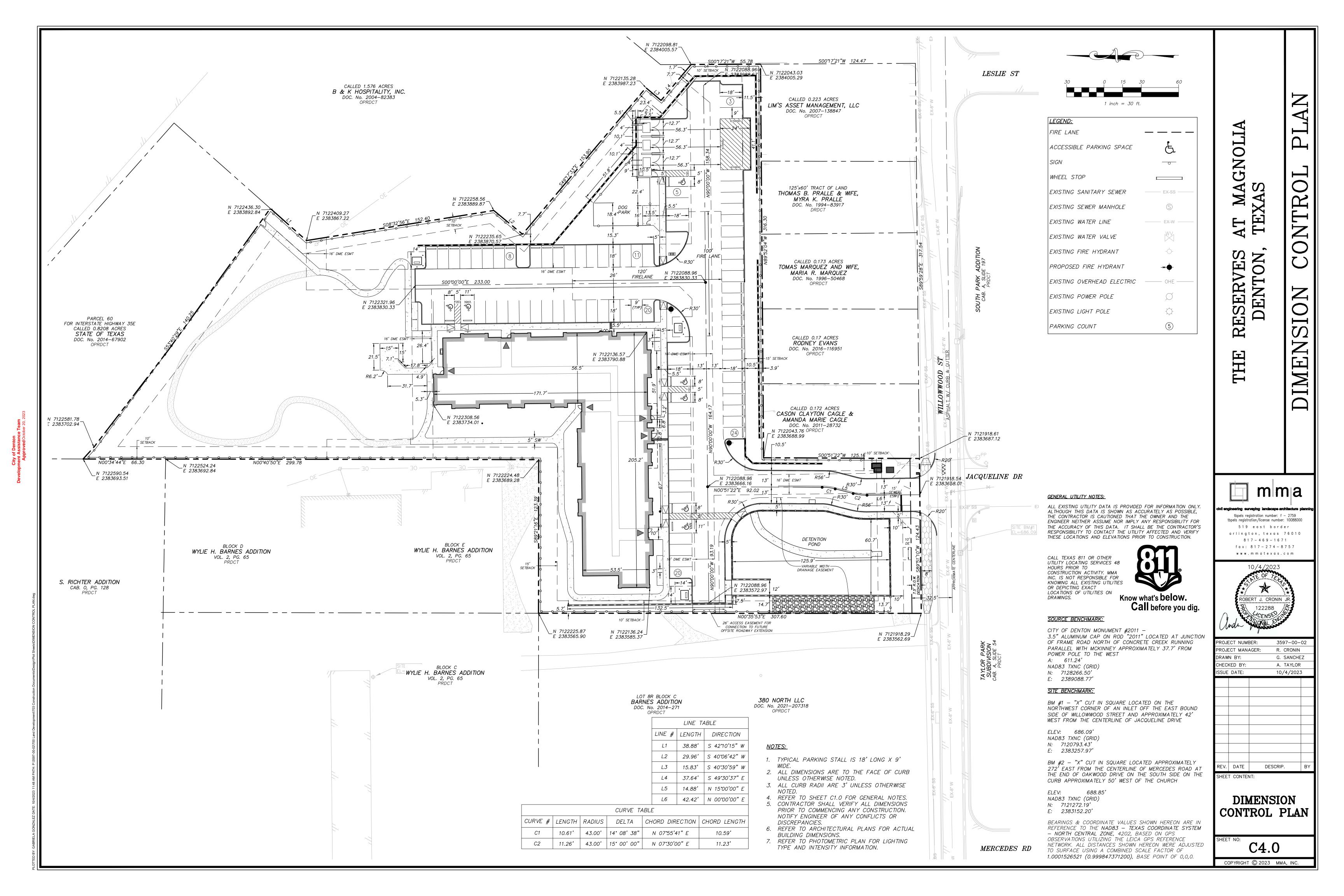
GENERAL NOTES

C1.0









NECESSARY.

tbpels registration number: f - 2759 tbpels registration/license number: 10088000 519 east border arlington, texas 76010

TO SURFACE USING A COMBINED SCALE FACTOR OF 1.0001526521 (0.999847371200), BASE POINT OF 0,0,0.



817-469-1671 fax: 817-274-8757



PROJE	ECT MAN	AGER: R. CRONIN	
DRAWN BY:		G. SANCHE	Z
CHEC	KED BY:	A. TAYLOR	
SSUE	DATE:	10/4/2023	5
REV.	DATE	DESCRIP.	BY
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PAVING PLAN

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1.0001526521 (0.999847371200), BASE POINT OF 0,0,0.

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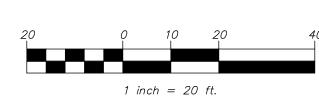
www.mmatexas.com * ROBERT J. CRONIN JE 122288

PROJE	ECT NUME	BER: 3597-00-	02
PROJECT MANA		AGER: R. CRONIN	
DRAW	N BY:	G. SANCHE	Z
CHEC	KED BY:	A. TAYLOR	
ISSUE	DATE:	10/4/2023	5
REV.	DATE	DESCRIP.	BY

OVERALL GRADING PLAN

SHEET CONTENT:





LEGEND:

EXISTING MAJOR CONTOUR — *-640-* — EXISTING MINOR CONTOUR *− −641− −* PROPOSED MINOR CONTOUR ----640---- PROPOSED MAJOR CONTOUR -----641-----• FG 630.00 FINISHED GRADE FINISHED FLOOR ELEVATION FF 630.0 PROPOSED TOP OF PAVEMENT • TP 630.00 PROPOSED TOP OF CURB PROPOSED GUTTER • GT 630.00 PROPOSED TOP OF WALL • TW 630.00 PROPOSED BOTTOM OF WALL • BW 630.00 PROPOSED TOP INLET • TI 630.00 MATCH EXISTING • NG 630.00± PROPOSED RETAINING WALL

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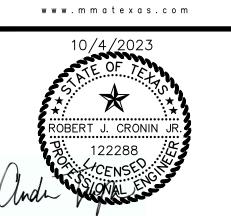
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THE RESERVES AT DENTON, TEX

mma

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SSUE	DATE:	10/4/202	3
REV.	DATE	DESCRIP.	BY

GRADING PLAN

SHEET CONTENT:

C6.1

T NO.

SECTION B-B

4. UNSUITABLE MATERIAL, INCLUDING ALL ROCKS MEASURING

LARGER THAN 6" IN THE LARGEST DIMENSION, REFUSE

AND OTHER DEBRIS SHALL BECOME THE PROPERTY OF

THE CONTRACTOR AND SHALL BE REMOVED FROM THE

DEBRIS WHILE TRANSPORTING TO A DISPOSAL SITE.

CONTRACTOR SHALL, ALSO, COMPLY WITH ALL APPLICABLE

SITE AND DISPOSED OF BY THE CONTRACTOR.

LAWS GOVERNING THE SPILLING OF

DENTO

mma

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KNOWING ALL EXISTING UTILITIES Know what's below. Call before you dig.

SOURCE BENCHMARK:

OR DEPICTING EXACT

CITY OF DENTON MONUMENT #2011 -3.5" ALUMINUM CAP ON ROD "2011" LOCATED AT JUNCTION OF FRAME ROAD NORTH OF CONCRETE CREEK RUNNING PARALLEL WITH MCKINNEY APPROXIMATELY 37.7' FROM POWER POLE TO THE WEST

ALL EXISTING UTILITY DATA IS PROVIDED FOR INFORMATION ONLY.

ENGINEER NEITHER ASSUME NOR IMPLY ANY RESPONSIBILITY FOR

THE ACCURACY OF THIS DATA. IT SHALL BE THE CONTRACTOR'S

RESPONSIBILITY TO CONTACT THE UTILITY AFFECTED AND VERIFY

THESE LOCATIONS AND ELEVATIONS PRIOR TO CONSTRUCTION.

ALTHOUGH THIS DATA IS SHOWN AS ACCURATELY AS POSSIBLE,

THE CONTRACTOR IS CAUTIONED THAT THE OWNER AND THE

1 inch = 20 ft.

— *-640-* —

− −641− −

----640----

-----641-----

• FG 630.00

FF 630.0

• TP 630.00

• TC 630.00

• GT 630.00

• TW 630.00

• BW 630.00

• TI 630.00

• NG 630.00±

LEGEND:

EXISTING MAJOR CONTOUR

EXISTING MINOR CONTOUR

PROPOSED MINOR CONTOUR

PROPOSED MAJOR CONTOUR

FINISHED FLOOR ELEVATION

PROPOSED TOP OF CURB

PROPOSED TOP OF WALL

PROPOSED TOP INLET

MATCH EXISTING

GENERAL UTILITY NOTES:

CALL TEXAS 811 OR OTHER

HOURS PRIOR TO

UTILITY LOCATING SERVICES 48

CONSTRUCTION ACTIVITY. MMA

INC. IS NOT RESPONSIBLE FOR

LOCATIONS OF UTILITIES ON

PROPOSED BOTTOM OF WALL

PROPOSED RETAINING WALL

| PROPOSED GUTTER

PROPOSED TOP OF PAVEMENT

FINISHED GRADE

A: 611.24' NAD83 TXNC (GRID) N: 7128266.50' E: 2389088.77'

SITE BENCHMARK:

BM #1 - "X" CUT IN SQUARE LOCATED ON THE NORTHWEST CORNER OF AN INLET OFF THE EAST BOUND SIDE OF WILLOWWOOD STREET AND APPROXIMATELY 42' WEST FROM THE CENTERLINE OF JACQUELINE DRIVE

ELEV: 686.09' NAD83 TXNC (GRID) N: 7120793.43'

E: 2383257.97'

BM #2 - "X" CUT IN SQUARE LOCATED APPROXIMATELY 272' EAST FROM THE CENTERLINE OF MERCEDES ROAD AT THE END OF OAKWOOD DRIVE ON THE SOUTH SIDE ON THE CURB APPROXIMATELY 50' WEST OF THE CHURCH

ELEV: 688.85° NAD83 TXNC (GRID)

N: 7121272.19' E: 2383152.20'

11. REFERENCE LANDSCAPE PLANS FOR TREE PROTECTION

12. CONTRACTOR IS RESPONSIBLE FOR ENSURING GRADES ON

ALL WALKS DO NOT EXCEED 2% CROSS SLOPE AND 5%

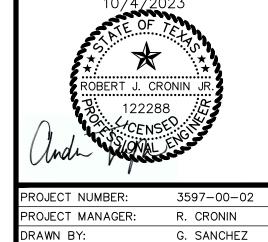
LONGITUDINAL SLOPE. ADJUST GRADES AS NECESSARY.

13. CONTRACTOR SHALL OBTAIN AND PAY FOR ALL PERMITS

METHODS.

REQUIRED.

BEARINGS & COORDINATE VALUES SHOWN HEREON ARE IN REFERENCE TO THE NAD83 - TEXAS COORDINATE SYSTEM - NORTH CENTRAL ZONE, 4202, BASED ON GPS OBSERVATIONS UTILIZING THE LEICA GPS REFERENCE NETWORK. ALL DISTANCES SHOWN HEREON WERE ADJUSTED TO SURFACE USING A COMBINED SCALE FACTOR OF 1.0001526521 (0.999847371200), BASE POINT OF 0,0,0.



CHECKED BY:		A. TAYLOR	
ISSUE	DATE:	10/4/2023	3
REV.	DATE	DESCRIP.	BY
SHEE	T CONTEN	NT:	

GRADING PLAN

DENTO

mma

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*

ROBERT J. CRONIN JE

122288

3597-00-02

G. SANCHEZ

A. TAYLOR

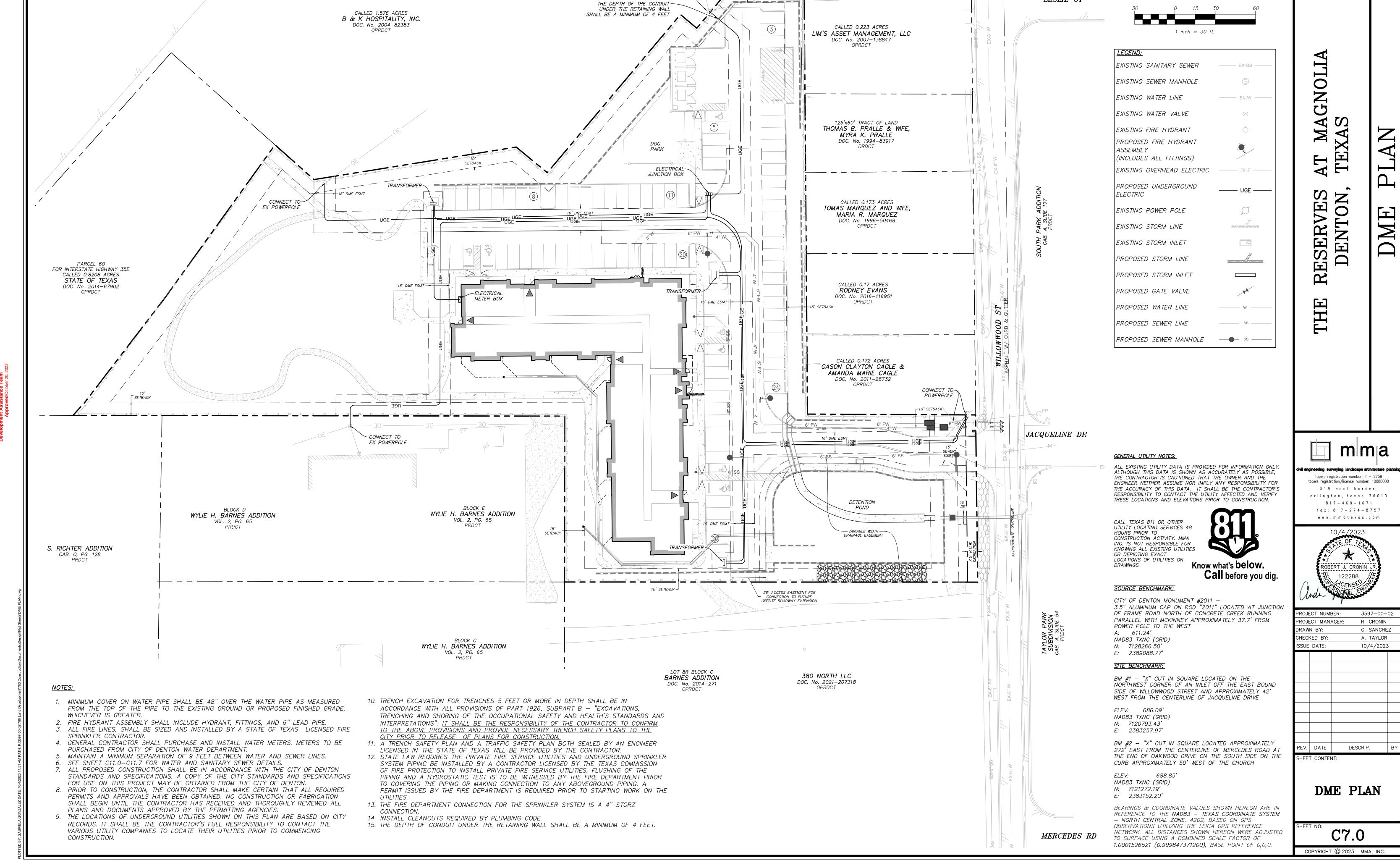
R. CRONIN

TO SURFACE USING A COMBINED SCALE FACTOR OF 1.0001526521 (0.999847371200), BASE POINT OF 0,0,0.

SUE DATE: 10/4/2023 REV. DATE DESCRIP.

GRADING PLAN

C6.3

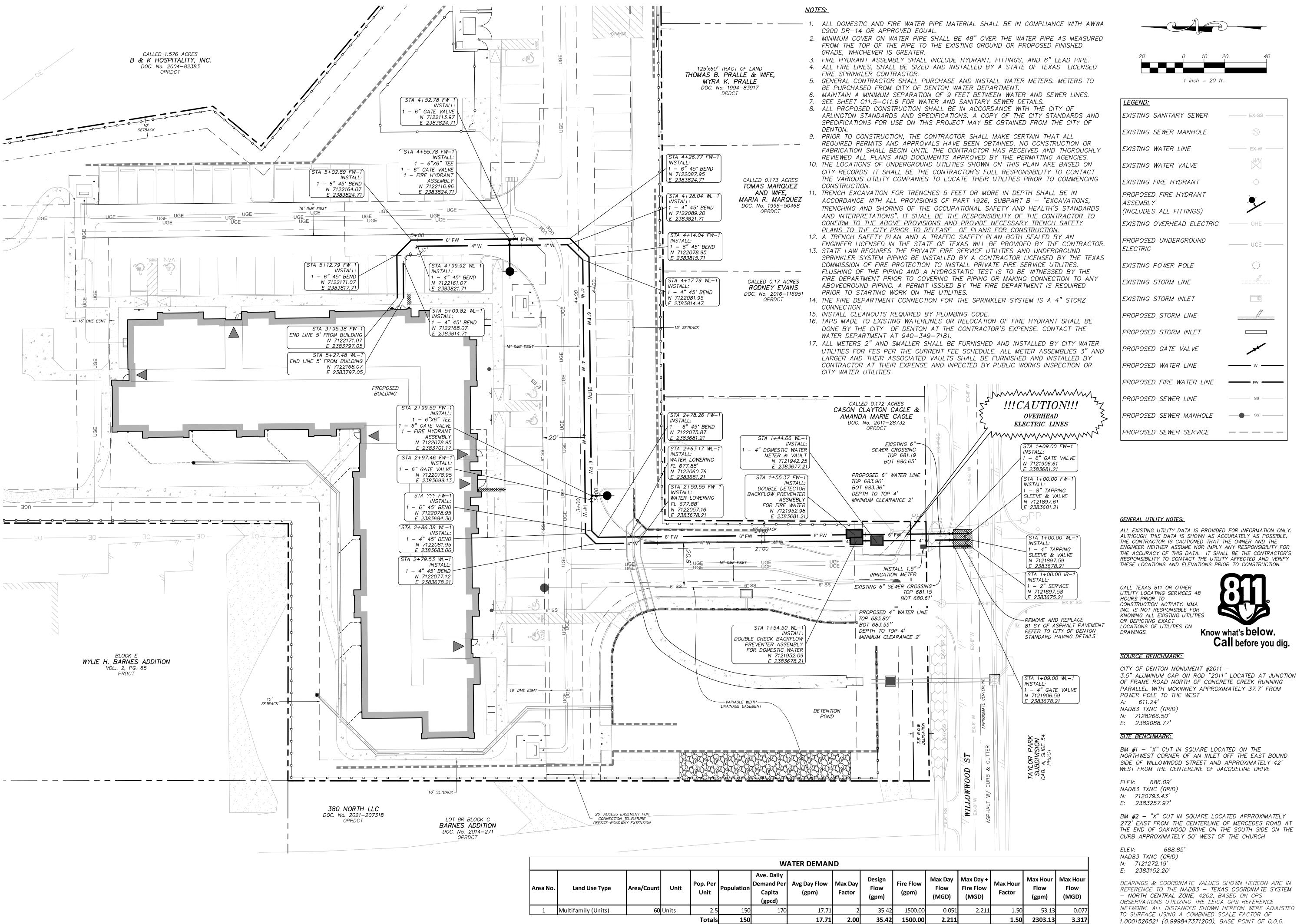


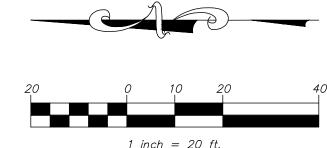
CONNECT TO EX POWERPOLE

10' SETBACK

LESLIE ST

PROJE	ECT NUME	BER:	3597	-00-02
PROJE	ECT MAN	AGER:	R. CR	RONIN
DRAWN BY:			G. SA	NCHEZ
CHEC	KED BY:		A. TA	YLOR
ISSUE	DATE:		10/4,	/2023





LEGEND:	
EXISTING SANITARY SEWER	——— EX-SS ———
EXISTING SEWER MANHOLE	(\$)
EXISTING WATER LINE	——— EX-W ———
EXISTING WATER VALVE	WY
EXISTING FIRE HYDRANT	
PROPOSED FIRE HYDRANT ASSEMBLY (INCLUDES ALL FITTINGS)	3
(INCLODES ALL FITTINGS) - EXISTING OVERHEAD ELECTRIC	OHE
EXISTING OVERHEAD ELECTRIC	——— OHE ———
PROPOSED UNDERGROUND ELECTRIC	——— UGE ———
EXISTING POWER POLE	\bigcirc
EXISTING STORM LINE	==== <u>///</u> ==
EXISTING STORM INLET	
PROPOSED STORM LINE	
PROPOSED STORM INLET	
PROPOSED GATE VALVE	X
PROPOSED WATER LINE	— w —
PROPOSED FIRE WATER LINE	
PROPOSED SEWER LINE	SS
PROPOSED SEWER MANHOLE	ss
 PROPOSED SEWER SERVICE	

ALL EXISTING UTILITY DATA IS PROVIDED FOR INFORMATION ONLY. ALTHOUGH THIS DATA IS SHOWN AS ACCURATELY AS POSSIBLE, THE CONTRACTOR IS CAUTIONED THAT THE OWNER AND THE ENGINEER NEITHER ASSUME NOR IMPLY ANY RESPONSIBILITY FOR THE ACCURACY OF THIS DATA. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE UTILITY AFFECTED AND VERIFY



3.5" ALUMINUM CAP ON ROD "2011" LOCATED AT JUNCTION OF FRAME ROAD NORTH OF CONCRETE CREEK RUNNING PARALLEL WITH MCKINNEY APPROXIMATELY 37.7' FROM

BM #1 - "X" CUT IN SQUARE LOCATED ON THE NORTHWEST CORNER OF AN INLET OFF THE EAST BOUND SIDE OF WILLOWWOOD STREET AND APPROXIMATELY 42' WEST FROM THE CENTERLINE OF JACQUELINE DRIVE

BM #2 - "X" CUT IN SQUARE LOCATED APPROXIMATELY 272' EAST FROM THE CENTERLINE OF MERCEDES ROAD AT THE END OF OAKWOOD DRIVE ON THE SOUTH SIDE ON THE CURB APPROXIMATELY 50' WEST OF THE CHURCH

PROJECT NUMBER: 3597-00-02 PROJECT MANAGER: R. CRONIN G. SANCHEZ RAWN BY: CHECKED BY: A. TAYLOR 10/4/2023 SUE DATE: REV. DATE DESCRIP.

tbpels registration number: f - 2759

tbpels registration/license number: 10088000

arlington, texas 76010

8 1 7 - 4 6 9 - 1 6 7 1

fax: 817-274-8757

www.mmatexas.com

*

ROBERT J. CRONIN JE

122288

519 east border

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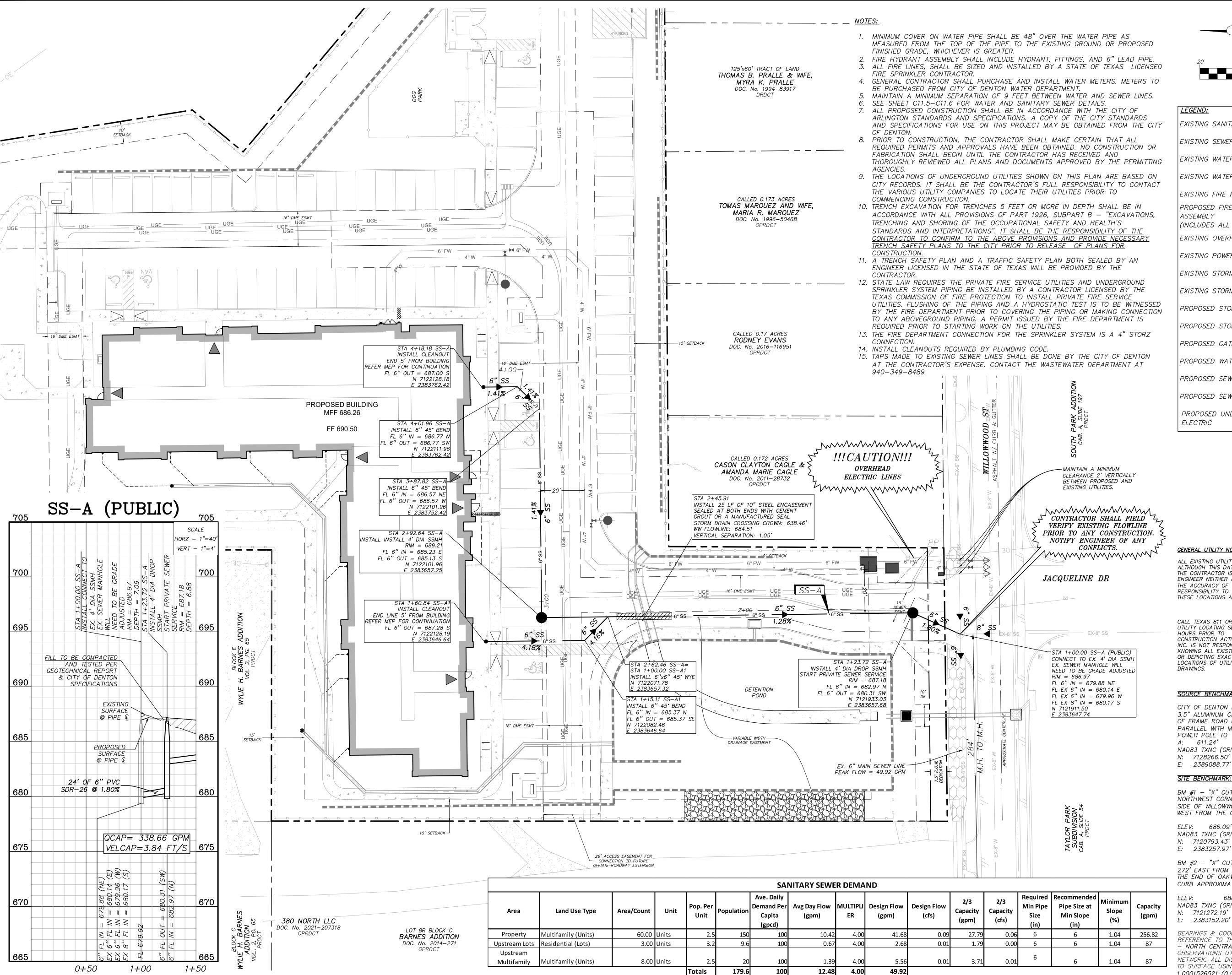
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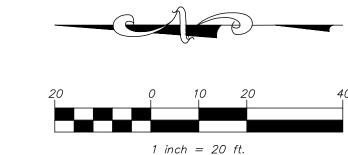
DENTO

WATER PLAN

SHEET CONTENT:

C7.1





<u>LEGEND:</u>	
EXISTING SANITARY SEWER	EX-SS
EXISTING SEWER MANHOLE	(\$)
EXISTING WATER LINE	EX-W
EXISTING WATER VALVE	WV
EXISTING FIRE HYDRANT	
PROPOSED FIRE HYDRANT ASSEMBLY (INCLUDES ALL FITTINGS)	
EXISTING OVERHEAD ELECTRIC	—— ОНЕ —
EXISTING POWER POLE	\bigcirc
EXISTING STORM LINE	==== <u>///</u> ===
EXISTING STORM INLET	
PROPOSED STORM LINE	
PROPOSED STORM INLET	
PROPOSED GATE VALVE	
PROPOSED WATER LINE	w
PROPOSED SEWER LINE	ss
PROPOSED SEWER MANHOLE	ss
PROPOSED UNDERGROUND ELECTRIC	——— UGE —

GENERAL UTILITY NOTES:

ALL EXISTING UTILITY DATA IS PROVIDED FOR INFORMATION ONLY. ALTHOUGH THIS DATA IS SHOWN AS ACCURATELY AS POSSIBLE, THE CONTRACTOR IS CAUTIONED THAT THE OWNER AND THE ENGINEER NEITHER ASSUME NOR IMPLY ANY RESPONSIBILITY FOR THE ACCURACY OF THIS DATA. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO CONTACT THE UTILITY AFFECTED AND VERIFY THESE LOCATIONS AND ELEVATIONS PRIOR TO CONSTRUCTION.

CALL TEXAS 811 OR OTHER UTILITY LOCATING SERVICES 48 HOURS PRIOR TO CONSTRUCTION ACTIVITY. MMA INC. IS NOT RESPONSIBLE FOR KNOWING ALL EXISTING UTILITIES OR DEPICTING EXACT LOCATIONS OF UTILITIES ON



SOURCE BENCHMARK:

CITY OF DENTON MONUMENT #2011 -3.5" ALUMINUM CAP ON ROD "2011" LOCATED AT JUNCTION OF FRAME ROAD NORTH OF CONCRETE CREEK RUNNING PARALLEL WITH MCKINNEY APPROXIMATELY 37.7' FROM POWER POLE TO THE WEST

NAD83 TXNC (GRID) N: 7128266.50'

SITE BENCHMARK:

BM #1 - "X" CUT IN SQUARE LOCATED ON THE NORTHWEST CORNER OF AN INLET OFF THE EAST BOUND SIDE OF WILLOWWOOD STREET AND APPROXIMATELY 42' WEST FROM THE CENTERLINE OF JACQUELINE DRIVE

ELEV: 686.09'

NAD83 TXNC (GRID) N: 7120793.43'

BM #2 - "X" CUT IN SQUARE LOCATED APPROXIMATELY 272' EAST FROM THE CENTERLINE OF MERCEDES ROAD AT THE END OF OAKWOOD DRIVE ON THE SOUTH SIDE ON THE CURB APPROXIMATELY 50' WEST OF THE CHURCH

NAD83 TXNC (GRID) N: 7121272.19' E: 2383152.20'

BEARINGS & COORDINATE VALUES SHOWN HEREON ARE IN REFERENCE TO THE NAD83 - TEXAS COORDINATE SYSTEM NORTH CENTRAL ZONE, 4202, BASED ON GPS OBSERVATIONS UTILIZING THE LEICA GPS REFERENCE NETWORK. ALL DISTANCES SHOWN HEREON WERE ADJUSTED TO SURFACE USING A COMBINED SCALE FACTOR OF 1.0001526521 (0.999847371200), BASE POINT OF 0,0,0.

DENTO

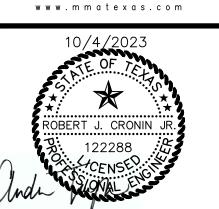
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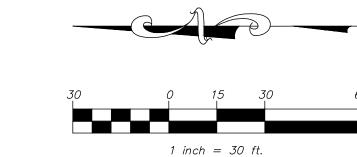
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3597-00-02

PROJECT MANAGER:		AGER: R. CRC	NIN	
DRAW	N BY:	G. SAN	G. SANCHEZ	
CHEC	KED BY:	A. TAY	LOR	
SSUE	DATE:	10/4/2	2023	
REV.	DATE	DESCRIP.	BY	
SHEE	T CONTEN	NT:		

SANITARY SEWER PLAN



EXISTING MAJOR CONTOUR

EXISTING MINOR CONTOUR

— *-640-* —

DRAINAGE AREA BOUNDARY

DRAINAGE AREA LABEL

NAME ---

AREA (ACRES)

FLOW AREA

- 1. PROPERTY IS ZONED PLANNED DEVELOPMENT 'PD' MULTI-FAMILY.
- 2. DESIGN CRITERIA IS BASED ON CITY OF DENTON STORMWATER DESIGN CRITERIA MANUAL DATED OCTOBER 2022.
- 3. EXISTING CONTOURS ARE BASED ON TOPOGRAPHIC SURVEY PREPARED BY MMA, DATED DECEMBER, 2022 AND LIDAR COMPLETED IN 2022 DOWNLOADED FROM
- 4. DRAINAGE AREAS ARE BASED ON PROPOSED GRADING DESIGN, EXISTING TOPOGRAPHY, AND ASBUILT PLAN INFORMATION.
- 5. REFER TO SHEETS C8.4-#### FOR STORM PLAN AND PROFILES.

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Know what's below. Call before you dig.

SOURCE BENCHMARK:

CITY OF DENTON MONUMENT #2011 -3.5" ALUMINUM CAP ON ROD "2011" LOCATED AT JUNCTION OF FRAME ROAD NORTH OF CONCRETE CREEK RUNNING PARALLEL WITH MCKINNEY APPROXIMATELY 37.7' FROM POWER POLE TO THE WEST

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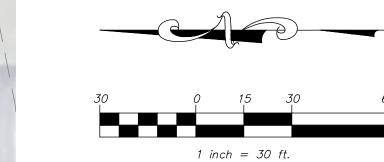


PROJECT MANAGER:

RAW	N BY:	G. SANCHE	Z
HECKED BY:		A. TAYLOR	
SSUE	DATE:	10/4/2023	i
REV.	DATE	DESCRIP.	BY
SHEE	T CONTE	NT:	

R. CRONIN

PRE-DEVELOPED DRAINAGE AREA MAP



LEGEND: EXISTING MAJOR CONTOUR *− −640− −* EXISTING MINOR CONTOUR *− −641− −* ----640---- PROPOSED MINOR CONTOUR PROPOSED MAJOR CONTOUR -----641----PROPOSED STORM DRAIN DRAINAGE AREA BOUNDARY DRAINAGE AREA LABEL 2.6 AREA (ACRES) -INLET DESIGNATION XX

NOTES:

FLOW AREA

DESIGN POINT

- 1. PROPERTY IS ZONED PLANNED DEVELOPMENT 'PD' MULTI-FAMILY.
- 2. DESIGN CRITERIA IS BASED ON CITY OF DENTON STORMWATER DESIGN CRITERIA MANUAL DATED OCTOBER 2022.
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- 5. REFER TO SHEETS C8.4-C8.5 FOR STORM PLAN AND PROFILES.

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www.mmatexas.com ROBERT J. CRONIN JE 122288

PROJE	ECT NUME	BER: 3597-00	-02
PROJE	ECT MAN	AGER: R. CRONI	N
DRAW	N BY:	G. SANC	ΗEZ
CHEC	KED BY:	A. TAYLO	R
ISSUE	DATE:	10/4/20	23
REV.	DATE	DESCRIP.	BY

POST DEVELOPED **DRAINAGE** AREA MAP

C8.1

ALL DRAINAGE CALCULATIONS ON THIS SHEET ARE
PERFORMED USING NRCS (SCS) METHODOLOGY. NO
RATIONAL CALCULATIONS ARE UTILIZED FOR THE DETENTION
POND PRE/POST COMPARISON.

EXISTING RUNOFF CALCULATIONS

	BASIN PARAMETERS										
Area Name	Area (ac.)	Base Composite CN	Percent Impervious	Composite Curve Number	Time of Concentration (min)						
EX A (DP 1)	1.88	80	24.57	84.4	4.58						
EX B (DP 2)	3.88	80	34.78	86.3	6.11						

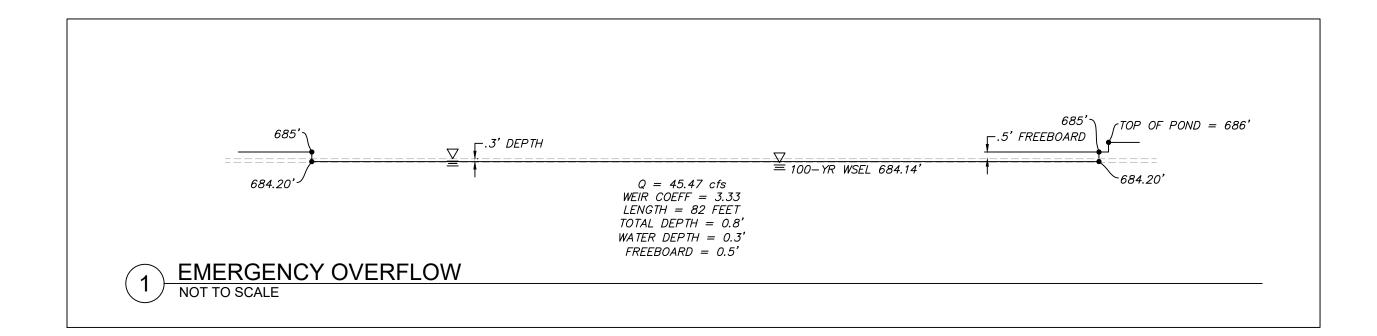
		Segment Information	n		Sh	eet Flow		Shallov	w Flow			Cha	nnel Flow				
DA	Segment	Flow Type	Length	Slope (ft/ftt)	Rainfall Depth 2yr24hr	Manning's Coefficient		Surface Type	Velocity (fps)	Section Area	Wetted Perimeter	Manning's Coefficient	Velocity (fps)	Description	Segment TOC (min)	Segment	С
EXA	1	Sheet Flow	50	0.0018	3.63	0.011	0.49								1.71	1	E
EXA	2	Shallow Concentrated Flow	89	0.0034				Paved	1.19						1.25	2	E
EXA	3	Shallow Concentrated Flow	295	0.0353				Unpaved	3.03						1.62	3	E
EX B	1	Sheet Flow	50	0.0064	3.63	0.011	0.81								1.03	1	E
EX B	2	Shallow Concentrated Flow	413	0.0171				Paved	2.66						2.59	2	E.
EX B	3	Shallow Concentrated Flow	462	0.0366				Unpaved	3.09						2.49	3	Е

PROPOSED RUNOFF CALCULATIONS

BASIN PARAMETERS

Area Name	Area (ac.)	Base Composite CN	Percent Impervious	Composite Curve Number	Time of Concentration (min)
Α	1.41	80	33.55	86	4.49
B1	2.83	80	71.26	92.8	7.5
B2	1.26	80	57.76	90.4	5.78
В3	0.25	80	28.14	85.1	5.12

PΑ	Segment	Flow Type	Length	Slope (ft/ft)	Rainfall Depth 2yr24hr	Manning's Coefficient	Velocity (ft/s)	Surface Type	Velocity (ft/s)	Section Area	Wetted Perimeter	Manning's Coefficient	Velocity (ft/s)	Description	Segment TOC (min)	Segment	ı
A	1	Sheet Flow	50	0.0018	3.63	0.011	0.49								1.71	1	
A	2	Shallow Concentrated Flow	89	0.0034				Paved	1.19						1.25	2	Г
A	3	Shallow Concentrated Flow	283	0.0367				Unpaved	3.09						1.53	3	
31	1	Sheet Flow	50	0.0064	3.63	0.011	0.81								1.03	1	
31	2	Shallow Concentrated Flow	410	0.0165				Paved	2.61						2.62	2	
31	3	Shallow Concentrated Flow	16	0.0775				Unpaved	4.49						0.06	3	
31	4	Shallow Concentrated Flow	160	0.0284				Paved	3.43						0.78	4	
31	5	Channel Flow	150	0.0284						0.284	5.9	0.04	0.83	Trapezoid channel, 2' bottom, 50:1;4:1 sides, .07' depth	3.01	5	1
32	1	Sheet Flow	50	0.0322	3.63	0.15	0.19								4.37	1	
32	2	Shallow Concentrated Flow	93	0.0288				Unpaved	2.74						0.57	2	
32	3	Channel Flow	325	0.01						1.77	4.71	0.012	6.44	18" HDPE pipe, flowing full	0.84	3	
33	1	Sheet Flow	77	0.0514	3.63	0.15	0.25								5.12	1	



ANALYSIS POINT PRE/POST COMPARISON

Junction	Existing Discharge (cfs)			Propo	sed Dischar	ge (cfs)	Difference			
Junction	1-Yr	25-Yr	100-Yr	1-Yr	25-Yr	100-Yr	1-Yr	25-Yr	100-Yr	
1	6.5	17.8	23.8	5.1	13.6	18.1	-1.4	-4.2	-5.6	
2	13.8	36.6	48.7	12.5	35.9	48.1	-1.3	-0.7	-0.5	

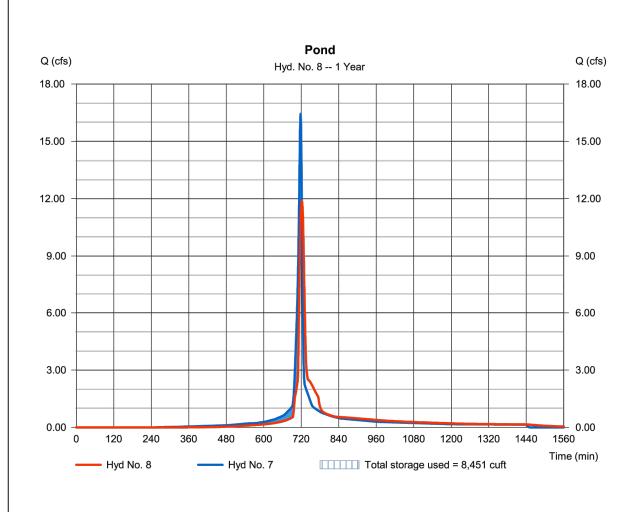
Detention Pond Results

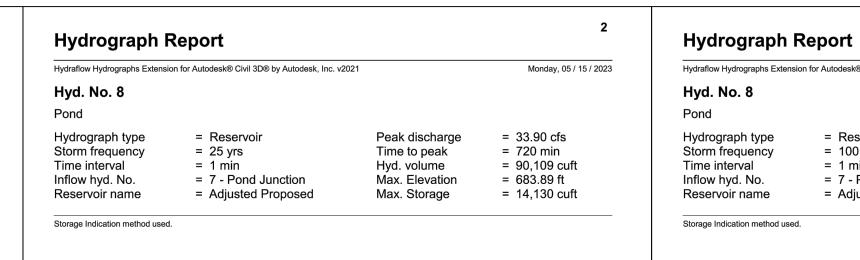
Storm	Peak inflow	Peak Outflow	WSEL	Storage	Freeboard
(year)	(cfs)	(cfs)	VVJEL	(cu-ft)	rieebbaiu
WQ		0.092	681.46	12481	4.54
1	16.44	11.88	683.11	8451	2.89
25	38.51	33.90	683.89	14130	2.11
100	50.10	45.47	684.14	16117	1.86
Outfall Structure	1	e (water quality) a elevation 682' and			

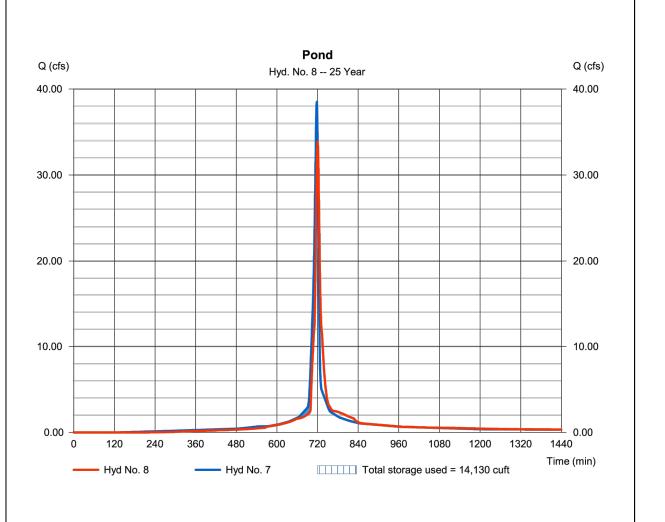
Top of Bond	686					
Structure	683.20'					
Structure	orifices at FL ele					

Top of Pond	686				
		Total Storage	Orifice	Weir	Total
Stage	Elevation	(cu-ft)	Discharge	Discharge	Discharge
		(64 16)	(cfs)	(cfs)	(cfs)
0	682.00	0	0.00	0.00	0.00
0.40	682.40	3076	1.60	0.00	1.60
0.80	682.80	6168	7.04	0.00	7.04
1.20	683.20	9071	12.52	0.00	12.52
1.60	683.60	11864	15.13	7.58	22.71
2.00	684.00	15002	17.36	21.44	38.80
2.20	684.20	16576	18.37	29.97	48.34
2.40	684.40	18154	19.33	39.40	58.73
2.60	684.60	19735	20.24	49.64	69.88
2.80	684.80	21320	21.12	60.65	81.77
3.00	685.00	22909	21.95	72.38	94.33
3.20	685.20	24501	22.76	84.77	107.53
3.40	685.40	26097	23.54	97.80	121.34
3.50	685.50	27696	23.92	104.54	128.46
3.80	685.80	29300	25.03	125.64	150.67
4.00	686.00	30906	25.74	140.42	166.16

Hydraflow Hydrographs Extensi	on for Autodesk® Civil 3D® by Autodesk, Inc. v2	2021	Monday, 05 / 15 / 2023
Hyd. No. 8			
Pond			
Hydrograph type	= Reservoir	Peak discharge	= 11.88 cfs
Storm frequency	= 1 yrs	Time to peak	= 722 min
Time interval	= 1 min	Hyd. volume	= 36,216 cuft
Inflow hyd. No.	= 7 - Pond Junction	Max. Elevation	= 683.11 ft
Reservoir name	Adjusted Proposed	Max. Storage	= 8,451 cuft



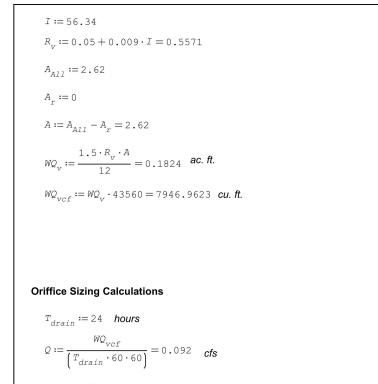


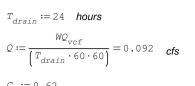


Q (cfs)
60 00
55.00
50.00
40.00
30.00
30.00
20.00
40.00
10.00
0.00 0

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2021

WATER QUALITY VOLUME CALCULATIONS





 $C_d := 0.62$ h := 1.8 ft

tbpels registration number: f - 2759 tbpels registration/license number: 10088000 519 east border arlington, texas 76010 8 1 7 - 4 6 9 - 1 6 7 1 fax: 817-274-8757

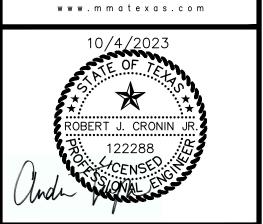
ALCULATIONS

DETENTION

AT MAGNOLIA TEXAS

RESERVES DENTON,

THE



Monday, 05 / 15 / 2023

PROJE	ECT NUME	BER:	35	597-00-	02
PROJE	ECT MAN	AGER:	R.	CRONIN	
DRAW	N BY:		G.	SANCHE	Z
CHEC	KED BY:		A.	TAYLOR	
ISSUE	DATE:		10	/4/2023	3
REV.	DATE		ESCRI	Ρ.	BY
SHEE	T CONTEN	NT:			

DETENTION CALCULATIONS

C8.2

RATIONAL METHOD CALCULATIONS ON THIS SHEET ARE USED FOR STORM SYSTEM CALCULATIONS ONLY

Reserves at Magnolia

Resei	ves	al I	viagi	iolia	

								PROI	POSED C	CONDITIO	NS DRAIN	NAGE A	REA CC	MPUTATIO	ONS						
Area Name	Runoff Coef. "C"	AREA "A" (ac.)	Total "CA"	Time of Concentration (min)	Ca 2	C*Ca 2	l 2yr (in/hr)	Q 2yr (cfs)	Ca 5	C*Ca 5	l5 (in/hr)	Q5 (cfs)	Ca 25	C*Ca 25	l25 (in/hr)	Q25 (cfs)	Ca 100	C*Ca 100	l100 (in/hr)	Q100 (cfs)	COMMENTS
1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22
B1	0.77	2.83	2.17	10.00	1.00	0.77	4.32	9.35	1.00	0.77	5.57	12.07	1.10	0.84	7.70	18.34	1.25	0.96	9.45	25.56	TO PROP. CULVERT
B2	0.85	0.22	0.19	10.00	1.00	0.85	4.32	0.81	1.00	0.85	5.57	1.04	1.10	0.94	7.70	1.58	1.25	1.00	9.45	2.08	TO PROP. CURB INLET C1
B3	0.85	0.26	0.22	10.00	1.00	0.85	4.32	0.95	1.00	0.85	5.57	1.23	1.10	0.94	7.70	1.87	1.25	1.00	9.45	2.46	TO LANDSCAPE DRAINS
B4	0.85	0.18	0.15	10.00	1.00	0.85	4.32	0.66	1.00	0.85	5.57	0.85	1.10	0.94	7.70	1.30	1.25	1.00	9.45	1.70	ROOF DRAINS
B5	0.85	0.22	0.19	10.00	1.00	0.85	4.32	0.81	1.00	0.85	5.57	1.04	1.10	0.94	7.70	1.58	1.25	1.00	9.45	2.08	ROOF DRAINS
B6	0.85	0.13	0.11	10.00	1.00	0.85	4.32	0.48	1.00	0.85	5.57	0.62	1.10	0.94	7.70	0.94	1.25	1.00	9.45	1.23	TO LANDSCAPE DRAINS
B7	0.53	0.27	0.14	10.00	1.00	0.53	4.32	0.62	1.00	0.53	5.57	0.80	1.10	0.58	7.70	1.21	1.25	0.66	9.45	1.69	DETENTION POND

	Reserves at Magnolia														
	WEIGHTED C VALUE COMPUTATIONS														
AREA NAME	DESCRIPTION	AREA - A (AC.)	"C"	C*A	WEIGHTED										
Area No.															
	B1 MULTIFAMILY HEAVY	1.2400	0.85	1.0540											
	B1 COMMERCIAL HEAVY	0.7850	0.80	0.6280											
	B1 SINGLE FAMILY (1/4 ACRE LOTS)	0.8050	0.60	0.4830											
				-											
				-											
	TOTALS	2.8300		2.1650	0.765										
Area No.															
	B7 POND	0.1920	0.40	0.0768											
	B7 MULTIFAMILY	0.0780	0.85	0.0663											
				-											
				-											
				-											
	TOTALS	0.2700	_	0.1431	0.530										

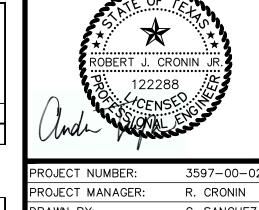
	Reserves at Magnolia Storm Hydraulic Calculations																																								
	Station	Pipe	e Drainage		Total		Increm	ent Tota	al L	Time of Co	oncentratio	n		1		Q	Crown	Inlet		Pip	e Size	Mannin	Pipe		Н	GL			Head L	Loss Calcul	ations			Entrance Co	ontrol	Design	Inver	t	Pipe		
Line No.	From T	o Leng		Area	Area	С	C _a /C _f C*C _f *			Travel	Increment	Design	15	(Design Below)	Q5	(Design	Million Co.	Bypass	Q _{Pipe}	No. Spa	n Dia/Hei	ght s	Capacity) Se	D/S	U/S	V_1	V ₂	V ₁ ² /2g	V ₂ ² /2g	K_j	$K_j V_1^2/2g$	H _k	Hwic C	ontrol		rom	То	Slope	T/C	Comments
	ft f	t		acres	acres				min	min	min	min	in/hr	in/hr	cfs	cfs	cfs	cfs	cfs	ft	in/ft		cfs	%	ft	ft	ft/s	ft/s	ft	ft		ft	ft	ft Tru	e/False	ft	ft	ft	%	ft	
	1 2	2 3	4	5	6	7a	7b 8	9	10	11	12	13	14	<i>15</i>	16	17	18a	18b	19	20a 20b	20c	21	22	23	24	25	26	27	28	29	30	31	32	33	34	<i>35</i> .	36	<i>37</i>	38	39	40
SD-A														100		100																									
	1+59.05 1+49	9.97 9.08	8 WQ		0.00	0.53	1.25	0.00	0 10.00	3.03	10.00	10.00	5.57	9.45	0.00	0.00			0.09	1	18	0.013	13.25	0.00%	679.10	679.10	0.00	0.05	0.00	0.00	0.00	0.00	0.00	0.00	ALSE	679.10 67	8.16	678.02	1.59%	682.00	Continuation From Another Line, Super Critical Flow
	1+49.97 1+00	0.00 49.9	7		0.00	0.53	1.25	0.00	0 10.00	16.66	13.03	10.00	5.57	9.45	0.00	0.00			0.09	1	18	0.015	11.48	0.00%	679.00	679.00	0.05	0.05	0.00	0.00	1.00	0.00	0.10	0.00	ALSE	679.10 67	8.02	677.22	1.59%	686.43	Manhole on Main Line with no Branch
SD-B														100		100																									
1	1+08.00 1+00	0.00 8.00) B2	0.22	0.22	0.85	1.25 0.22	0.22	2 10.00	0.11	10.00	10.00	5.57	9.45	1.23	2.08			2.08	1	18	0.013	19.65	0.04%	684.14	684.14	0.00	1.18	0.00	0.02	1.25	0.00	0.10	0.00 F	ALSE	684.24 68	2.28	682.00	3.50%	686.29	Inlet or Manhole at Beginning of Line
SD-C														100		100																									
1	1+45.20 1+38	8.09 7.11	1 B1	2.83	2.83	0.77	1.25 2.73	2.7	1 10.00	0.05	10.00	10.00	5.57	9.45	15.09	25.56			25.56	2	30	0.015	155.11	0.05%	684.36	684.36	0.00	2.60	0.00	0.11	0.50	0.00	0.10	1.70 F	ALSE	684.46 68	2.19	681.92	4.76%	687.46	Inlet on Main Line, Super Critical Flow
2	1+38.09 1+27	7.55 10.5	4		2.83			2.73	1 10.00	0.07	10.05	10.00	5.57	9.45	15.09	25.56			25.56	2	30	0.015	155.11	0.05%	684.25	684.26	2.60	2.60	0.11	0.11	0.35	0.04	0.10	0.00	ALSE	684.36 68	31.92	681.43	4.76%	688.01	Bend 45°
3	1+27.55 1+00	0.00 27.5	5		2.83			2.73	1 10.00	0.18	10.12	10.00	5.57	9.45	15.09	25.56			25.56	2	30	0.015	155.11	0.05%	684.14	684.15	2.60	2.60	0.11	0.11	0.10	0.01	0.10	0.00	ALSE	684.25 68	31.43	680.07	4.76%	687.46	Bend 11.25°
SD-D														100		100																									
1	4+30.22 3+76	6.24 53.9	8 B3 & B4	0.44	0.44	0.85	1.25 0.44	0.4	4 10.00	0.27	10.00	10.00	5.57	9.45	2.45	4.16			4.16	1	15	0.013	4.57	0.41%	687.30	687.52	0.00	3.39	0.00	0.18	1.25	0.00	0.22	1.30 F	ALSE	687.74 68	35.08	684.81	0.50%	688.33	Inlet or Manhole at Beginning of Line
2	3+76.24 3+54	4.73 21.5	1		0.44			0.44	4 10.00	0.11	10.27	10.00	5.57	9.45	2.45	4.16			4.16	1	15	0.013	4.57	0.41%	687.11	687.20	3.39	3.39	0.18	0.18	0.35	0.06	0.10	0.00	ALSE	687.30 68	34.81	684.70	0.50%	689.48	Bend 45°
3	3+54.73 2+46	6.85 107.8	88		0.44			0.44	4 10.00	0.53	10.38	10.00	5.57	9.45	2.45	4.16			4.16	1	15	0.013	4.57	0.41%	686.56	687.01	3.39	3.39	0.18	0.18	0.35	0.06	0.10	0.00 F	ALSE	687.11 68	34.70	684.16	0.50%	689.77	Bend 45°
4	2+46.85 2+22	2.37 24.4	8 B5&B6	0.35	0.79	0.85	1.25 0.35	0.79	9 10.00	0.07	10.91	10.91	5.38	9.11	4.25	7.20			7.20	1	15	0.013	4.57	1.24%	685.86	686.16	3.39	5.87	0.18	0.54	0.75	0.14	0.40	0.00	ALSE	686.56 68	4.16	684.04	0.50%	688.46	45° Wye Connection or Cut-in
	2+22.37 1+73				0.79			0.79	9 10.00	0.15	10.98	10.91	5.38	9.11	4.25	7.20			7.20	1	15	0.013	4.57	1.24%	685.03	685.67	5.87	5.87	0.54	0.54	0.35	0.19	0.19	0.00							Bend 45°
	1+71.08 1+43				0.79			0.79	9 10.00	0.12	11.13	10.91	5.38	9.11	4.25	7.20			7.20	1	18	0.013	7.43		684.67		5.87	4.07	0.54	0.26	0.35	+	0.10			684.91 68					
	1+41.19 1+00	0.00 41.1	9		0.79			0.79	9 10.00	0.17	11.25	10.91	5.38	9.11	4.25	7.20			7.20	1	18	0.013	7.43	0.47%	684.14	684.57	4.07	4.07	0.26	0.26	0.35	0.09	0.10	0.00 F	ALSE	684.67 68	3.38	683.18	0.50%	688.21	Bend 45°
SD-D1														100		100																									
1	1+15.72 1+00	0.00 15.7	2 B6	0.13	0.13	0.85	1.25 0.13	0.13	3 10.00	0.12	10.00	10.00	5.57	9.45	0.72	1.23			1.23	1	10	0.013	4.15	0.32%	686.56	686.61	0.00	2.26	0.00	0.08	1.25	0.00	0.10	0.74 F	ALSE	686.71 68	34.72	684.16	3.58%	687.39	Inlet or Manhole at Beginning of Line

	Curb Inlet and Drop Inlet in Sump																								
Inlet No	. Areal	lo. Are	12.1	year Flow (cfs)	100-year Carryover Flow (cfs)	100 Crown Overtop (cfs)	100 Total Gutter Flow Qo (cfs)	5-year Flow (cfs)	5-year Carryover Flow (cfs)	5 Crown Overtop (cfs)	Gutter Flow	Max Depth of Gutter Flow Yo (ft.)	100-year Actual Depth of Gutter Flow Yo (ft.)	5-year Actual Depth of Gutter Flow Yo (ft.)	Depth of Depression (ft.)	Depth of Flow at Opening Y (ft.)	Capacity of Inlet Per Foot of Length Q/L (cfs/ft.)		Clogging (%)	Max Depth Capacity of Inlet Q (cfs)	100 Carry Over Into OverFlow (cfs)	100- year Percent Q Captured By Inlet	5-year Carry Over Into OverFlow (cfs)	5- year Percent Q Captured By Inlet	Notes
1	2	3		4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25
C1	B2		0.22	2.08			2.08				0.00	0.28	0.24	0.00		0.24	0.44	5	(2.22	0	100	0	100	

Landscape Drain Sizing														
No. of Inlets	Q100 (cfs)	Design Head (ft.)		Inlet Size (in.)	Max Capacity (cfs)	Percent Captured (%)	Notes							
1	1.70	1.27	50	15	4.32	100	DOME GRATE INLET							
1	2.08	0.72	50	18	4.81	100	DOME GRATE INLET							
		No. of Q100 (cfs) 1.70	No. of Q100 Design Head (ft.) 1.70 1.27	No. of Q100 Design Clogging (cfs) Head (ft.) (%)	No. of Inlets Q100 (cfs) Design Head (ft.) Clogging (%) Inlet Size (in.) 1 1.70 1.27 50 15	No. of Inlets Copacity (cfs) Plead (ft.) Clogging (%) Inlet Size (in.) Capacity (cfs) 1 1.70 1.27 50 15 4.32	No. of Inlets Capacity (cfs) Head (ft.) Clogging (%) Inlet Size (in.) Max Capacity (cfs) (cfs) (%) 1 1.70 1.27 50 15 4.32 100							

RESERVES DENTON, THE

MAGNOLIA XAS



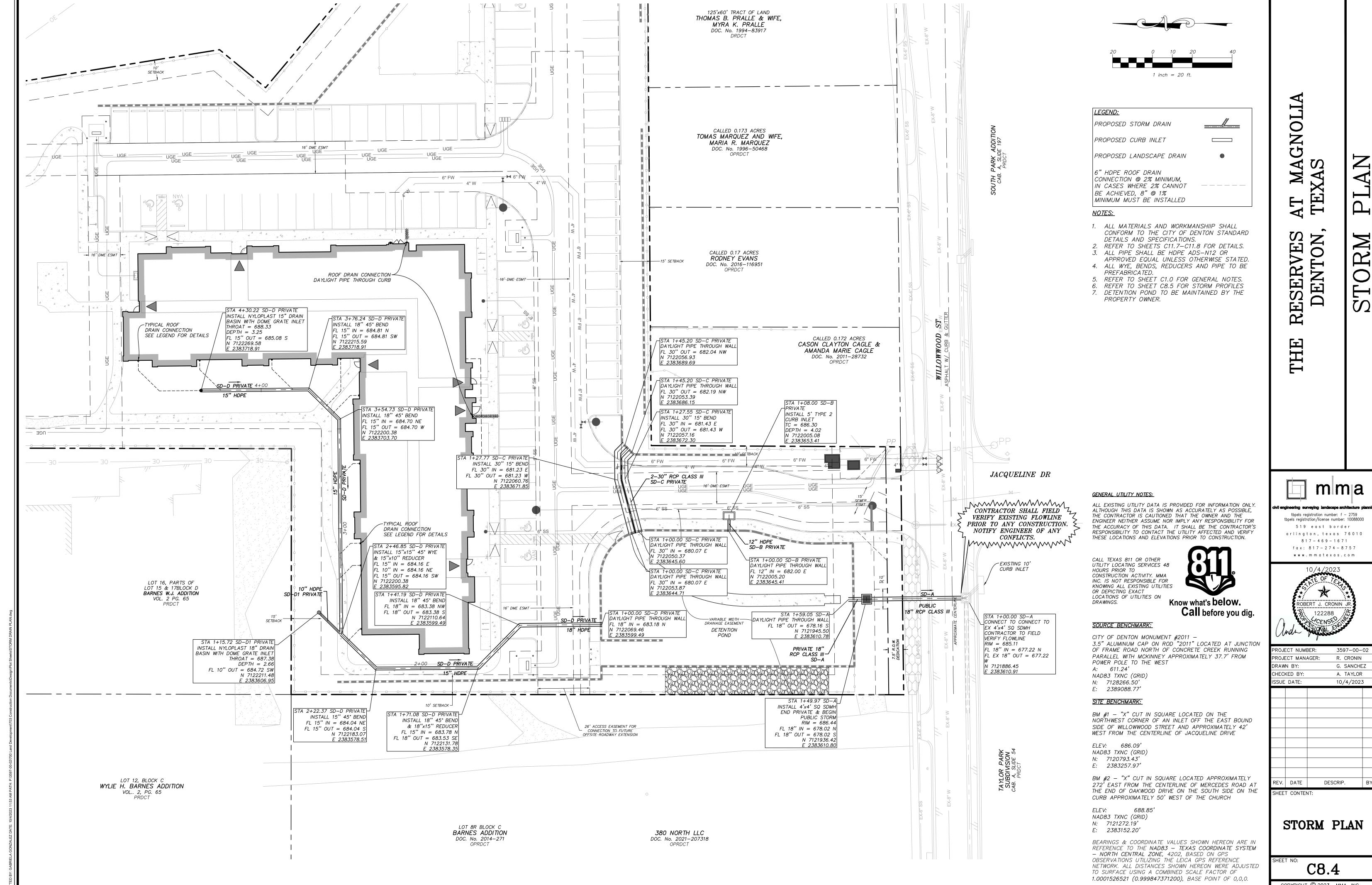
tbpels registration number: f - 2759 tbpels registration/license number: 10088000 519 east border arlington, texas 76010 8 1 7 - 4 6 9 - 1 6 7 1

fax: 817-274-8757 www.mmatexas.com

ſ	PROJE	ECT NUME	BER:	35	97-00-	02
	PROJE	ECT MAN	AGER:	R.	CRONIN	
	DRAW	N BY:		G.	SANCHE	Z
	CHEC	KED BY:		Α.	TAYLOR	
L	ISSUE	DATE:		10	/4/2023	3
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HYDRAULIC CALCULATIONS

C8.3



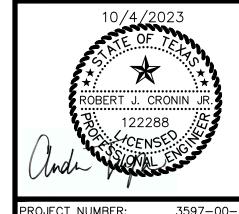
CHECK	ED BY:	A. TAYLOR	
SSUE	DATE:	10/4/2023	3
REV.	DATE	DESCRIP.	BY
SHEET	CONTEN	NT:	

THE RESERVES AT MAGNOLIA DENTON, TEXAS

PROFILES

STORM

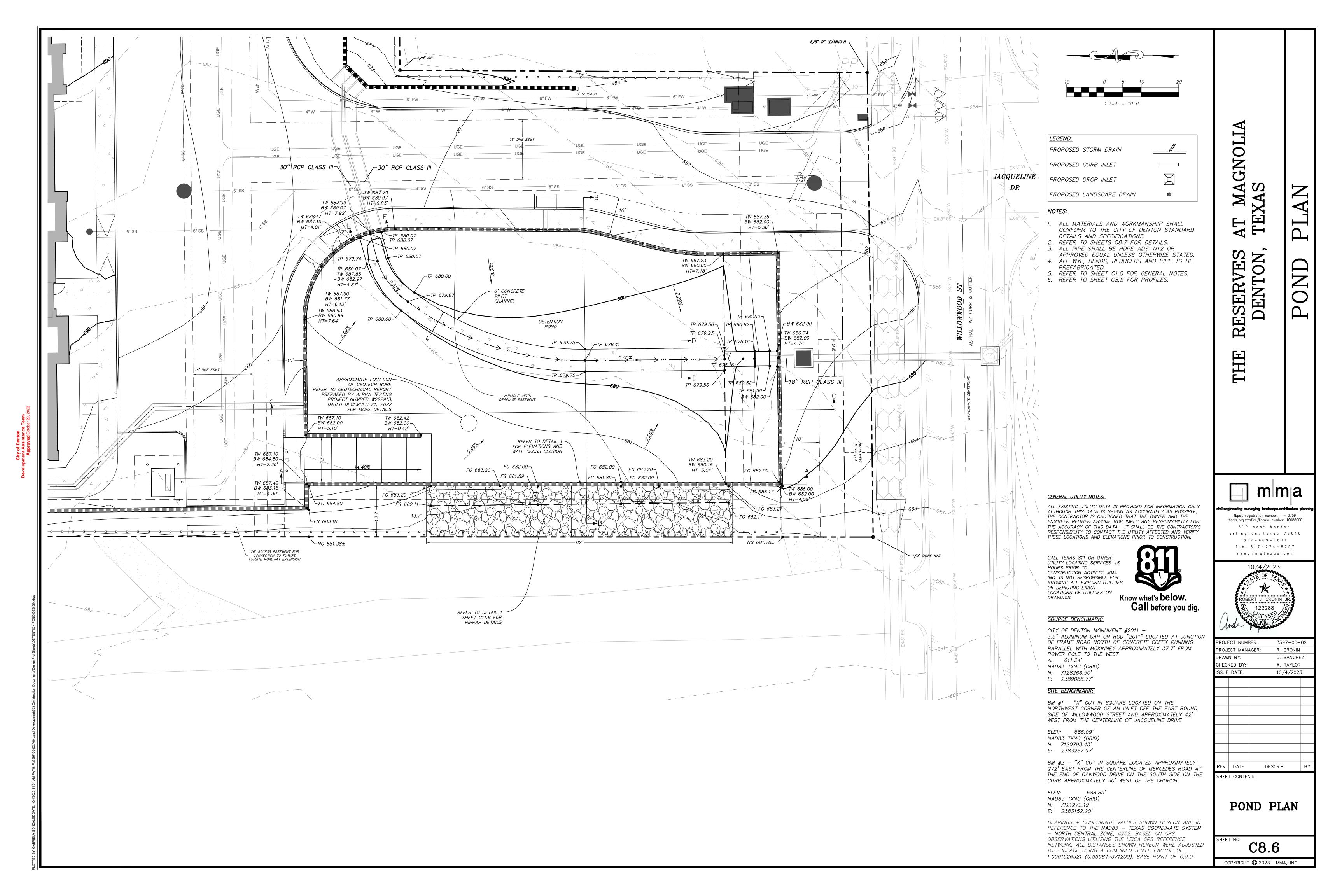
civil engineering surveying landscape architecture plants the surveying landscape architecture plants are surveying landscape architecture plants landscape architecture plantscape architecture plantscap

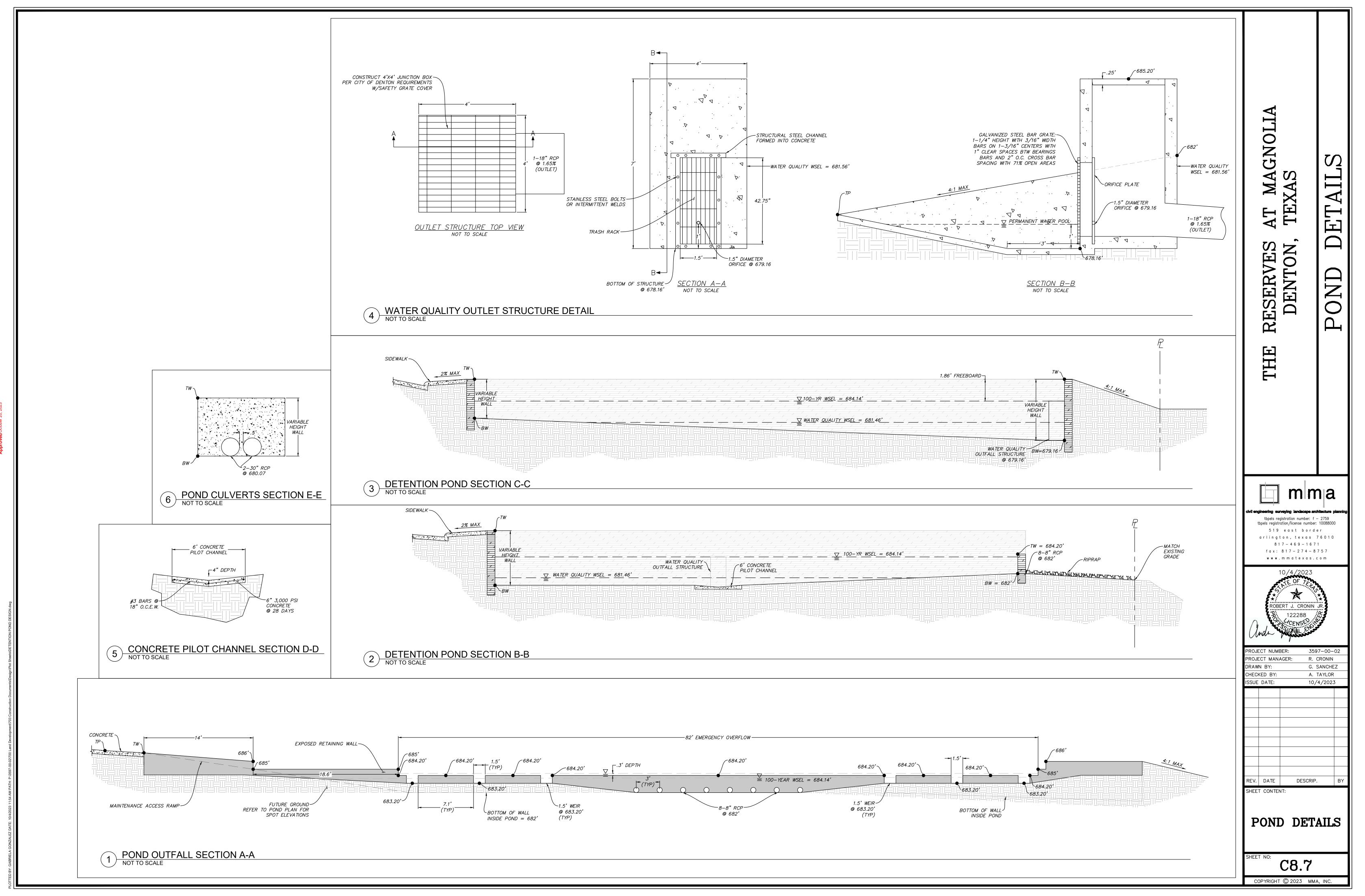


PROJE	ECT NUMI	BER: 359	3597-00-02						
PROJE	ECT MAN	AGER: R. (CRONIN						
DRAW	N BY:	G. 5	SANCHE	Z					
CHEC	KED BY:	Α.	TAYLOR						
ISSUE	DATE:	10/	4/2023	3					
REV.	DATE	DESCRIP.	,	ВУ					
SHEE	T CONTE	NT:							
_									

STORM PROFILES

C8.5





City of Denton

opment Assistance Team

- I. Concrete:
 - A. All concrete for foundations (walls and footings) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump.
- or slump.

 B. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.
- improved workability.

 C. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced.
- D. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement.
 E. No aluminum items shall be embedded in any concrete.

2. Reinforcing Steel:

- A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel.
 B. Clear minimum coverage of concrete over reinforcing steel shall be as follows:
- B. Clear minimum coverage of concrete over reinforcing steel shall be as follows:

 Concrete placed against earth 3"

 Formed concrete against earth 2"

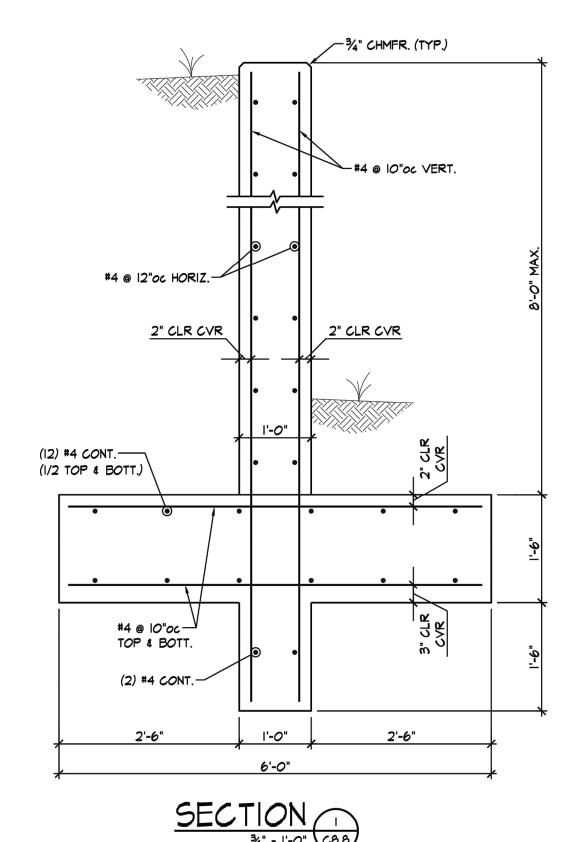
 Walls 2"
- All coverage shall be nominal bar diameter minimum.

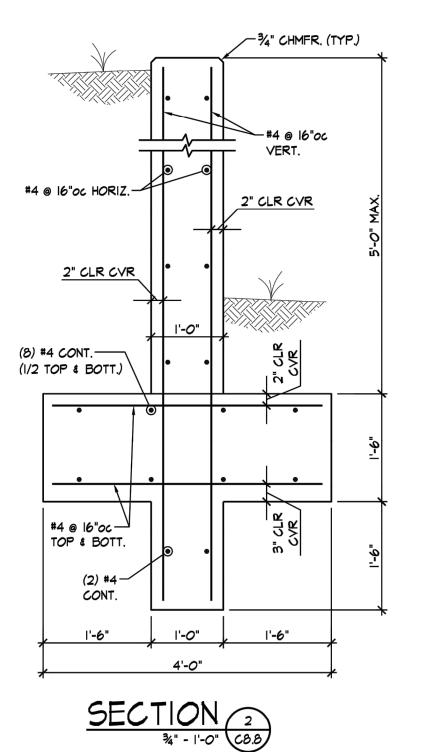
 C. At corners of all walls, supply corner bars (minimum 2'-6" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars.
- D. Bars marked continuous shall be lapped 48 bar diameters (3'-0" minimum) at splices.
- E. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.

Foundations:

- A. The soil investigation was prepared by Alpha Testing, LLC, the report number is W222913 and their telephone number is 817-496-5600.
- B. Spread footings and continuous wall footings are designed to bear on soil capable of safely sustaining 1500 psf.
- C. Contractor shall provide for dewatering at excavations
- from either surface water or seepage.

 D. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's









DETA

RETAININ



engineering surveying landscape architecture plan
tbpels registration number: f - 2759
tbpels registration/license number: 10088000
5 1 9 e a s t b o r d e r
a r l i n g t o n , t e x a s 7 6 0 1 0
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fax: 817-274-8757 www.mmatexas.com

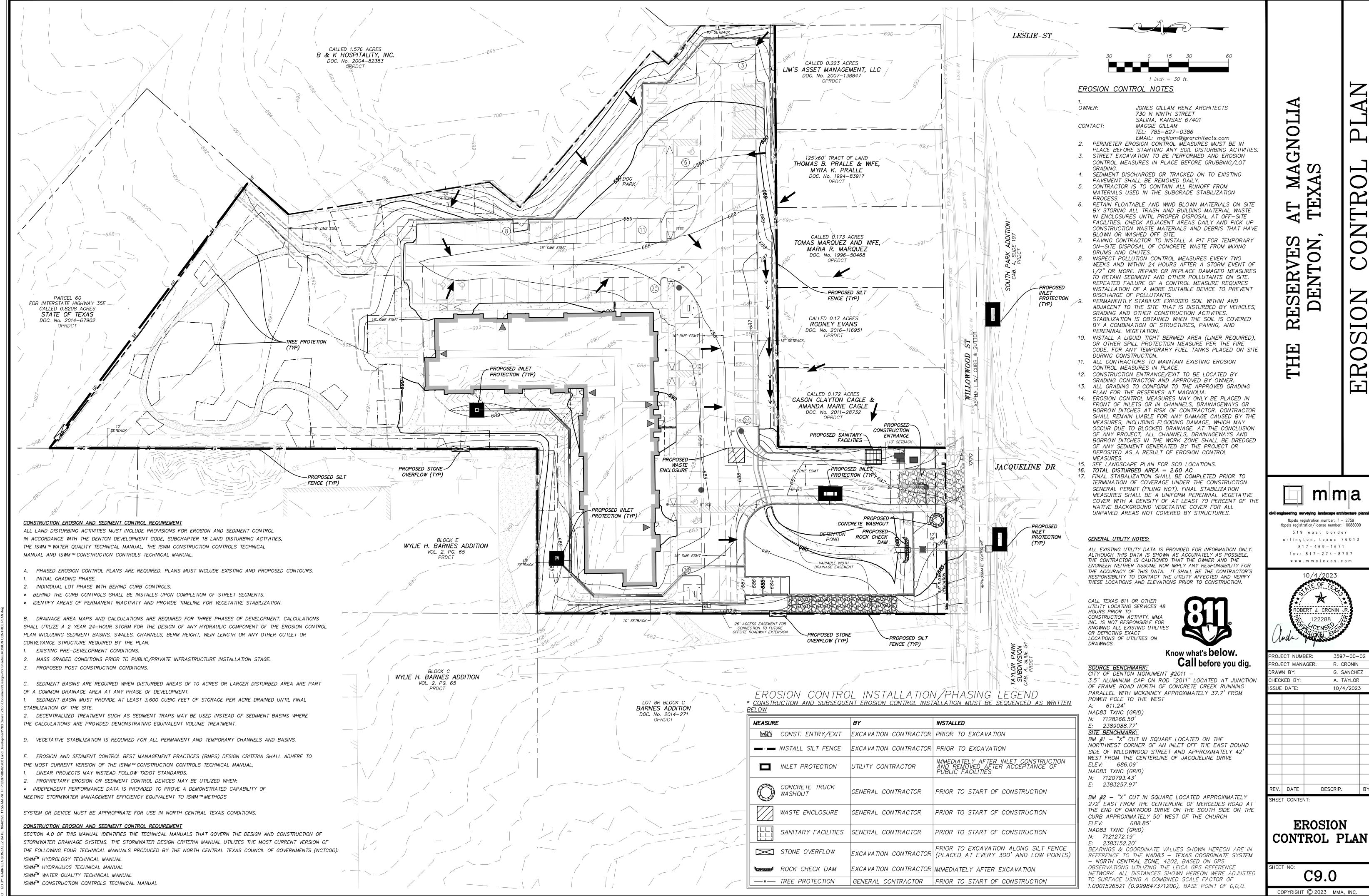
MICHAEL JOHN FALBE

		- /										
PROJE	ECT NUM	BER:	3597-00-	02								
PROJE	ECT MAN	AGER:	R. CRONIN									
DRAW	N BY:		J. FALBE									
CHEC	KED BY:		M. FALBE									
ISSUE	DATE:		5/16/2023	3								
REV.	DATE	DES	CRIP.	В								
SHEE	SHEET CONTENT:											

RETAINING WALL DETAILS

C8.8

City of Denton elopment Assistance Team Approved October 20, 2023



	7 7533		
ROJECT NUMI	BER:	35	97-00-02
ROJECT MAN	AGER:	R.	CRONIN
RAWN BY:		G.	SANCHEZ
CHECKED BY:		Α.	TAYLOR
SSUE DATE:		10	/4/2023

GNOLIA

RESERVES DENTON,

THE

mma

tbpels registration number: f - 2759 tbpels registration/license number: 10088000

817-469-1671 fax: 817-274-8757 www.mmatexas.com

ROBERT J. CRONIN JE

3597-00-02

R. CRONIN

G. SANCHEZ

A. TAYLOR

10/4/2023

PROJECT NUMBER:

DRAWN BY:

CHECKED BY:

SSUE DATE:

REV. DATE

PROJECT MANAGER:

519 east border arlington, texas 76010

Notes for Figure 6H-11—Typical Application 11 Lane Closure on a Two-Lane Road with Low Traffic Volumes

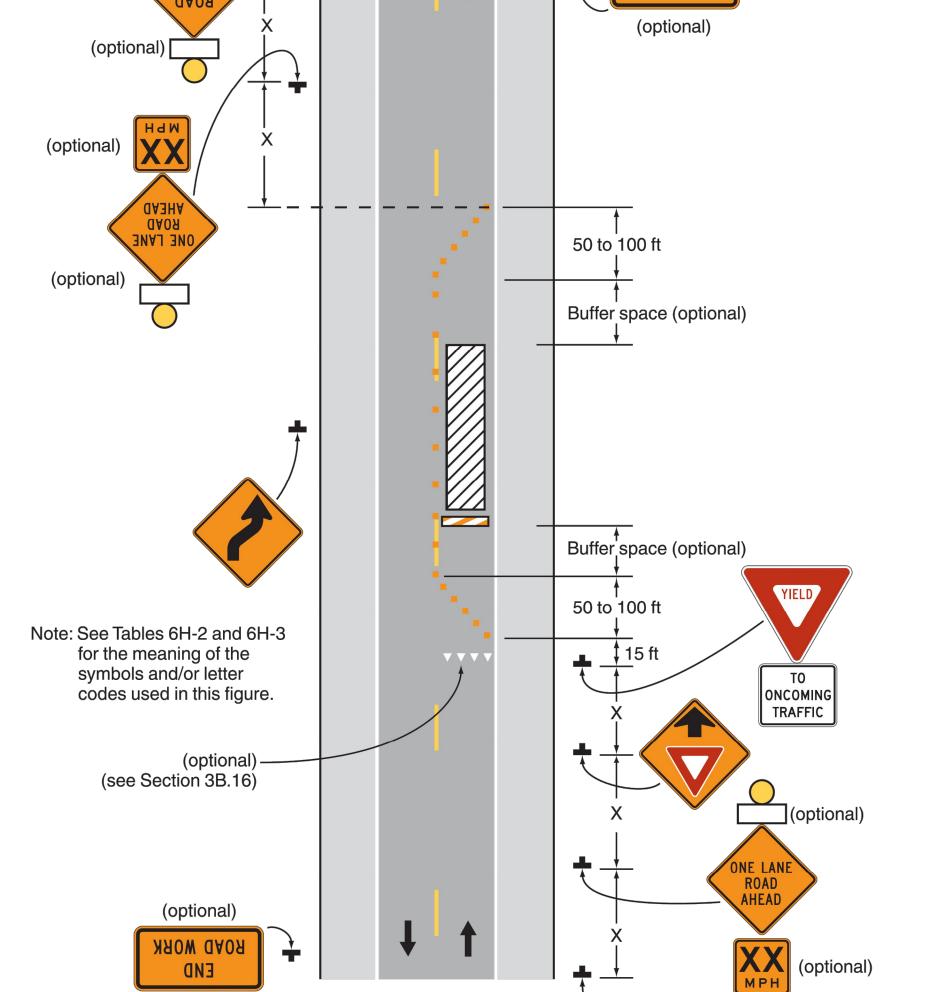
Option:

- This TTC zone application may be used as an alternate to the TTC application shown in Figure 6H-10 (using flaggers) when the following conditions exist:
- a. Vehicular traffic volume is such that sufficient gaps exist for vehicular traffic that must yield.
 b. Road users from both directions are able to see approaching vehicular traffic through and beyond the worksite and have sufficient visibility of approaching vehicles.
- The Type B flashing warning lights may be placed on the ROAD WORK AHEAD and the ONE LANE ROAD AHEAD signs whenever a night lane closure is necessary.

Guidance:

 The location of the yield bar should be selected using the same considerations as those applying to a flagger station (See section 6E.08, paragraphs 01-03).

Figure 6H-11. Lane Closure on a Two-Lane Road with Low Traffic Volumes (TA-11)



WORK

City of Denton
Development Assistance Team
Approved October 20, 2023



Anna Maria Canalana Dana mandala airal Bat Shadal CANGTDI (CTI) NOTANI S 4...

Table 6H-2. Meaning of Symbols on Typical Application Diagrams

Arrow board

0 0 0

Arrow board support or trailer

Direction of temporary traffic detour

Changeable message sign or support trailer

(shown facing down)

Channelizing device

Crash cushion

Direction of traffic

High-level warning device

Longitudinal channelizing device

Pavement markings that should be removed for a long-term project

Flagger

(Flag tree)

Shadow vehicle

Surveyor

 $\overline{}$

Sign (shown facing left)

Temporary barrier

Temporary barrier with warning light

Traffic or pedestrian signal

Truck-mounted attenuator

Type 3 barricade

Warning light

Work space

Work vehicle

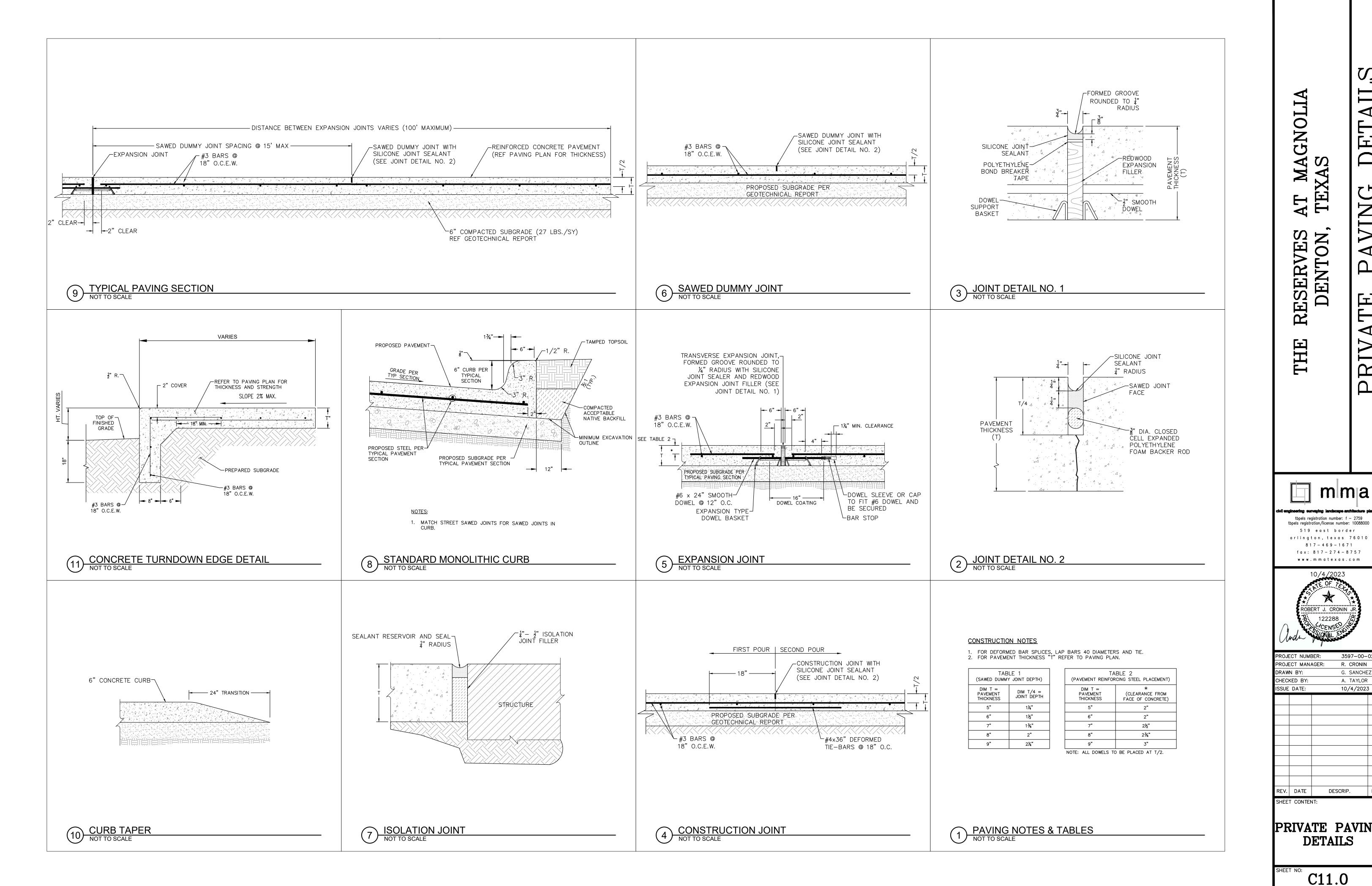
TED RV. GARBIEI A GONZAI EZ DATE: 40/4/2023 41-55 AM PATH: P.

C10.0

TRAFFIC

CONTROL PLAN

DESCRIP.



DETAILS PRIVATE

arlington, texas 76010 817-469-1671 fax: 817-274-8757 www.mmatexas.com * ROBERT J. CRONIN JE 122288 3597-00-02 R. CRONIN G. SANCHEZ A. TAYLOR 10/4/2023

PRIVATE PAVING **DETAILS**





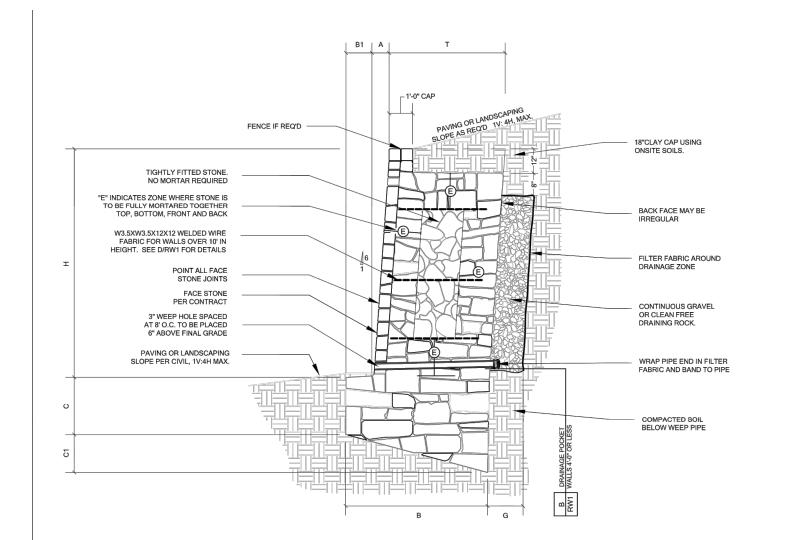


(6) RETAINING WALL DETAIL
NOT TO SCALE

RW1/1 MASONRY WALL W/ 6V:1H BATTER

WALL	BASE	TOE	BASE	BASE	BATTER	FULLY MORTARED	THICKNESS	DRAINAGE ZONE	BEARING
HEIGHT	WIDTH		DEPTH (TOE)	DEPTH (HEEL)		ZONE	OF WALL	THICKNESS	CAPACIT
Н	В	B1	С	C1	Α	E	Т	G	
1' - 0"	1' - 0"	0' - 0"	0' - 10"	0' - 2"	0' - 2"	FULLY	1' - 0"	SEE B/RW1	
2' - 0"	1' - 3"	0' - 0"	1' - 0"	0' - 3"	0' - 4"	FULLY	1' - 3"	SEE B/RW1	
3' - 0"	1' - 10"	0' - 0"	1' - 0"	0' - 4"	0' - 6"	FULLY	1' - 10"	SEE B/RW1	
4' - 0"	2' - 2"	0' - 0"	1' - 3"	0' - 5"	0' - 8"	FULLY	2' - 2"	SEE B/RW1	
5' - 0"	2' - 8"	0' - 1"	1' - 5"	0' - 6"	0' - 10"	0' - 11"	2' - 7"	0' - 10"	
6' - 0"	3' - 2"	0' - 2"	1' - 8"	0' - 7"	1' - 0"	1' - 0"	3' - 0"	0' - 10"	
7' - 0"	3' - 9"	0' - 7"	2' - 0"	0' - 8"	1' - 2"	1' - 1"	3' - 2"	1' - 0"	2500 ps
8' - 0"	4' - 8"	0' - 9"	2' - 3"	0' - 10"	1' - 4"	1' - 4"	3' - 11"	1' - 2"	
9' - 0"	5' - 6"	1' - 2"	2' - 8"	0' - 11"	1' - 6"	1' - 4"	4' - 4"	1' - 4"	
10' - 0"	6' - 3"	1' - 5"	3' - 0"	1' - 1"	1' - 8"	1' - 7"	4' - 10"	1' - 6"	
11' - 0"	7' - 1"	1' - 9"	3' - 3"	1' - 3"	1' - 10"	1' - 9"	5' - 4"	1' - 8"	
12' - 0"	7' - 8"	2' - 0"	3' - 6"	1' - 4"	2' - 0"	1' - 11"	5' - 8"	1' - 10"	
13' - 0"	8' - 3"	2' - 3"	3' - 9"	1' - 5"	2' - 2"	2' - 0"	6' - 0"	2' - 0"	
				WALL DES	IGN CRITERIA				
BEARING	SLOPE TOP	SLOPE BOT	ACTIVE PRESSURE	PASSIVE PRESSURE	COEFFICIENT OF FRICTION	SLOPE OF BACK OF WALL	SURCHARGE		
\mathbf{Q}_{a}	β	β ₁	Фа	Фр		α	q		
2500 psf	14.3 deg	14.3 deg	30 deg	30 deg	0.3	99.5 deg	0 psf	1	

. STRUCTURAL ENGINEER TO VERIFY ALL FINAL



6" 6" 6" ½" THICK GALVANIZED STEEL DIAMOND PLATE

-1" EXPANSION

-1" THICK GALVANIZED

2" X 1" GALVANIZED ANGLE IRON

WITH STUD FASTENER CAST IN

- 물" X 3" LONG STAINLESS STEEL;

CONTERSINK IN PLATE (4 REQ'D-2

CONCRETE.

EACH SIDE)

STEEL DIAMOND PLATE

CONCRETE-

SIDEWALK

SILICONE-

CONCRETE FLUME WITH

DIAMOND PLATE CROSSING
NOT TO SCALE

JOINT SEALANT

CONCRETE

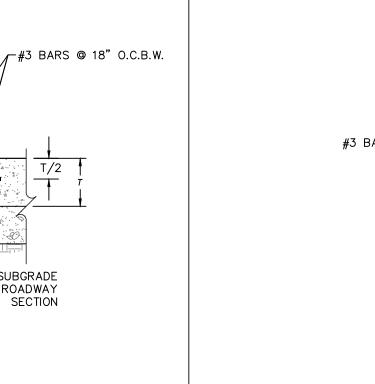
SIDEWALK

1" PREMOLDED

JOINT MATERIAL

EXPANSION

CONSTRUCTION JOINT
(BETWEEN EXISTING AND PROPOSED PAVEMENT)
NOT TO SCALE



EXPANSION CONSTRUCTION JOINT
(BETWEEN EXISTING AND PROPOSED PAVEMENT)
NOT TO SCALE

#5 x 18" DEFORMED BAR-

@ 18" O.C.

DOWELED AND EXPOXYED INTO EXISTING PAVEMENT

EXISTING CONCRETE-

EXISTING-

SUBGRADE

PAVEMENT

CONSTRUCTION JOINT WITH

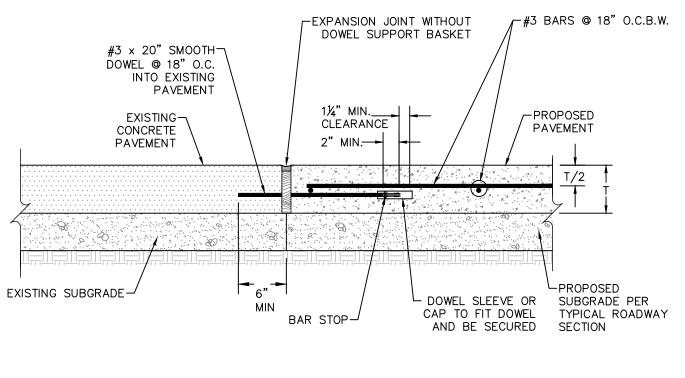
PROPOSED-PAVEMENT

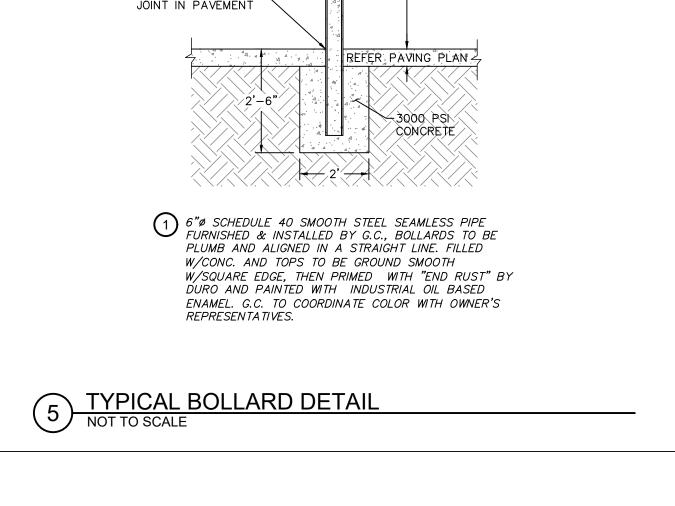
SILICONE JOINT SEALANT

PROPOSED SUBGRADE

PER TYPICAL ROADWAY

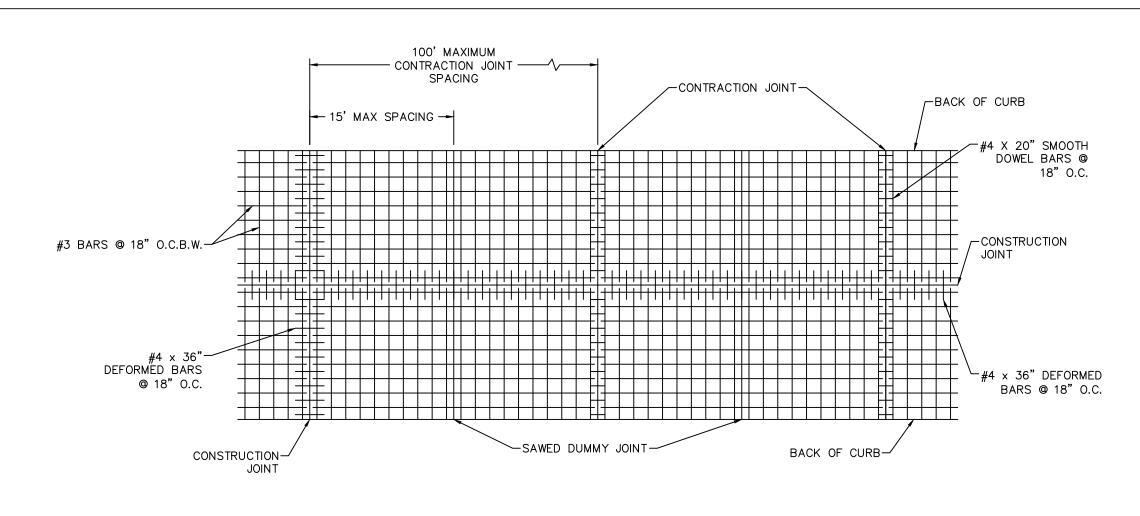
SECTION





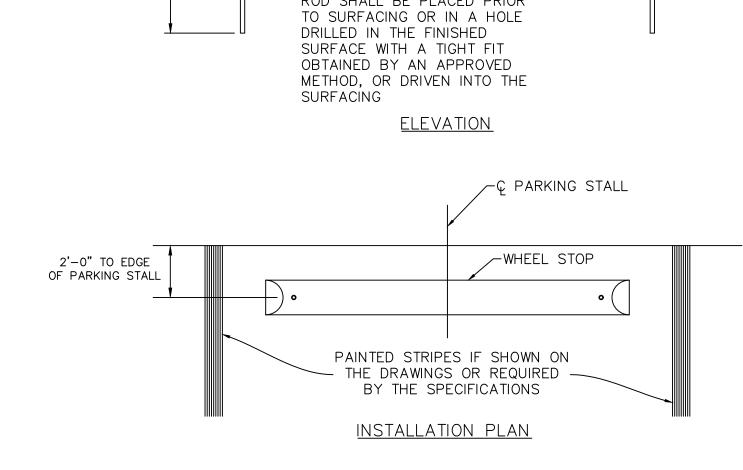
INSTALL ISOLATION-JOINT IN PAVEMENT

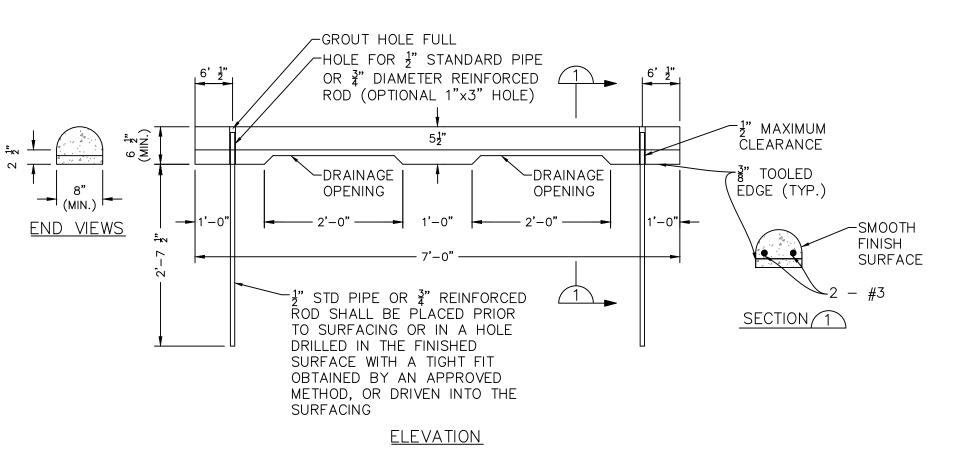
1 PAVEMENT STEEL REINFORCING GENERAL LAYOUT NOT TO SCALE



2 PRECAST CONCRETE WHEEL STOP DETAILS

NOT TO SCALE





<u>PLAN</u>

GNOLIA MA XAS AT TE ESERVES DENTON, RE THE

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3597-00-02

R. CRONIN

G. SANCHEZ

A. TAYLOR

10/4/2023

C11.1

THE RESERVES AT MAGNOLIA DENTON, TEXAS

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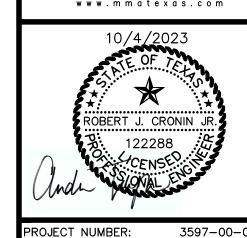
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arlington, texas 76010
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fax: 817-274-8757
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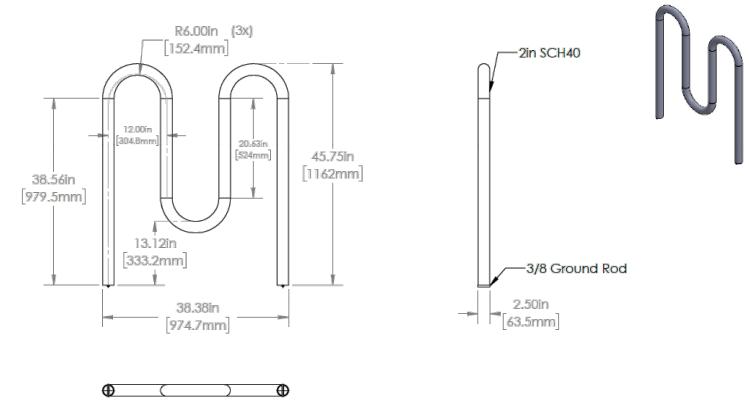


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PROJE	ECT NUME	BER:	3	597-0	00-	02
PROJE	ECT MAN	AGER:	R	. CRO	NIN	
DRAW	N BY:		G	. SAN	CHE	Z
CHEC	KED BY:		Α	. TAYI	LOR	
ISSUE	DATE:		1	0/4/2	023	j
REV.	DATE		DESCR	IP.		B١
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PRIVATE PAVING DETAILS

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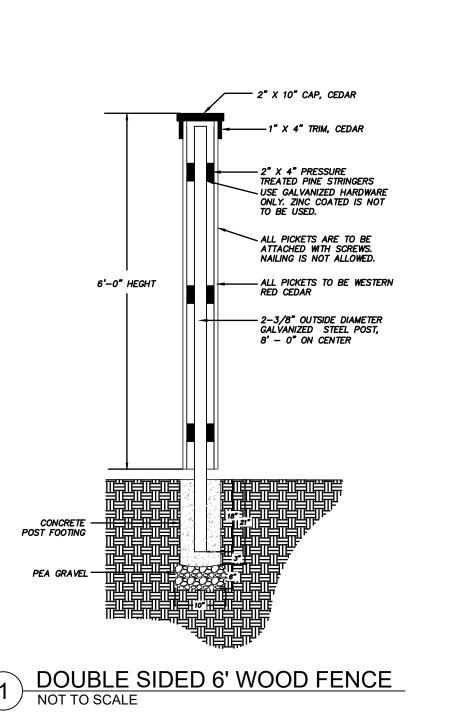
Radius Bike Rack - In-Ground Mount 900br140-1

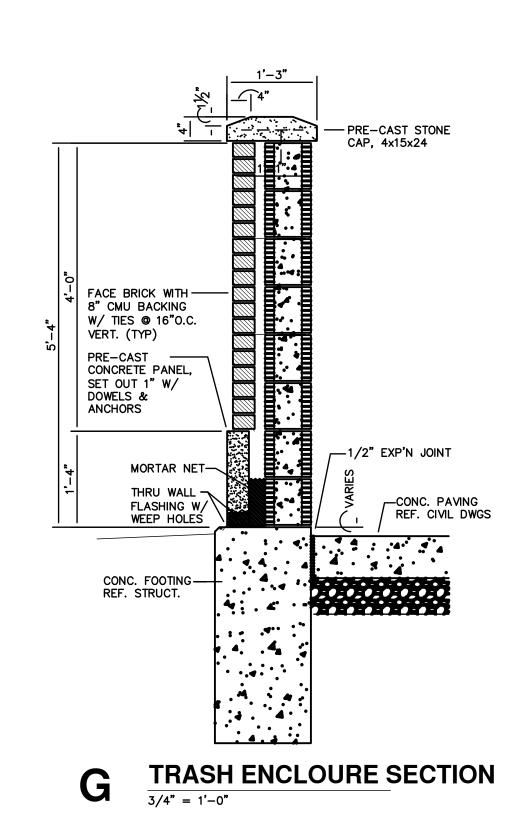


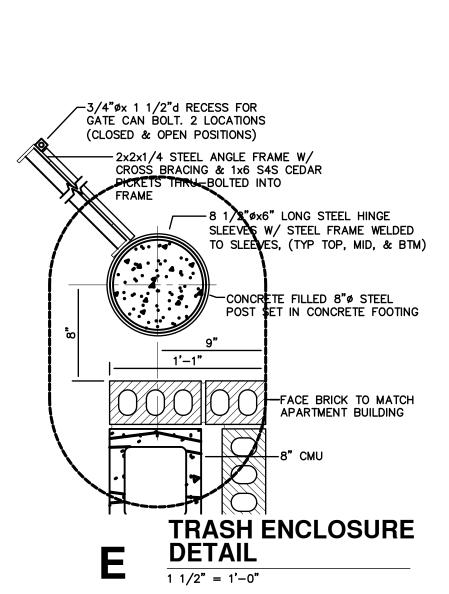
RADIUS BIKE RACK (3 total) (www.parkwarehouse.com)

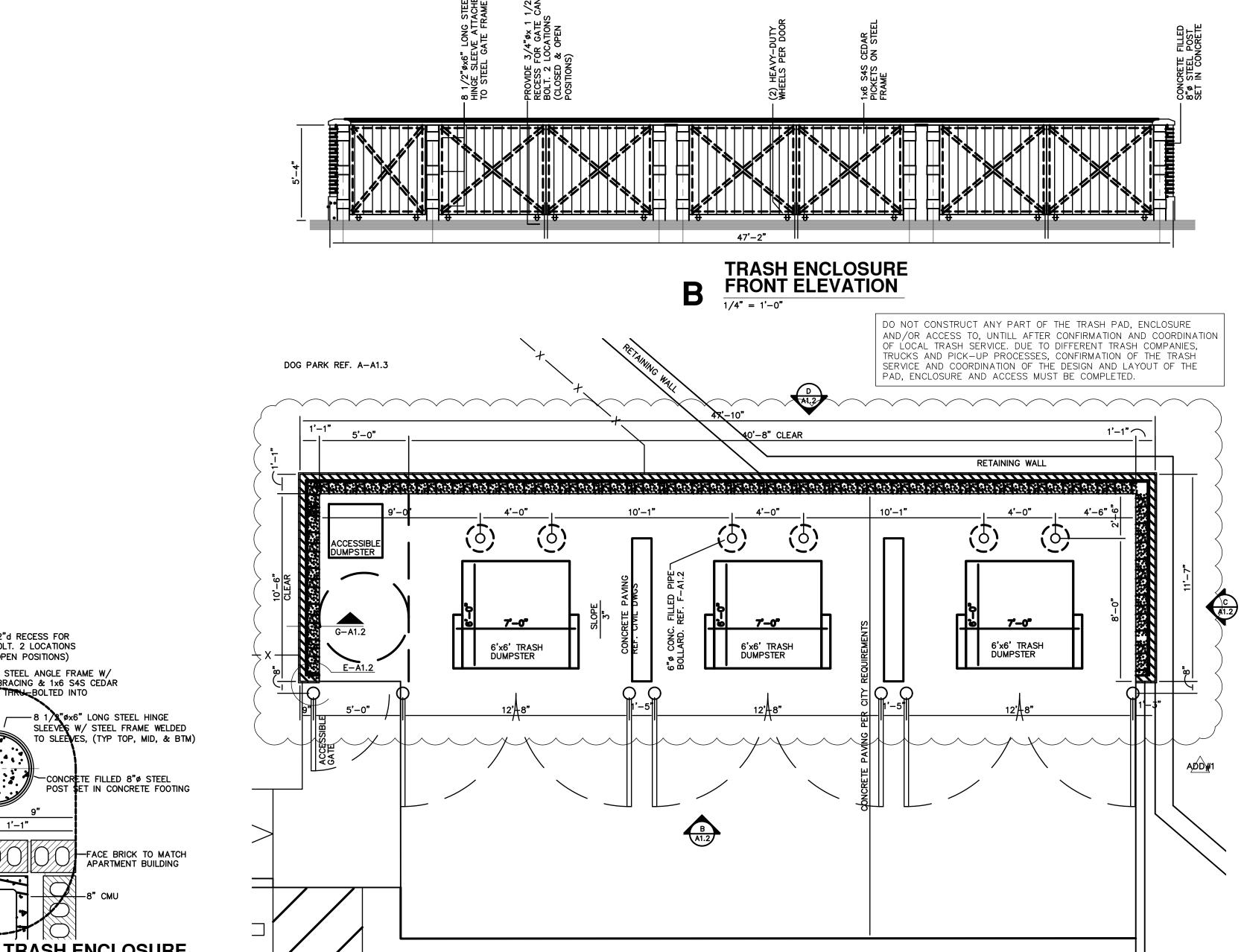
BIKE RACK











TRASH ENCLOSURE REAR ELEVATION

1/4" = 1'-0"

AT MAGNOLIA TEXAS RESERVES DENTON, THE

TRASH ENCLOSURE SIDE ELEVATION

1/4" = 1'-0"

DETAILS

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* ROBERT J. CRONIN JE 122288 PROJECT NUMBER: 3597-00-02 PROJECT MANAGER: R. CRONIN DRAWN BY: B. SHELTON CHECKED BY: A. TAYLOR SSUE DATE: 10/4/2023

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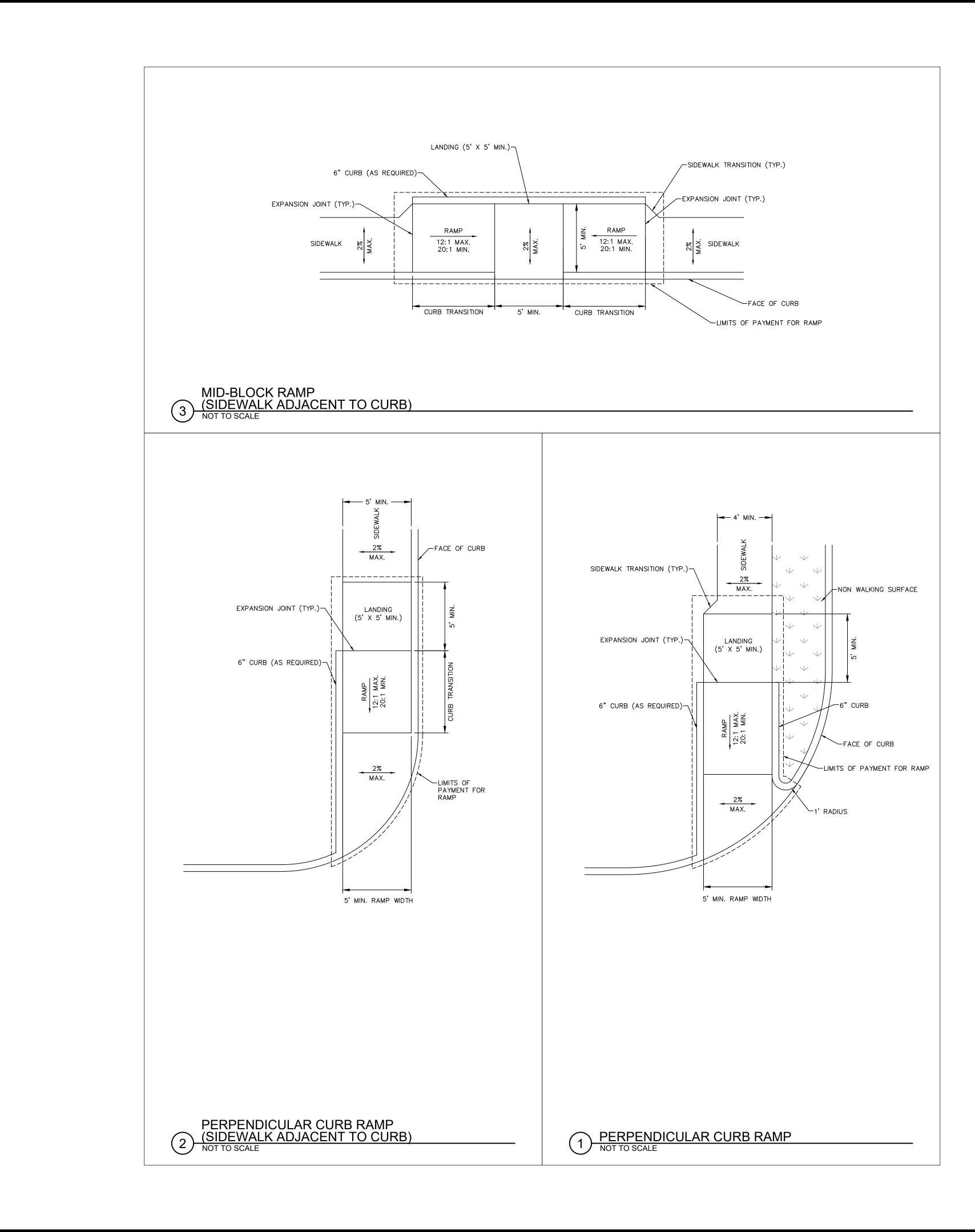
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arlington, texas 76010 8 1 7 - 4 6 9 - 1 6 7 1 fax: 817-274-8757 www.mmatexas.com ROBERT J. CRONIN JE PROJECT NUMBER: 3597-00-02 PROJECT MANAGER: R. CRONIN G. SANCHEZ A. TAYLOR 10/4/2023 DESCRIP. REV. DATE

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PRIVATE RAMP

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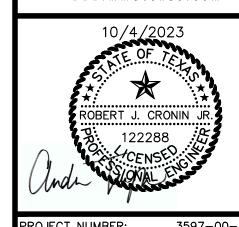
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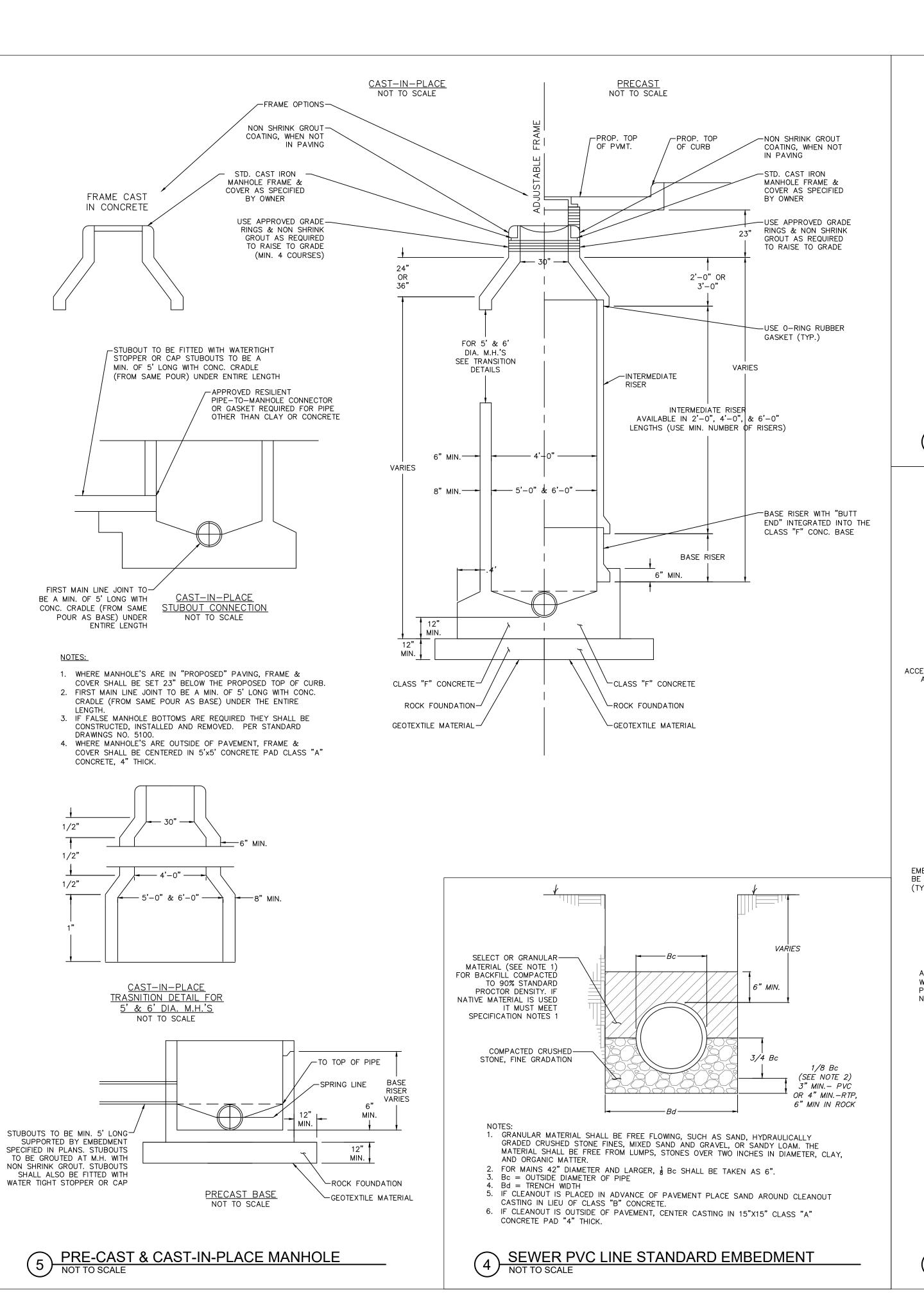
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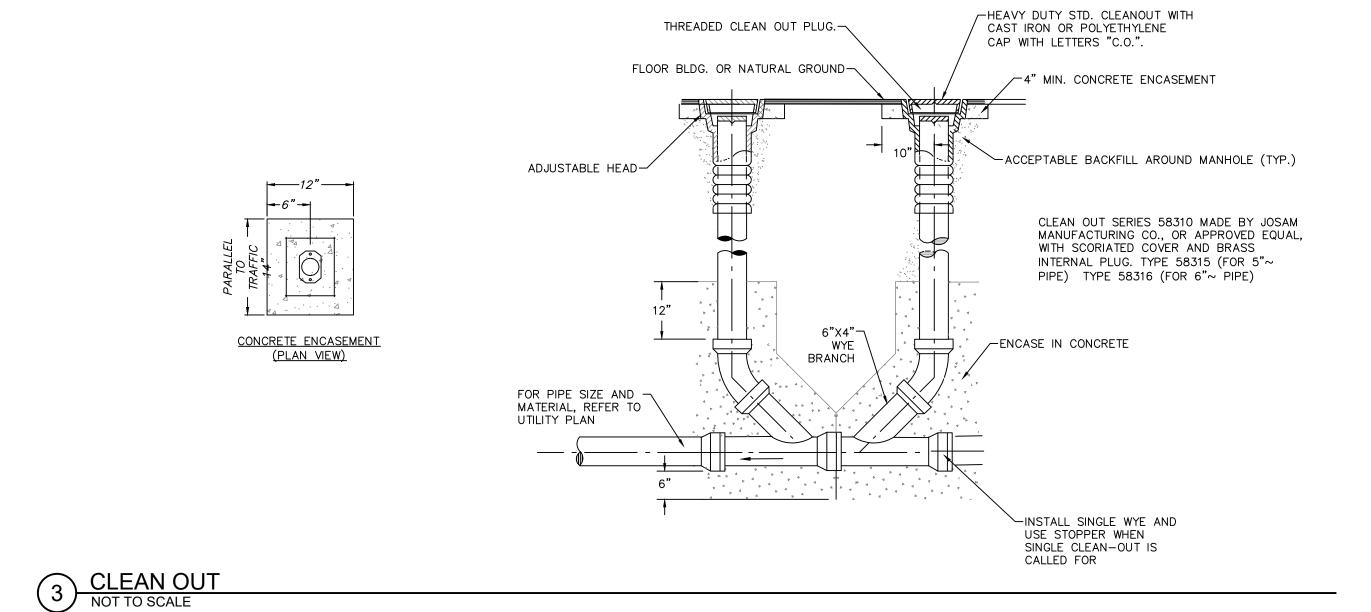
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PROJE	ECT MAN	AGER:	R.	CRONIN	
DRAW	N BY:		G.	SANCHE	Z
CHEC	KED BY:		Α.	TAYLOR	
ISSUE	DATE:		10	/4/2023	3
				_	
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PRIVATE WATER **DETAILS**





MANHOLE -

MANHOLE —

WALL

FLOW

TOP OF-

EDGE OF-

CHANNEL

GROUT

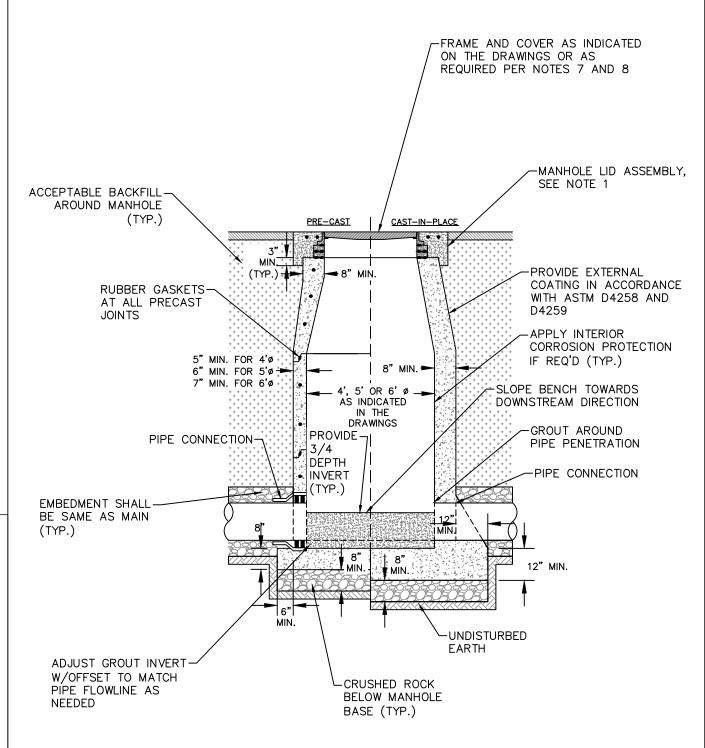
BOTTOM OF SLOPE

AS SHOWN

NOTE:

MANHOLE

REFER TO MANHOLE STANDARD DRAWINGS FOR ADDITIONAL DETAIL ON



- NOTES:
 1. MANHOLE TOP SHALL BE PER MANHOLE LID ASSEMBLY DETAILS AS INDICATED IN THE
- DRAWINGS.

 2. FOR CAST—IN—PLACE MANHOLES, CURE FOR THREE DAYS BEFORE BACKFILLING AROUND STRUCTURE

 3. IF SOIL CONDITIONS OR GROUND WATER PREVENT USE OF COARSE AGGREGATE BASE A
- 2-INCH MUD SLAB MAY BE SUBSTITUTED.

 4. PRE-CAST JOINTS SHALL CONFORM TO ASTM C478.

 5. UNLESS OTHERWISE INDICATED IN THE DRAWINGS, 4'Ø MANHOLES SHALL BE USED FOR 21"

 AND SMALLER SANITARY SEWER LINES AND 5'Ø MANHOLES SHALL BE USED 24" UP TO 36"
- AND SMALLER SANITARY SEWER LINES AND 5'Ø MANHOLES SHALL BE USED 24" UP TO 36" SANITARY SEWER LINES.

 6. HINGED LIDS SHALL BE REQUIRED FOR ALL SANITARY SEWER MANHOLES WITH CONNECTING
- 6. HINGED LIDS SHALL BE REQUIRED FOR ALL SANITARY SEWER MANHOLES WITH CONNECTING LINES OF 24-INCHES AND GREATER.
 7. HINGED LIDS SHALL BE REQUIRED FOR ALL SANITARY SEWER MANHOLES WITH A RIM ELEVATION GREATER THAN 12-INCHES ABOVE NATURAL GROUND.

2 STANDARD MANHOLE

NOT TO SCALE



SECTION A-A

THE RESERVES AT MAGNOLIA DENTON, TEXAS

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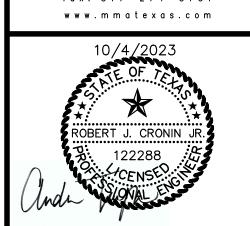
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engineering surveying landscape architecture plans
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arlington, texas 7 6 0 1 0
8 1 7 - 4 6 9 - 1 6 7 1
f a x: 8 1 7 - 2 7 4 - 8 7 5 7

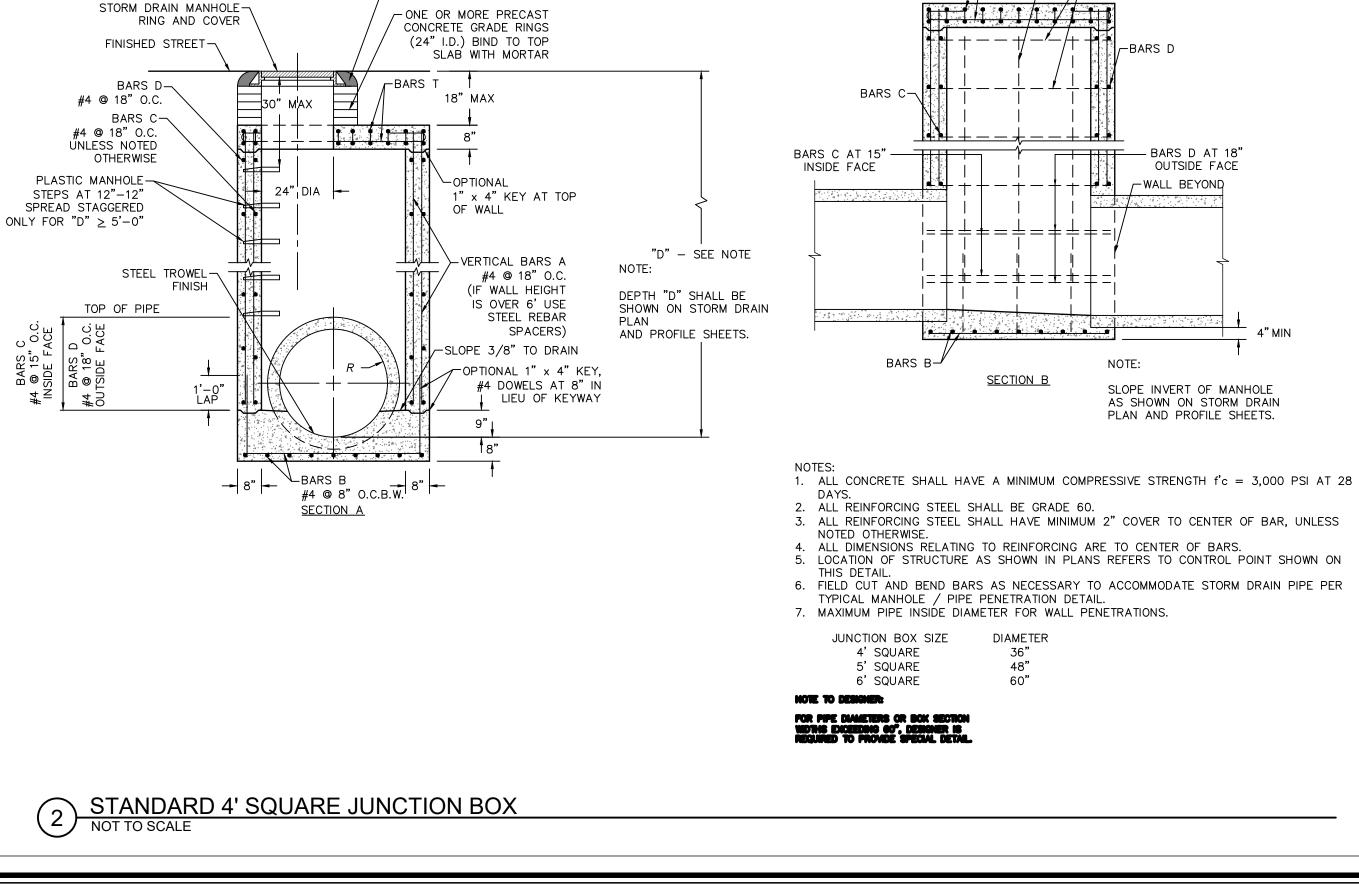


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PROJE	CT MAN	AGER:	R. CRO	R. CRONIN	
DRAW	N BY:		G. SAN	G. SANCHEZ	
CHEC	KED BY:		A. TAYI	_OR	
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PRIVATE SANITARY SEWER DETAILS

C11.6





V−BARS T

_MORTAR COVER

#4 @ 6" O.C.B.W. WITH STD. ACI 180°

HOOK, EACH END

INSIDE FACE

AS SHOWN

CONTROL POINT

PLAN VIEW

INTEGRATED DUCTILE IRON-GRATE TO MATCH BASIN O.D.

VARIABLE INVERT

HEIGHTS AVAILABLE

(ACCORDING TO

PLANS/ TAKE OFF)

VARIOUS TYPES OF INLET & OUTLET ADAPTERS-

AVAILABLE: 4" - 36" FOR CORRUGATED HDPE

(ADS N-12/HANCOR DUAL WALL, ADS/HANCOR

PVC DWV (EX: SCH 40), PVC C900/C905,

(CORRUGATED HDPE SHOWN)

ALL DOME GRATE INLETS TO BE NYOPLAST

OR APPROVED EQUAL.

CORRUGATED & RIBBÉD PVC

WATERTIGHT JOINT -

3 DRAIN BASIN WITH DOME GRATE
NOT TO SCALE

BARS C-

LAP

CORNER DETAIL

SINGLE WALL), N-12 HP, PVC SEWER (EX: SDR 35),

MINIMUM PIPE BURIAL DEPTH PER

PIPE MANUFACTURER

RECOMMENDATION (MIN.

MANUFACTURING REQ. SAME AS

MIN. SUMP)

-ADAPTER ANGLES

VARIABLE 0°-360° ACCORDING TO PLANS

ON 30" BASED ON

MANUFACTURING REQ.)

4" MIN ON 8"-24"

6" MIN ON 30"

THE BACKFILL MATERIAL SHALL BE CRUSHED STONE OR OTHER GRANULAR

AS DEFINED IN ASTM D2321.
BEDDING & BACKFILL FOR SURFACE DRAINAGE INLETS SHALL BE PLACED & COMPACTED UNIFORMLY IN ACCORDANCE

WITH ASTM D2321.

NOTE: VERTICAL REINFORCING

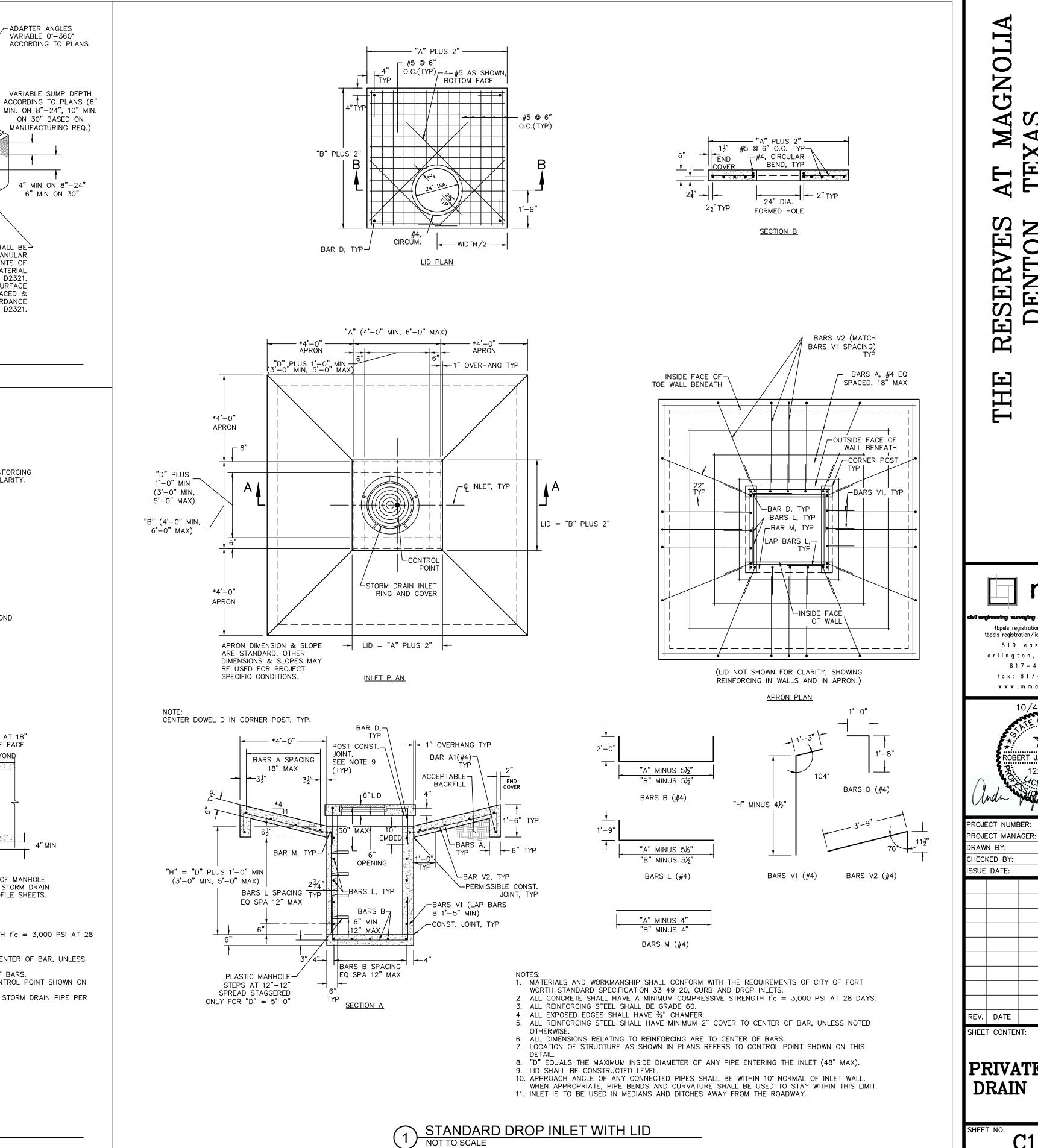
NOT SHOWN FOR CLARITY.

-VERTICAL BARS A, BEYOND

_BARS C AND D,

BEYOND

MATERIAL MEETING THE REQUIREMENTS OF CLASS I, CLASS II, OR CLASS III MATERIAL



OLI Z AT TE ESERVES DENTON,

TORM $\bar{\Omega}$ TE ablatbpels registration number: f - 2759 tbpels registration/license number: 10088000

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ROBERT J. CRONIN JE

122288

SSUE DATE:

3597-00-02

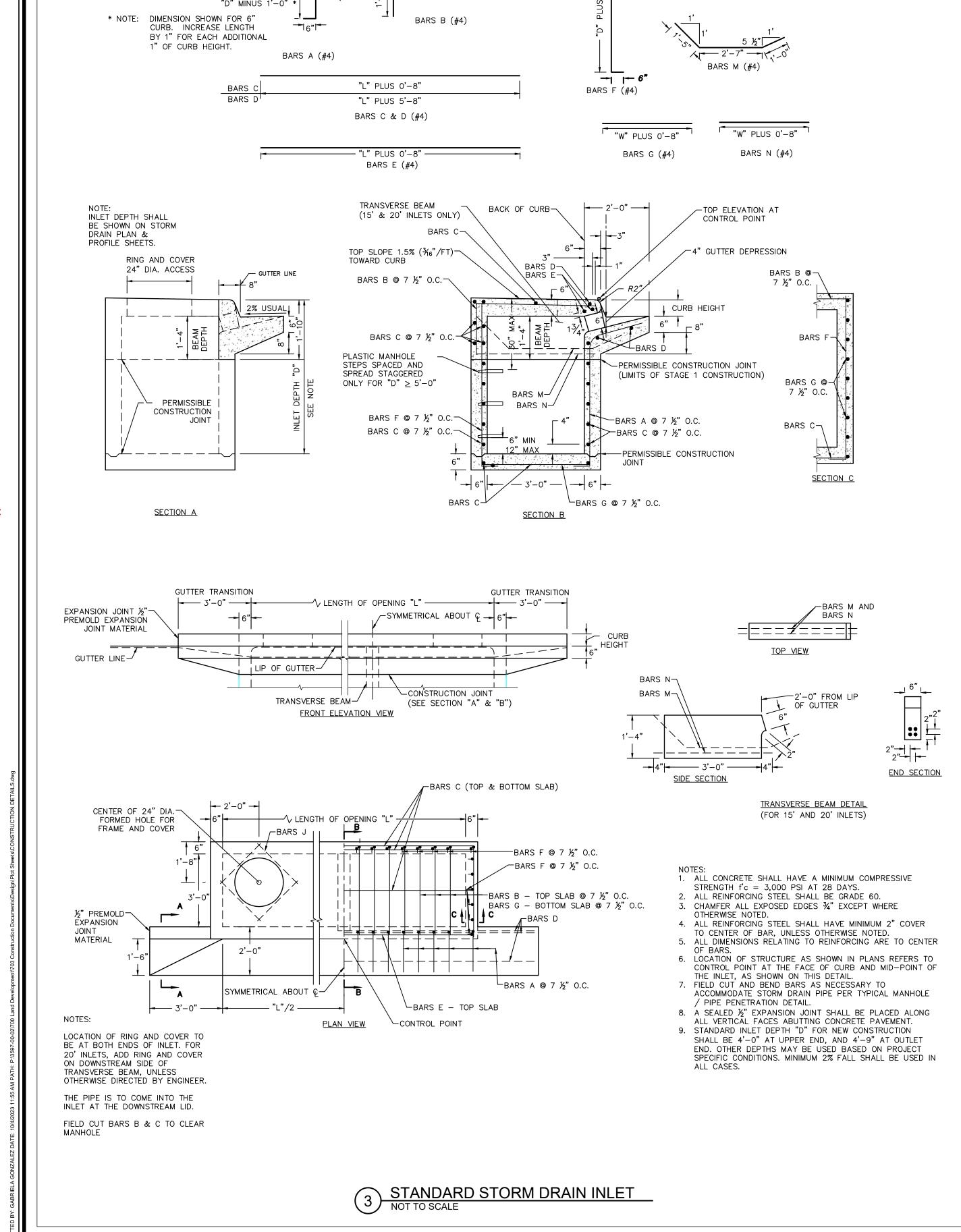
R. CRONIN

G. SANCHEZ

10/4/2023

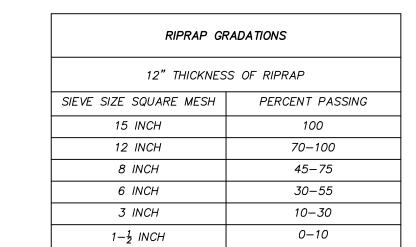
A. TAYLOR

PRIVATE STORM DRAIN DETAILS

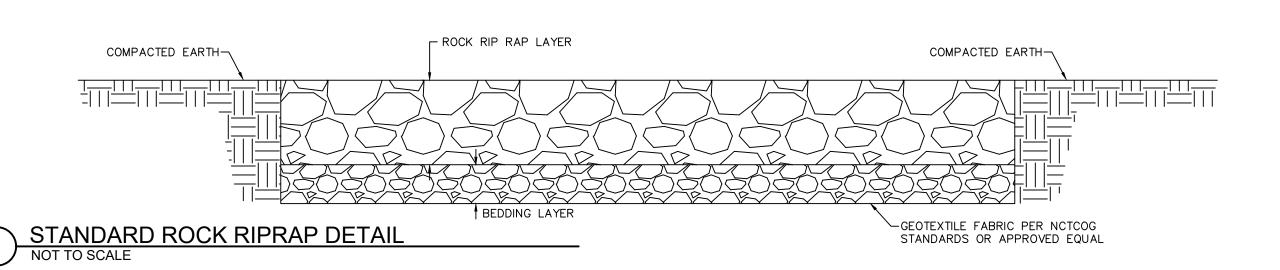


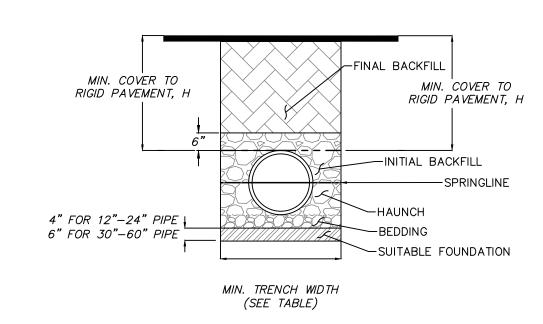
2'-4" ---

BARS J (#4)



BEDDING GRADATIONS		
6" THICKNESS OF RIPRAP		
SIEVE SIZE SQUARE MESH	PERCENT PASSING	
3 INCH	100	
$1-\frac{1}{2}$ INCH	55-100	
₹ INCH	25-60	
₹ INCH	5-30	
NO.4	0-10	





NOTES:

- 1. ALL PIPE SYSTEMS SHALL BE INSTALLED IN ACCORDANCE WITH ASTM D2321, "STANDARD PRACTICE FOR UNDERGROUND INSTALLATION OF THERMOPLASTIC PIPE FOR SEWERS AND OTHER GRAVITY FLOW APPLICATIONS", LATEST ADDITION
- MEASURES SHOULD BE TAKEN TO PREVENT MIGRATION OF NATIVE FINES INTO BACKFILL MATERIAL, WHEN REQUIRED.
 FOUNDATION: WHERE THE TRENCH BOTTOM IS UNSTABLE, THE CONTRACTOR SHALL EXCAVATE TO A DEPTH REQUIRED BY THE ENGINEER AND REPLACE WITH SUITABLE MATERIAL AS SPECIFIED BY THE ENGINEER. AS AN ALTERNATIVE AND AT THE DISCRETION OF THE DESIGN ENGINEER, THE TRENCH BOTTOM MAY BE STABILIZED
- USING A GEOTEXTILE MATERIAL.

 4. BEDDING: SUITABLE MATERIAL SHALL BE CLASS I, II OR III. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR MATERIAL SPECIFICATION TO ENGINEER. UNLESS OTHERWISE NOTED BY THE ENGINEER, MINIMUM BEDDING THICKNESS SHALL BE 4" FOR 4"-24"; 6" FOR 30"-60".
- 5. INITIAL BACKFILL: SUITABLE MATERIAL SHALL BE CLASS I, II OR III IN THE PIPE ZONE EXTENDING NOTE LESS THAN 6" ABOVE CROWN OF PIPE. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR MATERIAL SPECIFICATION TO ENGINEER. MATERIAL SHALL BE INSTALLED AS PECULIFIED IN ASTAL DOZGO.
- SHALL BE INSTALLED AS REQUIRED IN ASTM D2321, LATEST EDITION.

 6. MINIMUM COVER: MINIMUM COVER, H, IN NON-TRAFFIC APPLICATIONS (GRASS OR LANDSCAPE AREAS) IS 12" FROM THE TOP OF PIPE TO GROUND SURFACE. ADDITIONAL COVER MAY BE REQUIRED TO PREVENT FLOATATION. FOR TRAFFIC APPLICATIONS, MINIMUM COVER, H, IS 12" UP TO 48" DIAMETER PIPE AND 24" OF COVER FOR 60" DIAMETER PIPE, MEASURED FROM TOP OF PIPE TO BOTTOM OF FLEXIBLE PAVEMENT OR TO TOP OF RIGID PAVEMENT. FOR TRAFFIC APPLICATIONS WITH LESS THAN FOUR FEET OF COVER, EMBEDMENT OF THE PIPE SHALL BE USING ONLY A CLASS I OR CLASS II BACKFILL.

RECOMMENDED MINIMUM TRENCH WIDTHS

PIPE DIAM.	MIN. TRENCH WIDTH
4"	21"
6"	23"
8"	26"
10"	28"
12"	30"
15"	34"
18"	39"
24"	48"
30"	56"
36"	64"
42"	72"
48"	80'
60"	96"

MAXIMUM RECOMMENDED COVER BASED ON VEHICLE LOADING CONDITIONS

PROP	CLA	SS I	CLA	SS II
	COMPACT ED	DUMPED	95%	90%
4"	34	16	23	16
6"	40	19	27	19
8"	30	14	21	14
10"	34	16	23	16
12"	35	17	24	17
15"	37	18	25	18
18"	32	15	22	15
24"	27	13	19	13
30"	22	11	16	11
36"	26	12	18	12
42"	24	11	17	11
48"	23	11	16	11
60"	26	12	18	12

FILL HEIGHT TABLE GENERATED USING AASHTO SECTION 12, LOAD RESISTANCE FACTOR DESIGN (LRFD) PROCEDURE WITH THE FOLLOWING ASSUMPTIONS:
NO HYDROSTATIC PRESSURE,
UNIT WEIGHT OF SOIL (YS) = 120 PCF

MINIMUM RECOMMENDED COVER BASED ON VEHICLE LOADING CONDITIONS**

	SURFACE LIVE LOADING CONDITION		
PIPE DIAM.	H-25	HEAVY CONSTRUCTION (75T AXLE LOAD)*	
12" - 48"	12"	48"	
60"	24"	60"	

* VEHICLES IN EXCESS OF 75T MAY REQUIRE ADDITIONAL COVER ** SEE BACKFILL REQUIREMENTS IN NOTE 6

STORM HDPE DRAINAGE STANDARD EMBEDMENT

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PROJECT NUMBER: 3597-00-02
PROJECT MANAGER: R. CRONIN
DRAWN BY: G. SANCHEZ
CHECKED BY: A. TAYLOR
ISSUE DATE: 10/4/2023

REV. DATE DESCRIP. BY
SHEET CONTENT:

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ROBERT J. CRONIN JE

122288

519 east border

PRIVATE STORM DRAIN DETAILS

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ISOMETRIC PLAN VIEW

SILT FENCE OVERFLOW STRUCTURE

Figure 3.30 Schematics of Silt Fence

EXCAVATED STONE OUTLET SEDIMENT TRAP SECTION VIEW (B-B) Figure 3.35 Schematics of Excavated Stone Outlet Sediment Trap Stone Outlet Sediment Trap April 2010, Revised 9/2014

Revised 2019

CC-165

FABRIC SPLICES. HOG WIRE WEIR CURB INLET PROTECTION ISOMETRIC VIEW ROCK BAGS @3' O.C. HOG WIRE WEIR CURB INLET PROTECTION CROSS SECTION NOTE: THIS CONTROL WILL DECREASE THE CAPACITY OF THE INLET. IT SHALL ONLY BE USED WHEN AN ENGINEER HAS DETERMINED THERE IS ADEQUATE STORAGE OR POSITIVE OVERFLOW. REFERENCE: NCTCOG STANDARD SPECIFICATIONS (2017), SECTION 202.14

Figure 3.7 Schematics of Hog Wire Weir Curb Inlet Protection (Source: Modified from City of Round Rock Detail E-03)

CONCRETE WASHOUT PLAN VIEW

CONCRETE WASHOUT SECTION VIEW

2. WASHOUT STRUCTURES SHALL BE CLEANED OUT WHEN THE STRUCTURE IS 75% FULL. TEMPORARY CONCRETE WASHOUT FACILITY SHOULD BE MAINTAINED TO PROVIDE ADEQUATE HOLDING CAPACITY.

Figure 4.1 Schematics of Concrete Washout Containment

1. WASHOUT AREA MUST BE CLEARLY MARKED WITH SIGNAGE NOTING THE WASHOUT AREA.

Concrete Waste Management April 2010, Revised 9/2014

CONCRETE WAHSOUT NOTES:

CC-200

Revised 2019

ROBERT J. CRONIN J 122288 PROJECT NUMBER: PROJECT MANAGER: RAWN BY: CHECKED BY: SUE DATE: REV. DATE DESCRIP.

519 east border arlington, texas 76010

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Construction Controls

Revised 2018

Construction Controls

3597-00-02 R. CRONIN G. SANCHEZ A. TAYLOR 10/4/2023

PRIVATE EROSION CONTROL **DETAILS**

CC-94 April 2010, Revised 9/2014

ALONG TOP AND ALONG THE FRONT OF

HOG WIRE WEIR CURB INLET PROTECTION PLAN VIEW

TABLE EC1

NOTES:

1.A SECTION OF FILTER FABRIC SHALL BE REMOVED AS SHOWN ON THIS DETAIL TO PROVIDE A 2" MINIMUM CLEAR OPENING FABRIC MUST BE SECURED TO WIRE BACKING WITH CLIPS OR HOG RINGS AT THIS LOCATION.

2. INSPECTION SHALL BE MADE BY THE CONTRACTOR AND SILT ACCUMULATION MUST BE REMOVED WHEN DEPTH REACHES 2".

3. INLET PROTECTIONS SHALL BE REMOVED AS SOON AS THE SOURCE OF SEDIMENT IS STABILIZED.

Revised 2018 Figure 3.8 Hog Wire Weir Curb Inlet Protection

EXCAVATED STONE OUTLET SEDIMENT TRAP VIEW LOOKING UPSTREAM (A-A)

Construction Controls

Revised 2018

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Construction Controls

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SILT FENCE GENERAL NOTES:

1. DESIGN SHALL SHOW ON THE DRAWINGS THE LOCATIONS WHERE OVERFLOW STRUCTURES SHALL BE INSTALLED OVERFLOW STRUCTURES ARE REQUIRED AT ALL LOW POINTS AND AT A SPACING OF APPROXIMATELY 300 FEET WHERE NO LOW POINT IS APPARENT.

2. DESIGNER SHALL SHOW ON THE DRAWINGS THE LOCATIONS WHERE SILT FENCE IS TO BE TURNED UPSLOPE AT THE ENDS. UPSLOPE LENGTHS SHALL BE A MINIMUM OF 10 FEET.

4. THE TOE OF THE SILT FENCE SHALL BE TRENCHED IN WITH A SPADE OR MECHANICAL TRENCHER, SO THAT THE DOWNSLOPE FACE OF THE TRENCH IS FLAT AND PERPENDICULAR TO THE LINE OF FLOW.

7. INSPECTION SHALL BE AS SPECIFIED IN THE SWPPP, REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED.

10. SEE NCTCOG STANDARD SPECIFICATIONS (2017), SECTION 202.5

Figure 3.31 Silt Fence General Notes

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Construction Controls

Revised 2018

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Stabilized Construction Exit

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LENGTH AS SHOWN ON PLANS

GRADE TO DRAIN AWAY FROM STABILIZATION AND STREET PAVED SURFACE

PROFILE VIEW

PAVED SURFACE -

PLAN VIEW

ENTRACE MUST BE SLOPED SO THAT STORM WATER IS NOT ALLOWED TO LEAVE THE SITE AND ENTER ROADWAYS

Figure 3.32 Schematics of Stabilized Construction Exit

= 5' MIN.

FROM LEAVING SITE

EXISTING GRADE

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Construction Controls

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Stabilized Construction Exit

April 2010, Revised 9/2014

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EDGE OF PLASTIC LINING

10 MIL. MINIMUM PLASTIC LINING

STABILIZED CONSTRUCTION ENTRANCE GENERAL NOTES:

3. STONE SHALL BE 3 TO 5 INCH DIAMETER COURSE AGGREGATE, NO CRUSHED PORTLAND CEMENT CONCRETE ALLOWED.

5. THE WIDTH SHALL BE NO LESS THAN 20' FOR SITES LESS THAN 5 AC, AND 30' FOR SITES GREATER THAN 5 AC, AT ALL POINTS OF INGRESS OR EGRESS.

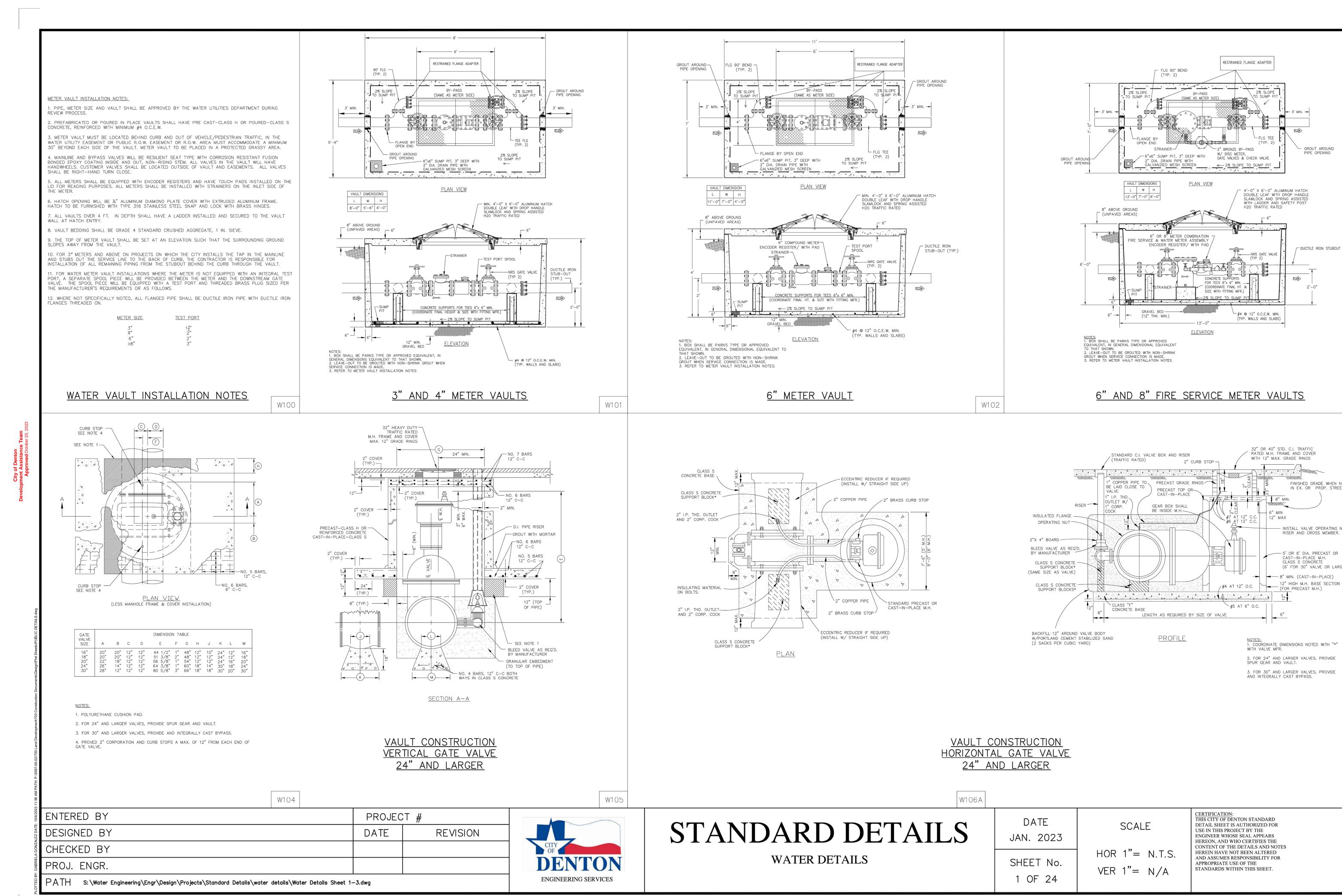
Figure 3.33 Stabilized Construction Entrance General Notes

1. SEE NCTCOG STANDARD SPECIFICATIONS (2017), SECTION 202.11

2. THE THICKNESS SHALL NOT BE LESS THAN 6 INCHES.

4. LENGTH SHALL BE SHOWN ON PLANS, WITH A MINIMUM LENGTH OF 50 FEET.

9. INSPECTION SHALL BE SPECIFIED IN THE SWPPP.



RESTRAINED FLANGE ADAPTER

DOUBLE LEAF WITH DROP HANDLE SLAMLOCK AND SPRING ASSISTED WITH LADDER AND SAFETY POST H20 TRAFFIC RATED

−#4 @ 12" O.C.E.W. MIN.

(TYP. WALLS AND SLABS)

DUCTILE IRON STUBOUT (TYP.)

W103

W106

FLOW

32" OR 40" STD. C.I. TRAFFIC

WITH 12" MAX. GRADE RINGS

FINISHED GRADE WHEN NOT IN EX. OR PROP. STREETS

INSTALL VALVE OPERATING NUT RISER AND CROSS MEMBER.

(6' FOR 30" VALVE OR LARGER)

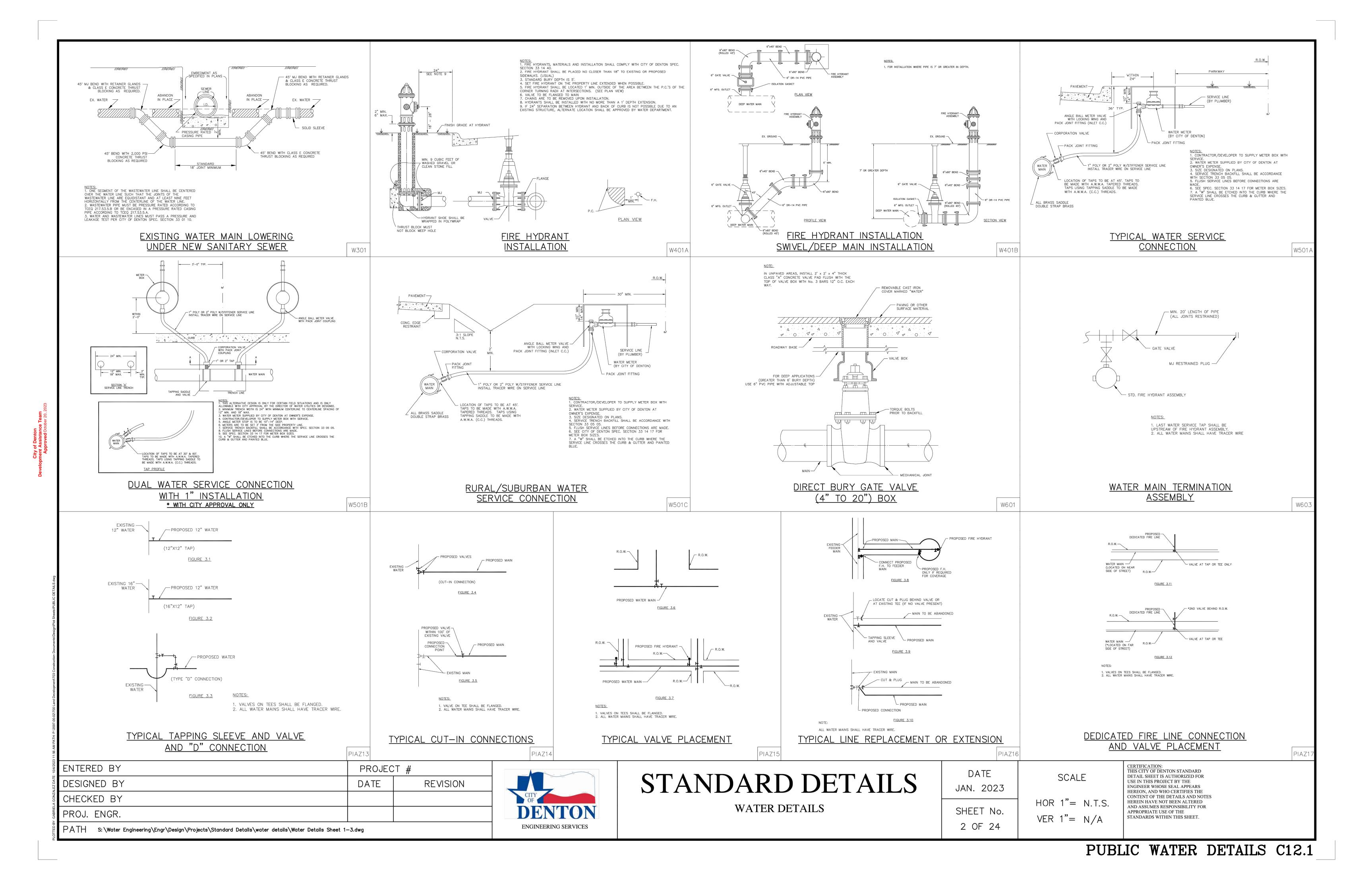
CAST-IN-PLACE M.H. CLASS S CONCRETE

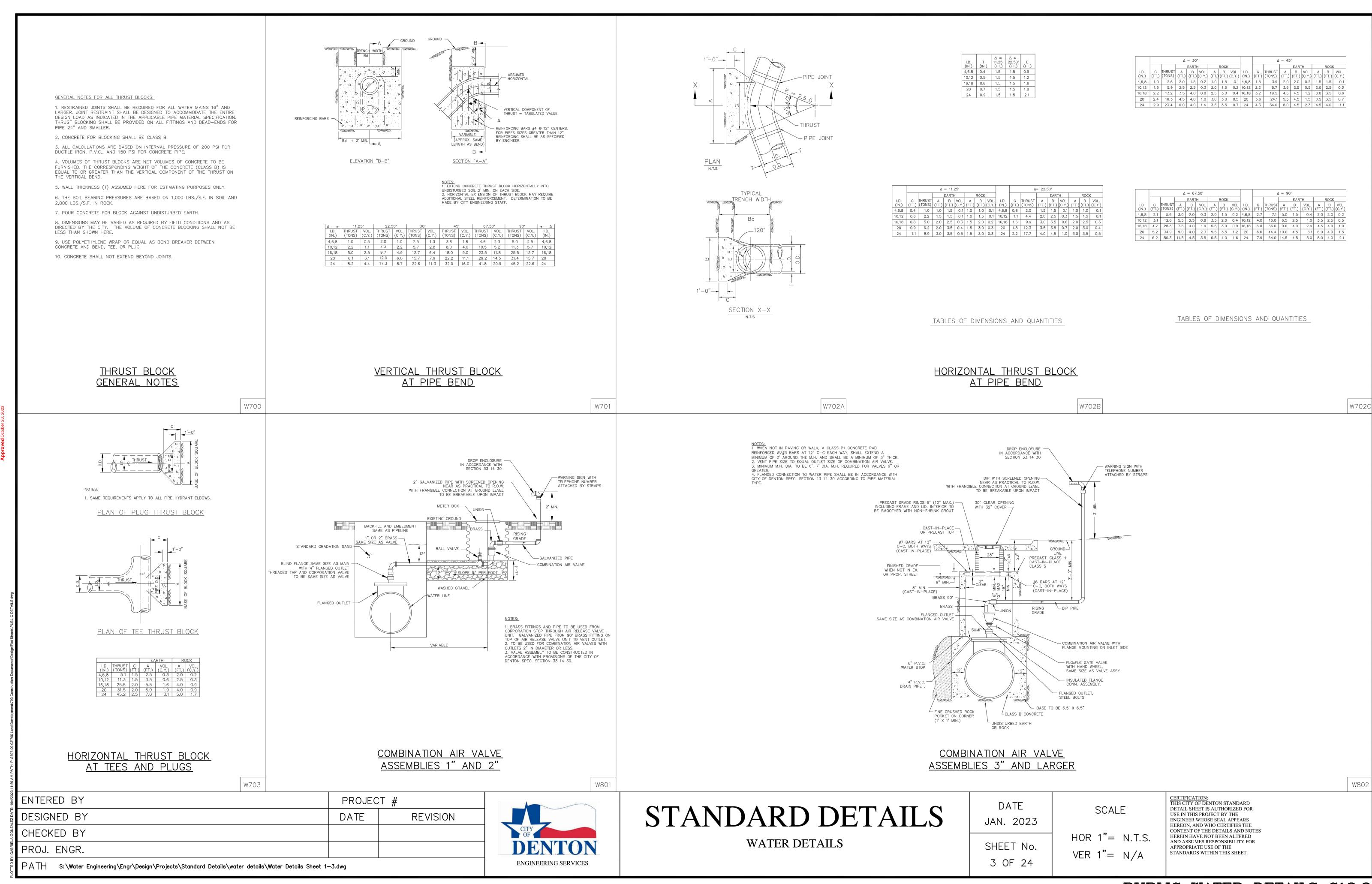
■ 8" MIN. (CAST-IN-PLACE)

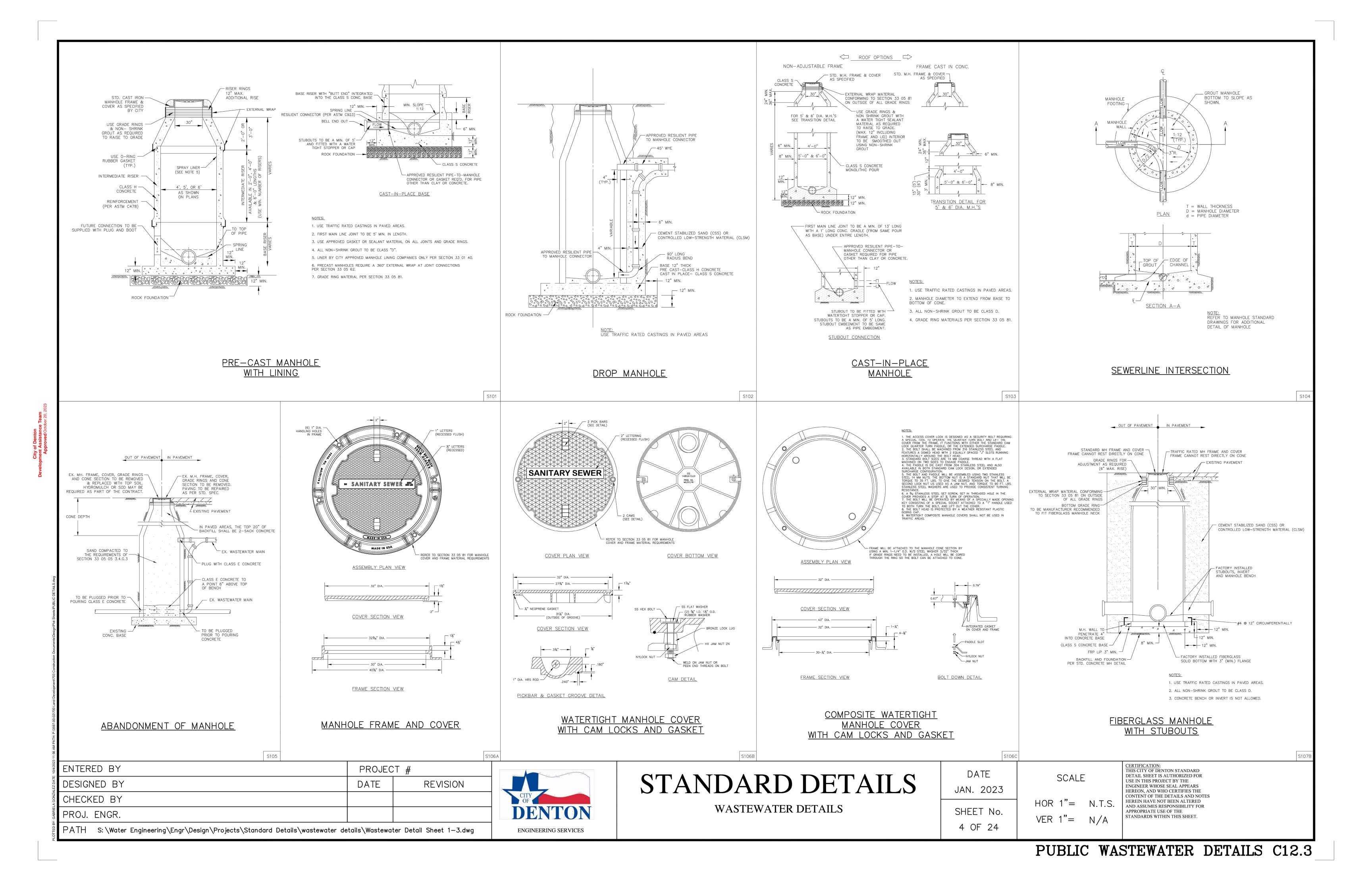
NOTES:
1. COORDINATE DIMENSIONS NOTED WITH "*"

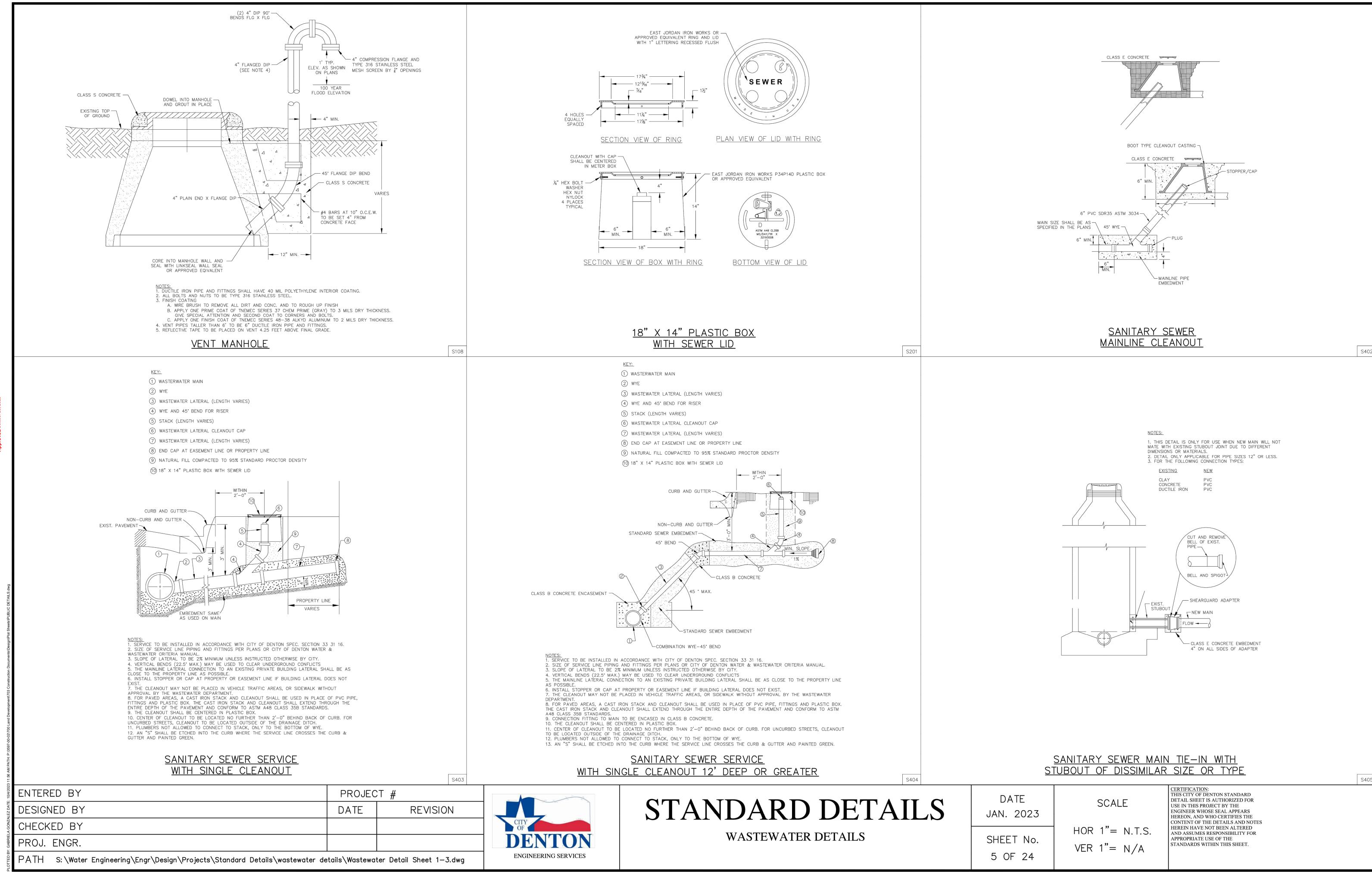
2. FOR 24" AND LARGER VALVES, PROVIDE SPUR GEAR AND VAULT.

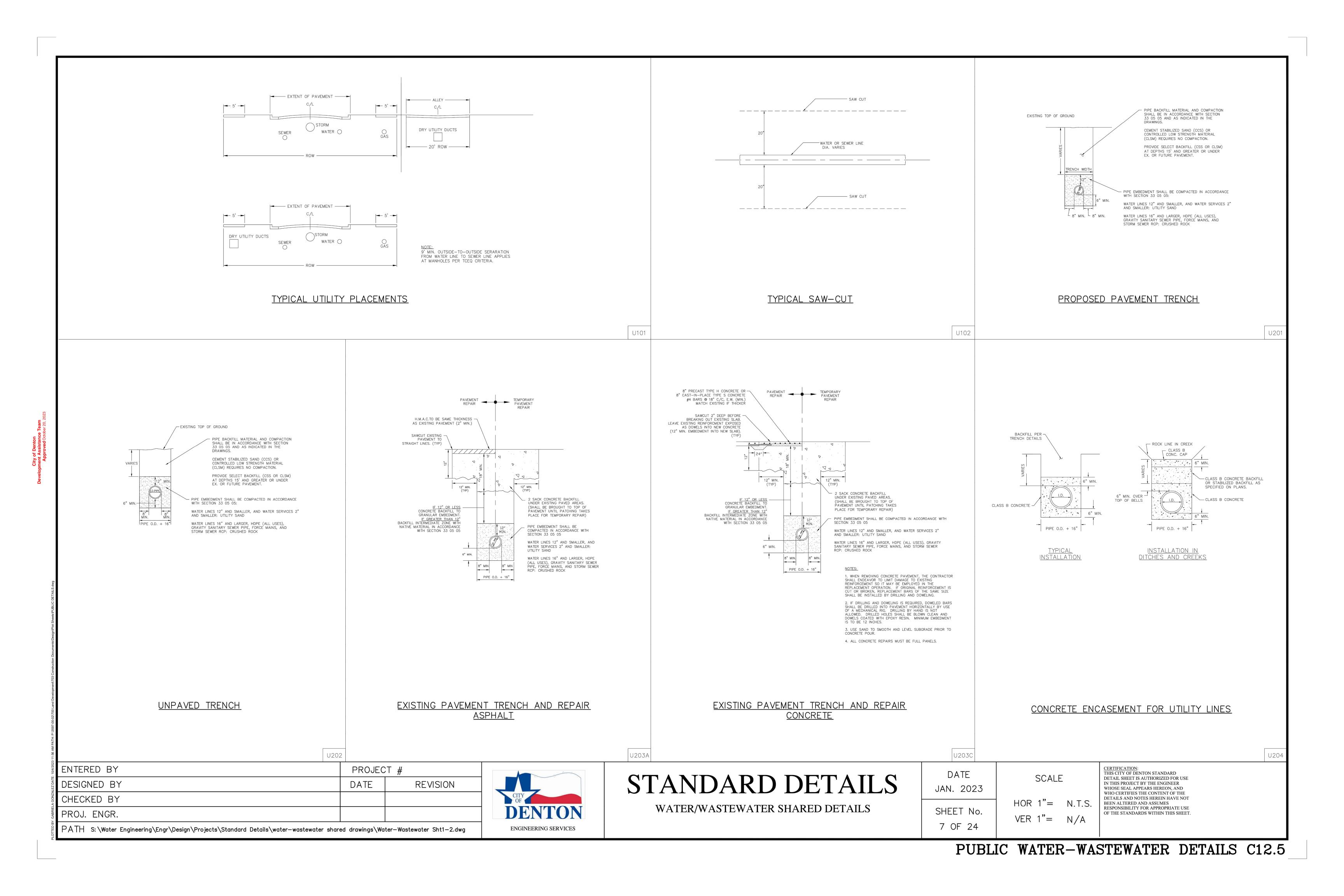
3. FOR 30" AND LARGER VALVES, PROVIDE AND INTEGRALLY CAST BYPASS.

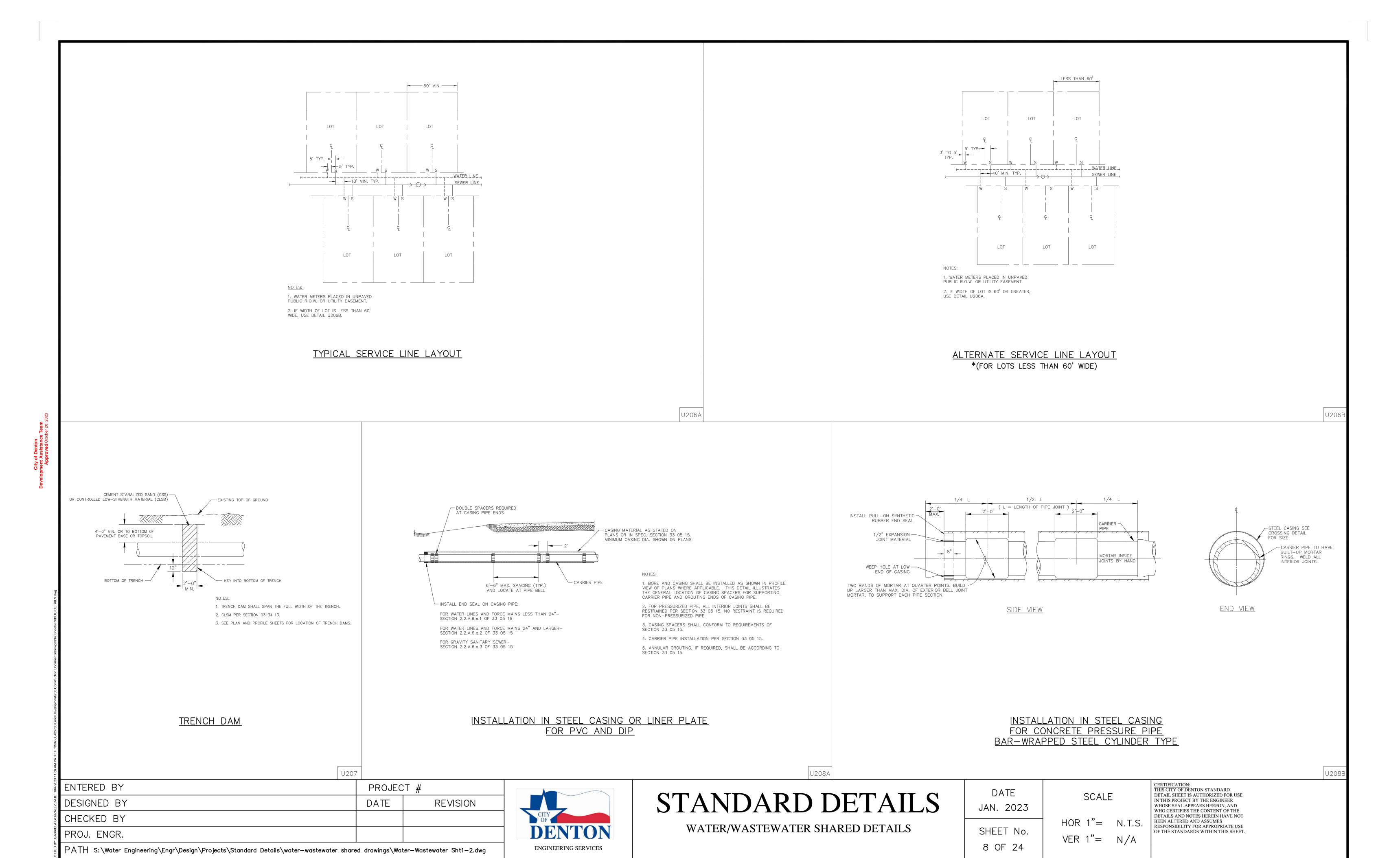


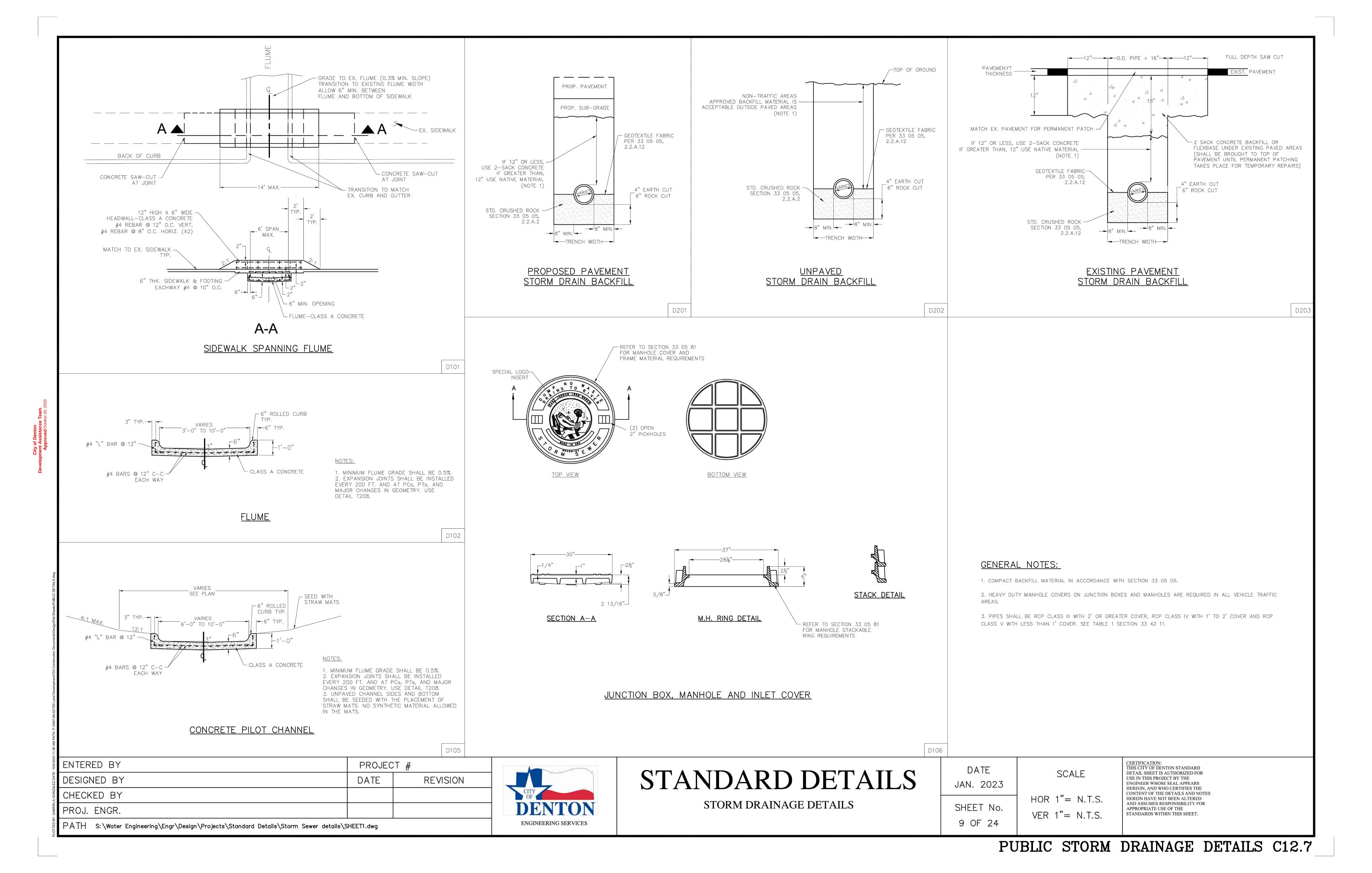


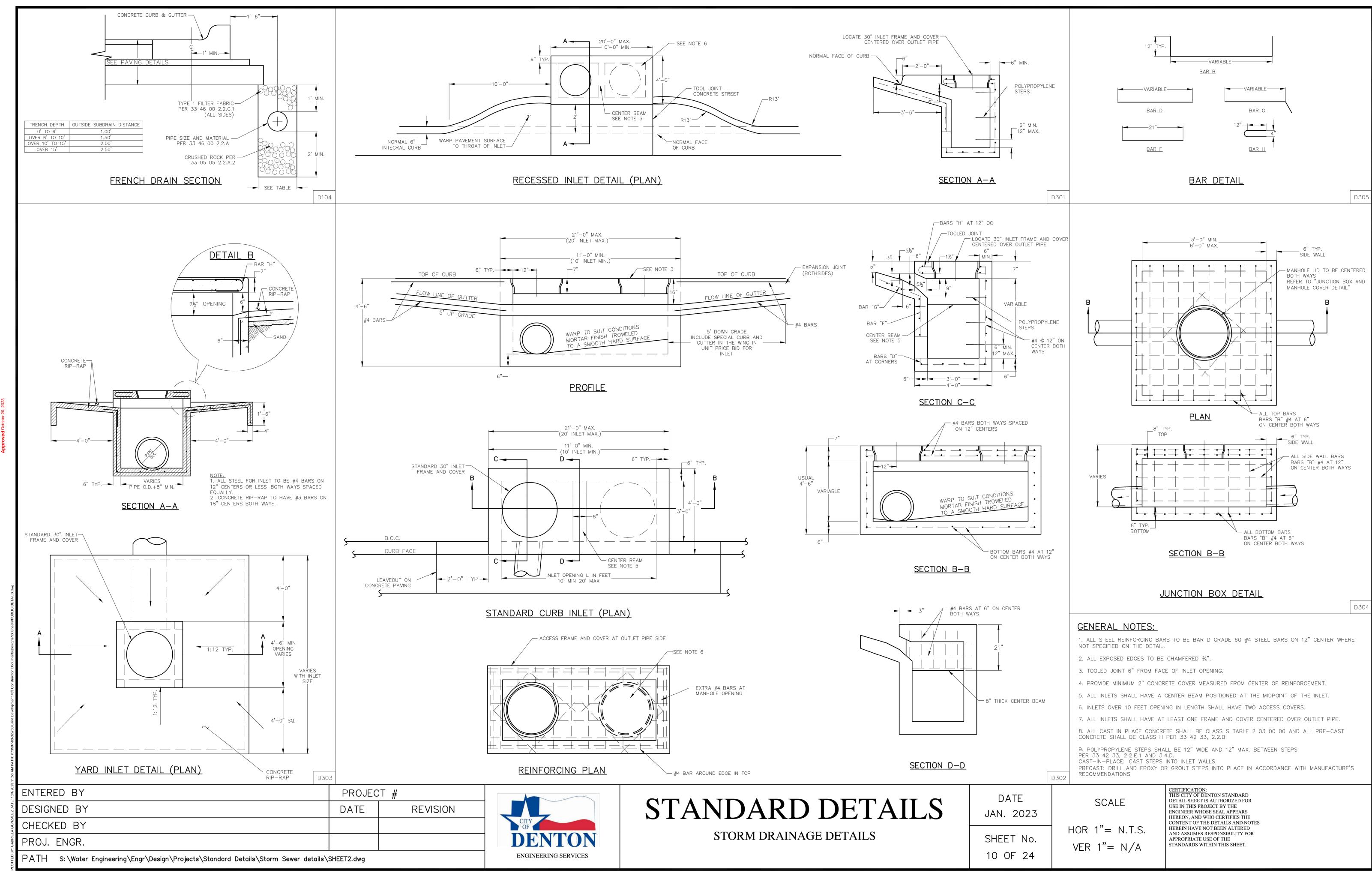


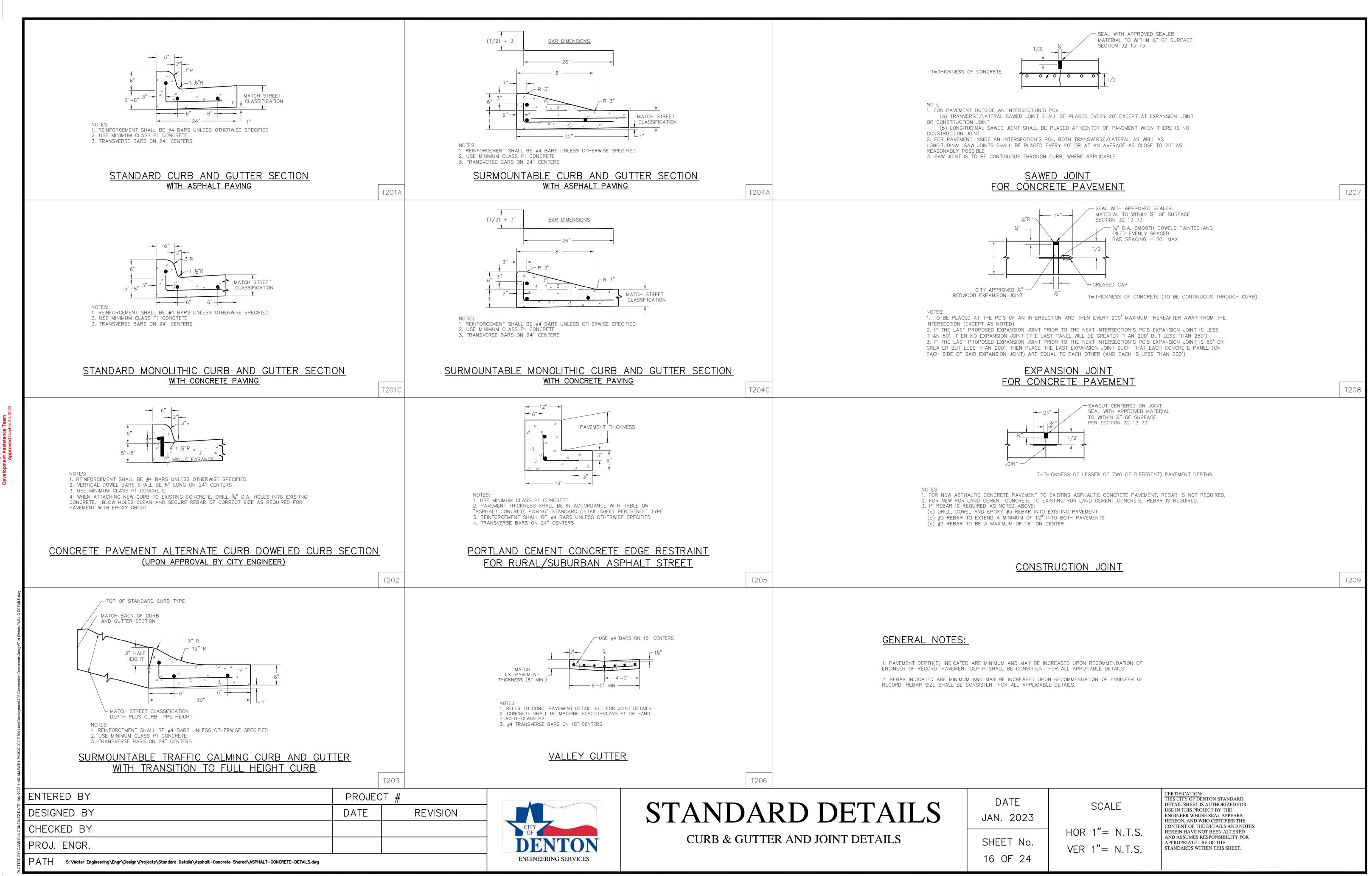












STANDARD DRIVE APPROACH-REINFORCEMENT PLAN EXISTING ASPHALT STREET

WITH CURB & GUTTER NO CURB & GUTTER (RURAL RD. OR ALLEY) #4 BARS 18" ON CENTERS BOTH WAYS ½" REDWOOD — EXPANSION JOINT _____ R.O.W. EXPANSION-JOINT GUTTER -

STANDARD DRIVE APPROACH-REINFORCEMENT PLAN EXISTING CONCRETE STREET

ENTERED BY PROJECT # DESIGNED BY REVISION DATE CHECKED BY PROJ. ENGR.

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STANDARD DETAILS

DRIVE APPROACH DETAILS

JAN.	2023	
SHEE	T No.	

18 OF 24

CERTIFICATION: THIS CITY OF DENTON STANDARD DETAIL SHEET IS AUTHORIZED FOR USE IN THIS PROJECT BY THE ENGINEER WHOSE SEAL APPEARS HEREON, AND WHO CERTIFIES THE CONTENT OF THE DETAILS AND NOTES HEREIN HAVE NOT BEEN ALTERED AND ASSUMES RESPONSIBILITY FOR APPROPRIATE USE OF THE STANDARDS WITHIN THIS SHEET.

STANDARD DRIVE APPROACH-DIMENSION TABLE

DRIVE APPROACH				
USE	WIDTH	RADIUS	MIN. THICKNESS*	
SINGLE FAMILY & DUPLEX RESIDENTIAL	MIN. WIDTH = 12 FEET MAX. WIDTH = 20 FEET	5 FEET	6 INCHES	
MULTI-FAMILY RESIDENTIAL	MIN. WIDTH = 24 FEET MAX. WIDTH = 38 FEET	10 TO 20 FEET	8 INCHES	
COMMERCIAL & INDUSTRIAL	MIN. WIDTH = 30 FEET MAX. WIDTH = 38 FEET	20 TO 25 FEET	8 INCHES	

1. THICKNESS AND REINFORCEMENT TO BE DESIGNED BY ENGINEER OF RECORD, MATERIAL TO BE CLASS P2 REINFORCED CONCRETE WITH A MINIMUM

2. ALL DESIGNS MUST MEET THE MINIMUM APPROVED REQUIREMENTS OF THE STANDARD DRIVE APPROACH DIMENSION TABLE.

GENERAL NOTES:

NOTES:

1. REFER TO SECTION 32 16 00 OF CITY OF DENTON SPECIFICATIONS FOR DRIVEWAY APPROACH REQUIREMENTS.

2. JOINTS ARE TO BE SAWED AS SOON AS THE SETTING OF THE CONCRETE WILL PERMIT WITHOUT SPALLING OR MARKING THE SLAB. AN APPROVED CURING COMPOUND SHALL BE APPLIED TO THE FINISHED SLAB PRIOR TO THE LOSS OF SURFACE MOISTURE AND NO LATER THAN 30 MIN. AFTER FINISHING OPERATIONS.

3. SUBGRADE FOR DRIVE APPROACH SHALL MATCH COMPACTION AND STABILIZATION REQUIREMENTS FOR ADJACENT ROADWAY CLASSIFICATION. 4. DRIVE APPROACH GREATER THAN 12' IN WIDTH SHALL HAVE A TOOLED JOINT PERPENDICULAR TO THE CURB LINE, FROM THE FRONT OF THE GUTTER TO THE BACK OF THE DRIVE APPROACH, AT THE MIDPOINT. DRIVE APPROACHES WITH A WIDTH GREATER THAN 24' SHALL HAVE TWO OR MORE PERPENDICULAR TOOLED JOINTS PLACED AT

THE DIRECTION OF THE ENGINEER OF RECORD.
5. ALL JOINTS TO BE SEALED PER SPEC. SECTION 32 13 73. 6. DRIVE APPROACH SLOPE REQUIREMENTS ARE SPECIFIED IN SECTION 2.3 OF THE TRANSPORTATION CRITERIA MANUAL.

7. REMOVE AND REPLACE ALL CURB AND GUTTER WITHIN THE LIMITS OF CONSTRUCTION. 8. NEW CURB AND GUTTER SHALL BE POURED MONOLITHIC WITH THE DRIVE APPROACH.

9. FOR RESIDENTIAL DRIVE APPROACHES: (a) USE MINIMUM CLASS P2 CONCRETE; MIN. 6" DEPTH.

(b) USE #4 BARS ON 18" CENTERS BOTH WAYS.

10. FOR ALL OTHER DRIVE APPROACHES, INCLUDING APARTMENTS.

(a) USE MINIMUM CLASS P2 CONCRETE; MIN. 8" DEPTH. (b) USE #4 BARS ON 18" CENTERS, BOTH WAYS.

11. DRIVE APPROACH CONSTRUCTION WHICH EXTENDS PAST THE DRIVE APPROACH RADIUS SHALL HAVE A TRANSVERSE TOOLED JOINT AT THE RADIUS POINT AND AN EXPANSION JOINT AT THE POINT OF CONNECTION TO THE EXISTING OR PROPOSED CURB & GUTTER.

12. PAVEMENT DEPTHS INDICATED ARE MINIMUM AND MAYBE INCREASED UPON RECOMMENDATION OF ENGINEER OF RECORD-PAVEMENT DEPTH SHALL BE CONSISTENT FOR 13. BARS INDICATED ARE MINIMUM AND MAY BE INCREASED UPON RECOMMENDATION OF ENGINEER OF RECORD-REBAR SIZE SHALL BE CONSISTENT FOR ALL APPLICABLE

14. SEE DETAILS P500, P501, AND P502 FOR TIEING PROPOSED OR EXISTING SIDEWALK TO DRIVEWAY APPROACH.

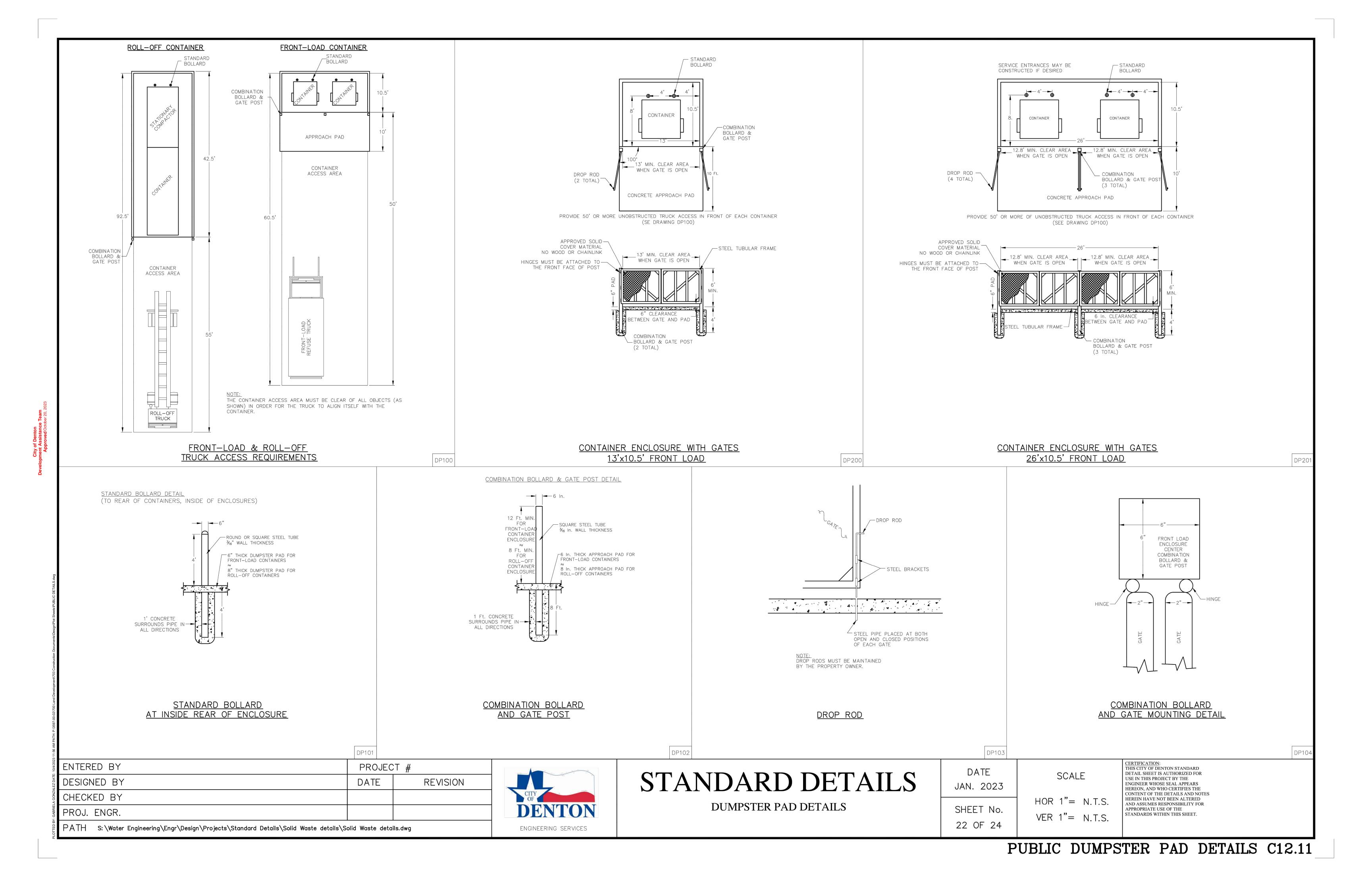
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T306A

DATE

HOR 1"= N.T.S. VER 1"= N.T.S.

SCALE



Drawing #	Subject	Section I: Item #
1010	RESERVED	N/A
1020A	Silt Fence	201.5. pages 201-1 to 201-11
1020B	Silt Fence	201.5. pages 201-1 to 201-11
	General Notes	
1030A	Interceptor Swale	201.6. pages 201-1 to 201-11
1030B	Interceptor Swale	201.6. pages 201-1 to 201-11
1040A	Diversion Dike	201.7. pages 201-1 to 201-11
1040B	Diversion Dike	201.7. pages 201-1 to 201-11
1050A	Triangular Sediment Filter Dike	201.8. pages 201-1 to 201-11
1050B	Triangular Sediment Filter Dike	201.8. pages 201-1 to 201-11
1060A	Rock Check Dam	201.9. pages 201-1 to 201-11
1060B	Rock Check Dam	201.9. pages 201-1 to 201-11
1070A	Stabilized Construction Entrance	201.11. pages 201-1 to 201-11
1070B	Stabilized Construction Entrance	201.11. pages 201-1 to 201-11
1080A	Sand Bag Check Dam	201.10. pages 201-1 to 201-11
1080B	Sand Bag Check Dam	201.10. pages 201-1 to 201-11
1090	Stone Outlet	201.12. pages 201-1 to 201-11
	Sediment Trap	
1100	Pipe Outlet	N/A
1110	Sediment Basin	201 12 201 1 +- 201 11
1110	Pipe Slope Drain	201.13. pages 201-1 to 201-11
1120	Inlet Protection	201.14. pages 201-1 to 201-11
	Filter Barrier	
1130	Inlet Protection-Drop	201.14. pages 201-1 to 201-11
	Block and Gravel	
1140	Inlet Protection-Curb	201.14. pages 201-1 to 201-11
	Block and Gravel	
1150	Inlet Protection	201.14. pages 201-1 to 201-11
	Excavated Impoundment	
1060A	Erosion Control Blankets	201.15. pages 201-1 to 201-11
1060B	Erosion Control Blankets	201.15. pages 201-1 to 201-11

NOTES:

- 1. The City is adopting use of the NCTCOG Erosion and Sedimentation Standard Detail Drawings referenced in the table above. The drawings can be found in the 4th Edition of the NCTCOG specifications, October 2004.
- 2. Modifications to the above referenced drawings may be considered for individual projects upon submittal by a registered Professional Engineer in the State of Texas and supporting documentation as to why the modification is being requested.

ENTERED BY	PROJECT #			
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PROJ. ENGR.				
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STANDARD DETAILS

EROSION & SEDIMENTATION CONTROL DETAILS

DATE JAN. 2023

SHEET No.
24 OF 24

HOR 1"= N.T.S.

VER 1"= N.T.S.

SCALE

CERTIFICATION:
THIS CITY OF DENTON STANDARD
DETAIL SHEET IS AUTHORIZED FOR
USE IN THIS PROJECT BY THE
ENGINEER WHOSE SEAL APPEARS
HEREON, AND WHO CERTIFIES THE
CONTENT OF THE DETAILS AND NOTES
HEREIN HAVE NOT BEEN ALTERED
AND ASSUMES RESPONSIBILITY FOR
APPROPRIATE USE OF THE
STANDARDS WITHIN THIS SHEET.

PUBLIC EROSION CONTROL DETAILS C12.12