

GENERAL STRUCTURAL NOTES

GENERAL

- 1. DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH PROVISIONS OF THE 2021 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC)
2. ELEVATIONS (XXX'-XX") SHOWN ON PLANS ARE TO TOP OF CONCRETE, STEEL, OR WOOD DECK U.N.O.
3. ALL CONTRACTORS AND ANY SUB-CONTRACTORS SHALL VERIFY AND COORDINATE ALL DIMENSIONS AND DETAILS AS SHOWN ON STRUCTURAL DRAWINGS WITH ARCHITECTURAL DRAWINGS.
4. ALL CONTRACTORS AND ANY SUB-CONTRACTORS SHALL CONSULT ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR VERIFICATION OF LOCATION AND DIMENSIONS OF CURBS, PADS, INSERTS, SLEEVES, DRIPS, REGLETS, REVEALS, FINISHES, DEPRESSIONS, DOOR CLOSERS, AND OTHER PROJECT REQUIREMENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
5. SIZE AND LOCATION OF ALL ROOF, FLOOR, AND WALL OPENINGS TO BE VERIFIED WITH MECHANICAL AND ELECTRICAL DRAWINGS AND CONTRACTORS.
6. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR THE ACTS, ERRORS, OR OMISSIONS OF THE CONTRACTOR OR ANY SUB-CONTRACTOR, OR ANY OF THE CONTRACTOR OR SUBCONTRACTORS AGENTS OR EMPLOYEES, OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK.
7. THE ARCHITECT, CONTRACTOR, OWNER, AND END-USER OF THE STRUCTURE SHOULD EXPECT TO SEE SOME DEGREE OF RANDOM CRACKING IN THE SLAB-ON-GRADE.
8. MECHANICAL UNITS AND EQUIPMENT SUPPORTED BY ROOF AND ELEVATED FLOOR STRUCTURE ARE SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER, AND MUST BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR VERIFICATION OF UNIT SIZE, WEIGHT, AND LOCATION.
9. THE STRUCTURAL DRAWINGS HEREIN REPRESENT THE FINISHED STRUCTURE.
10. CONSTRUCTION DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION.
11. ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERRECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE, AND ERECTION IN ACCORDANCE WITH THE SUPPLIERS INSTRUCTIONS AND REQUIREMENTS.
12. CONTRACTOR AND SUB-CONTRACTORS SHALL THOROUGHLY REVIEW ALL DRAWINGS AND SPECIFICATIONS PRIOR TO SUBMITTING BIDS.
13. ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE CONSTRUCTION DRAWINGS AND/OR SPECIFICATION AND/OR EXISTING CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE WORK.
14. CONTRACTOR SHALL REVIEW, STAMP, SIGN, AND DATE ALL SHOP DRAWINGS PRIOR TO FORWARDING TO THE ARCHITECT/ENGINEER.
15. THE CONTRACTOR SHALL COORDINATE WITH ALL TRADES ALL DEPRESSIONS, DIMENSIONS, ELEVATIONS, SLEEVES, CHASES, HANGERS, OPENING, INSERTS, ANCHORS, EQUIPMENT SUPPORTS, AND DETAILS WITH THE ENTIRE CONTRACT DOCUMENT PACKAGE.
16. THESE DRAWINGS INCLUDE SPECIFIED COMPONENTS AND PRODUCTS, I.E. EPOXY, METAL DECK.
17. THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR TO PERFORM INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF THE IBC.

- 1. DESIGN ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF HAS BEEN ASSUMED. ALL EXTERIOR FOOTINGS TO BE 3'-0" BELOW FINISH GRADE UNO.
2. UNLESS NOTED OTHERWISE; CENTER COLUMN FOOTINGS ON COLUMN CENTERLINES, CENTER WALL FOOTINGS ON FOUNDATION WALLS.
3. SLAB ON GRADE SHALL BE UNDERLAIN BY VAPOR BARRIER AND 6 INCHES MINIMUM OF CRUSHED ROCK OR CONCRETE.
4. BACK FILL AROUND THE EXTERIOR FOUNDATION WALLS WITH A FREE DRAINING GRANULAR MATERIAL TO THE ELEVATION OF THE ROUGH GRADE.
5. CONTRACTOR TO KEEP EXCAVATIONS DRY AND PROTECTED FROM FROST AT ALL TIMES DURING THE FOUNDATION CONSTRUCTION.

CAST-IN-PLACE CONCRETE

- 1. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS
FOOTINGS 3500 PSI MAX. W/C RATIO OF 0.50
INTERIOR SLABS ON GRADE 4000 PSI MAX. W/C RATIO OF 0.45
SLABS OVER STEEL DECK 3500 PSI MAX. W/C RATIO OF 0.45
EXPOSED CONCRETE SLABS AND GARAGE SLABS 4000 PSI MAX. W/C RATIO OF 0.45
FOUNDATION WALLS, WALLS, COLUMNS AND BEAMS 4000 PSI MAX. W/C RATIO OF 0.45
2. EXTERIOR EXPOSED CONCRETE SHALL HAVE 4 TO 6% ENTRAINED AIR.
3. AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33.
4. NO ALUMINUM SHALL BE PLACED IN THE CONCRETE.
5. CONSTRUCTION TO BE IN ACCORDANCE WITH ACI 318-05 (R-05), "CHAPTER 3 FOR STANDARDS FOR TESTS & MATERIALS, CHAPTERS 4, 5, 6 & 7 FOR CONSTRUCTION REQUIREMENTS".
6. PIPE OR ELECTRICAL CONDUIT EMBEDDED IN CONCRETE SHALL NOT BE LARGER IN OUTSIDE DIAMETER AT ITS WIDEST (OR FITTING) THAN 1/3 THE THICKNESS OF THE SLAB OR WALL.
7. LOCATION OF ALL CONSTRUCTION AND CONTROL JOINTS SHALL BE LOCATED AND DETAILED ON SHOP DRAWINGS AND ARE SUBJECT TO ENGINEERS APPROVAL.
8. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND DIMENSION OF CONCRETE REVEALS, NOTCHES, REGLETS, DRIPS, PADS, CURBS, CHAMFERS BLOCKOUTS AT DOORWAYS, AND ALL OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS.
9. CONCRETE CAST AGAINST SOIL
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER (#6 OR GREATER) 2"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER (#5 OR LESS) 1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER (SLAB, WALL, JOIST) 3/4"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER (BEAM, COLUMN) 1 1/2"
SLAB ON GRADE 1 1/2"
10. SECURELY TIE ALL REINFORCING IN PLACE WITH DOUBLE ANNEALED 16-GAUGE IRON WIRE OR APPROVED CLIPS PRIOR TO CONCRETE OR GROUT PLACEMENT.
11. SUBMIT SHOP DRAWINGS OF REINFORCING STEEL FOR REVIEW BY THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION.

REINFORCING STEEL

- 1. TRUSS TYPE MASONRY JOINT REINFORCEMENT: W1.7 (9 GAGE), ASTM A1064, FY=70,000 PSI
2. WELDED WIRE REINFORCING (WWR): ASTM A82 AND A185
3. DEFORMED BARS (REBAR): ASTM A615, GRADE 40 FOR #3; GRADE 60 FOR #4 AND LARGER; ASTM A706 FOR WELDED CONDITIONS.
4. LAP SPLICES:
MASONRY: 48-BAR DIAMETERS AT CELLS WITH SINGLE BAR (HORIZ. AND VERT.)
64-BAR DIAMETERS AT CELLS WITH TWO BARS (HORIZ. AND VERT.)
CONCRETE: CLASS "B" LAP SPLICE, TYPICAL UNLESS NOTED OTHERWISE.
WELDED WIRE FABRIC: WIRE SPACING "2"
5. CONCRETE COVER FOR CAST-IN-PLACE AND NON-PRE-STRESSED CONCRETE SHALL BE AS SPECIFIED BELOW U.N.O. ON THESE DRAWINGS:
CONCRETE CAST AGAINST SOIL 3"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER (#6 OR GREATER) 2"
FORMED CONCRETE EXPOSED TO EARTH OR WEATHER (#5 OR LESS) 1 1/2"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER (SLAB, WALL, JOIST) 3/4"
CONCRETE NOT EXPOSED TO EARTH OR WEATHER (BEAM, COLUMN) 1 1/2"
SLAB ON GRADE 1 1/2"
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7. SUBMIT SHOP DRAWINGS OF REINFORCING STEEL FOR REVIEW BY THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION.

TYPICAL CMU WALL REINFORCING

- 1. UNLESS NOTED OTHERWISE ON THESE DRAWINGS, REINFORCE CMU WALLS AS FOLLOWS:
8" CMU WALLS - INTERIOR
#5 VERT @ 48" O.C. CENTER OF WALL
(2) #5 VERT AT EACH CORNER OF WALL
TRUSS TYPE HORIZ JOINT REINFORCEMENT @ 16" O.C.
SOLID GROUT AT REINFORCED CELLS ONLY UNO
PROVIDE (2) TYPICAL VERT FULL HEIGHT JAMB STEEL EACH SIDE OF OPENINGS
PROVIDE (2) #4 HORIZ BOND BEAM AT FLOOR AND ROOF LEVELS AND TOP OF WALL

DESIGN LOADS

Table with 2 columns: Load Description, Value. Includes Roof Live Load (20 PSF), Floor Live Load (40 PSF), Ground Snow Load (5 PSF), Roof Snow Load (5 PSF), Occupancy Category (II), Basic Wind Speed (105 M.P.H.), Wind Components and Cladding (39 PSF), Seismic Design Category (A).

EXISTING CONSTRUCTION

- 1. FIELD VERIFY GRADES, SIZES, LOCATIONS AND CONDITIONS OF ALL ITEMS ON PLANS AND DETAILS BEFORE STARTING WORK.
2. EXISTING STRUCTURE TO REMAIN IS SHOWN SCREENED (LIGHT). EXISTING STRUCTURE TO BE REMOVED IS NOT SHOWN.
3. ALL EXISTING CONSTRUCTION AFFECTED BY DEMOLITION SHALL BE SHORED UNTIL NEW CONSTRUCTION SUPPORT MEMBERS ARE IN PLACE.

MASONRY

- 1. LIGHT WEIGHT CONCRETE MASONRY UNITS WITH NET AREA MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS.
2. FILL ALL CELLS WITH REINFORCING WITH GROUT IN LIFTS NOT EXCEEDING 4'-0" IN HEIGHT.
3. CONSTRUCTION SHALL PROVIDE BRACING FOR MASONRY WALLS, AS REQUIRED, UNTIL CONNECTION TO FLOOR AND/OR ROOF DIAPHRAGMS ARE COMPLETED.
4. STRENGTH OF MASONRY ASSEMBLY SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH SECTION 2105.2.2.1 OF THE 2006 IBC.
5. PROVIDE HORIZONTAL TRUSS-TYPE REINFORCING AT 16" ON CENTER MAXIMUM UNO.
6. NON-BEARING INTERIOR PARTITIONS SHALL STOP 1" BELOW STRUCTURAL SLABS OR STEEL FRAMING U.N.O.
7. WHERE BOND BEAMS INTERSECT AT CORNERS AT DIFFERENT ELEVATIONS, RUN EACH BOND BEAM AROUND CORNER FOR TWO BLOCK LENGTHS MINIMUM.
8. WHERE BOND BEAMS INTERSECT PARALLEL AT DIFFERENT ELEVATIONS, LAP BOND BEAMS FOUR BLOCK LENGTHS MINIMUM.
9. PROVIDE CORNER AND INTERSECTION BARS IN ALL BOND BEAMS.
10. CONTROL AND EXPANSION JOINTS SHALL BE PROVIDED IN MASONRY WALLS AT 30' MAXIMUM PER TYPICAL MASONRY DETAILS.
11. PROVIDE (2) #4 VERTICAL EACH SIDE OF ALL OPENINGS IN MASONRY WALLS UNO.
12. PROVIDE (2) #4 VERTICAL AT ALL WALL CORNERS, ENDS AND INTERSECTIONS UNO.
13. PROVIDE BOND BEAM WITH (2) #4 CONTINUOUS BENEATH ALL SLAB AND BEAM BEARINGS UNO.
14. PROVIDE 1/2" AIR GAP AROUND SIDES, TOP AND END OF WOOD STRUCTURAL MEMBERS BEARING ON MASONRY.

STEEL JOISTS

- 1. STEEL JOISTS BEARING CONNECTIONS SHALL BE BY WELDED UNO.
2. SUSPENSION OF ANY MISCELLANEOUS ITEMS FROM THE JOISTS SHALL BE ONLY AT TOP AND BOTTOM CHORD PANEL POINTS UNLESS SPECIFICALLY DETAILED OTHERWISE.
3. JOIST FABRICATOR SHALL PROVIDE JOIST BRIDGING PER SJI RECOMMENDATIONS.
4. UNLESS NOTED OTHERWISE: ALL ROOF JOISTS TO BE DESIGNED FOR A 150 LB ADD-LOAD AND 150 LB BEND-CHECK ALL FLOOR JOISTS TO BE DESIGNED FOR A 300 LB ADD-LOAD AND 300 LB BEND-CHECK
5. JOIST MANUFACTURER MAY NOT DESIGN JOISTS FOR LESS THAN LOADS SPECIFIED IN THE SJI CAPACITY TABLES FOR JOIST DESIGNATIONS SHOWN ON PLANS.

STRUCTURAL AND MISCELLANEOUS STEEL

- 1. STEEL CONSTRUCTION MANUAL, 14TH EDITION MATERIAL SPECIFICATIONS U.N.O.
WIDE FLANGE AND S SHAPES ASTM A992, FY=50KSI
CHANNELS, ANGLES, PLATES AND BARS ASTM A36, FY=36KSI
HOLLOW STRUCTURAL SHAPES (HSS) ASTM A500 GR. B, FY=46KSI
PIPE ASTM A53, GR. B, FY=35KSI
STRUCTURAL BOLTS (U.N.O.) ASTM A325
MACHINE BOLTS (WHERE NOTED) ASTM A307
ANCHOR BOLTS AND RODS AND THREADED RODS ASTM F1554 GRADE 36KSI
HIGH STRENGTH ANCHOR BOLTS AND RODS (AS NOTED) ASTM F1554 GRADE 108KSI
HEADED OR THREADED STUD ANCHORS (H.S.A. OR T.S.A.) ASTM A108-69T
DEFORMED BAR ANCHORS (D.B.A.) ASTM A496 OR ASTM A706
WELDING ELECTRODES E70XX
NON-SHRINK GROUT (7,000 PSI) ASTM C1107, GR. A
POWDER ACTUATED FASTENER (PAF OR PDF) HILTI XU (0.157" DIA)
EXPANSION BOLTS (CONCRETE) HILTI KWIK BOLT T3
EXPANSION BOLTS (MASONRY) HILTI KWIK BOLT 3
EPOXY ADHESIVE - CONCRETE HILTI HIT-HY 200
EPOXY ADHESIVE - MASONRY HILTI HIT-HY 70 W/ SCREEN TUBE

WOOD

- 1. ALL WOOD BEARING ON CONCRETE OR MASONRY, IF LESS THAN 4'-0" ABOVE FINISH GRADE, SHALL BE PRESSURE TREATED.
2. PLYWOOD SHEATHING SHALL CONFORM TO THE CURRENT EDITION OF THE U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARD (OR 2 (DOC PS 1 OR 2) OR THE APA PANEL DESIGN SPECIFICATION (PDS) AND SHALL BE INSTALLED IN STAGGERED PATTERN.
3. BOLT HOLES IN WOOD SHALL BE DRILLED 1/16" MAXIMUM OVERSIZE.
4. PROVIDE SOLID BLOCKING AT MID-HEIGHT OF ALL WALLS U.N.O.
5. PROVIDE SOLID BLOCKING BETWEEN JOISTS AT ALL SUPPORTS.
6. WOOD FRAMING AND CONSTRUCTION SHALL CONFORM TO THE CURRENT EDITION OF THE NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION.
7. ALL COLUMNS SHOWN ON STRUCTURAL DRAWINGS SHALL BE CONTINUOUS U.N.O.
8. SET ALL JOISTS WITH CROWN UP.
9. TYPICAL FRAMING ANCHORS SHALL BE "SIMPSON STRONG TIE" OR APPROVED EQUIVALENT AS INDICATED ON DRAWINGS.
10. CONNECTORS, ANCHORS, AND FASTENERS ATTACHED TO PRESSURE TREATED WOOD TREATED WITH ACO-C OR ACO-D SHALL BE GALVANIZED AND SHALL MEET ONE OF THE FOLLOWING SPECIFICATIONS:
11. IF WOOD TREATMENT IS OTHER THAN LISTED ABOVE, CONTACT THE TREATMENT SUPPLIER FOR CORROSION PROTECTION REQUIREMENTS AND SUBMIT TO ARCHITECT/ENGINEER FOR APPROVAL.



LUMBER MINIMUM PROPERTIES SCHEDULE

Table with 8 columns: Species/Product, Grade, Fb (PSI), Ft (PSI), Fv (PSI), Fc (PSI), E (PSI). Rows include Spruce-Pine-Fir (North), MicroLam LVL, Parallam PSL, and Timberstrand LSL.

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10. CONNECTORS, ANCHORS, AND FASTENERS ATTACHED TO PRESSURE TREATED WOOD TREATED WITH ACO-C OR ACO-D SHALL BE GALVANIZED AND SHALL MEET ONE OF THE FOLLOWING SPECIFICATIONS:
11. IF WOOD TREATMENT IS OTHER THAN LISTED ABOVE, CONTACT THE TREATMENT SUPPLIER FOR CORROSION PROTECTION REQUIREMENTS AND SUBMIT TO ARCHITECT/ENGINEER FOR APPROVAL.

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11. IF WOOD TREATMENT IS OTHER THAN LISTED ABOVE, CONTACT THE TREATMENT SUPPLIER FOR CORROSION PROTECTION REQUIREMENTS AND SUBMIT TO ARCHITECT/ENGINEER FOR APPROVAL.

ABBREVIATIONS

Table mapping abbreviations to full names: @ AT, # NUMBER, AB ANCHOR BOLT, ADD'L ADDITIONAL, AEES ARCHITECTURALLY EXPOSED STRUCTURAL STEEL, ALTERNATE ARCHITECTURAL, ARCH ARCHITECTURAL, ATTM ATTACHMENT, BLDG BUILDING, BLKG BLOCKING, BOT BOTTOM, BSMIT BASEMENT, BTWN BETWEEN, CFS COLD FORMED STEEL, CJ CONTROL OR CONSTRUCTION JOINT, C/J COMPLETE JOINT PENETRATION WELD, CL CENTERLINE, CLR CLEAR, CMU CONCRETE MASONRY UNITS, COL COLUMN, CONC CONCRETE, CONN CONNECTION, CONT CONTINUOUS, COORD COORDINATE, DBA DEFORMED BAR ANCHOR, DET DETAIL, DIA DIAMETER, DIM DIMENSION, DIR DIRECTION, DF-L DOUGLAS FIR-LARCH, EA EACH, EA FACE, EMBED EMBEDDED, EN EDGE NAILING, EOR ENGINEER OF RECORD, EQ EQUAL, EXIST EXISTING, EXIST EXISTING, EXP EXPANSION, FDN FOUNDATION, FIN FINISH, FLR FLOOR, FN FIELD NAILING, FRP FIBER-REINFORCED POLYMER, FTG FOOTING, FV FIELD VERIFY, GA GAUGE, GR GRADE, HK HOOK, HORIZ HORIZONTAL, HS HIGH STRENGTH, HSA HEADED STUD ANCHOR, HSS HOLLOW STRUCTURAL SHAPE, IBC INTERNATIONAL BUILDING CODE, ID INSIDE DIAMETER, INFO INFORMATION, LBS POUNDS, LG LONG, LLH LONG LEG HORIZONTAL, LLV LONG LEG VERTICAL, LSL LAMINATED STRAND LUMBER, LVL LAMINATED VENEER LUMBER, MAX MAXIMUM, MECH MECHANICAL, MFR MANUFACTURER, MIN MINIMUM, MTL METAL, NIC NOT IN CONTRACT, NS NON-SHRINK, OC ON CENTER, OD OUTSIDE DIAMETER, OPP OPPOSITE, OSB ORIENTED STRAND BOARD, PAF POWDER ACTUATED FASTENER, PEMB PRE-ENGINEERED METAL BUILDING, PLATE, PLF POUNDS PER LINEAR FOOT, PSF POUNDS PER SQUARE FOOT, PSI POUNDS PER SQUARE INCH, PSL PARALLEL STRAND LUMBER, PT POINT, QTY QUANTITY, REINF REINFORCING, REM REMAINDER, REQ'D REQUIRED, RTU ROOF TOP UNIT, SCHD SCHEDULE, SIML SIMILAR, SIV SHORT LEG VERTICAL, SOG SLAB-ON-GRADE, SPF SPRUCE-PINE-FIR, SQ SQUARE, STD STANDARD, T&B TOP AND BOTTOM, THK THICK, TOF TOP OF FOOTING, TOM TOP OF MASONRY, TOS TOP OF STEEL, TOW TOP OF WALL, TSA THREADED STUD ANCHOR, TYP TYPICAL, VERT VERTICAL, UNO UNLESS NOTED OTHERWISE, W/W WITH, WF WIDE FLANGE, WWR WELDED WIRE REINFORCING

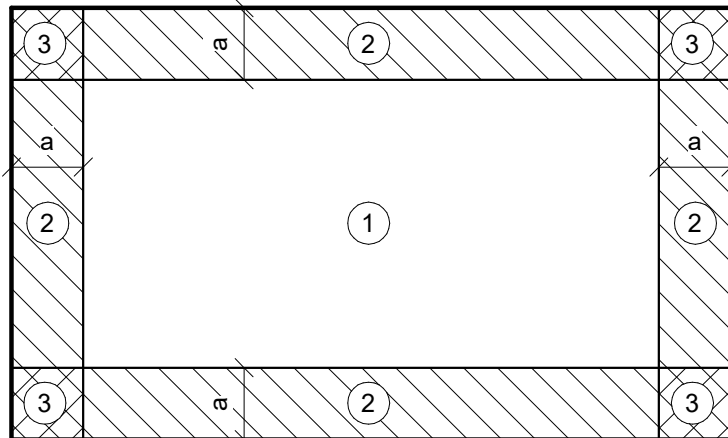
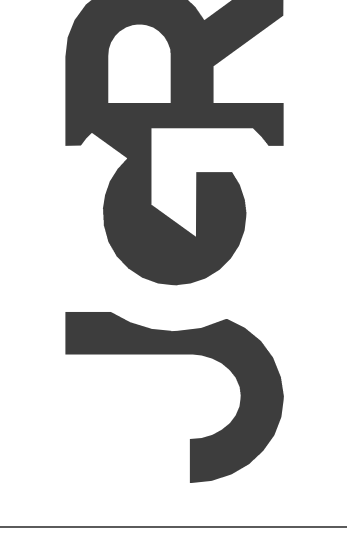


Table with 2 columns: Component Area (SF), Uplift Pressure (PSF). Rows for 10 SF and 100 SF areas across three roof zones.

- NOTES:
1. WIND COMPONENTS AND CLADDING PRESSURES SHOWN ACT AWAY FROM THE ROOF SURFACE.
2. COMPONENT AREA SHOWN IS THE AREA TRIBUTARY TO THE COMPONENT. LINEAR INTERPOLATION IS ALLOWED BETWEEN 10 AND 100 SQUARE FEET.
3. a = 14' - 0"

ENGINEERING CONSULTANTS, P.A. logo and contact information for Jones Gillam Renz.

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ROOSEVELT LOFTS HISTORIC REHABILITATION - APARTMENTS SAN ANGELO, TEXAS



Table for REVISIONS with columns for revision number, description, and date.

DATE: 1-16-2024
JOB: 22-3281
SHEET NO.:

SO.0

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 REVIEWED for code  
 compliance  
 4/15/2024

SCHEDULE OF SPECIAL INSPECTION SERVICES			
MATERIAL / ACTIVITY	SERVICE	INSPECTION	
		EXTENT	AGENT*
<b>1705.2 Steel Construction</b>			
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	Each submittal	S.I.
2. Material verification of structural steel	Shop (3) and field inspection	Periodic	S.I.
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Periodic	S.I.
4. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field inspection	Periodic	S.I.
5. Structural steel welding:			S.I.
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Observe or Perform as noted (4)	S.I.
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Observe (4)	S.I.
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Observe or Perform as noted (4)	S.I.
6. Structural steel bolting:	Shop (3) and field inspection		S.I.
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Observe or Perform as noted (4)	S.I.
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Observe (4)	S.I.
2) Snug-tight joints		Periodic	S.I.
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Perform (4)	S.I.
<b>1705.2.2 Steel Construction Other Than Structural Steel</b>			
1. Material verification of cold-formed steel deck:			S.I.
a. Identification markings	Field inspection	Periodic	S.I.
b. Manufacturer's certified test reports	Submittal Review	Each submittal	S.I.
2. Connection of cold-formed steel deck to supporting	Shop (3) and field inspection	Periodic	S.I.
a. Welding			S.I.
b. Other fasteners (in accordance with AISC 360 Section N6)			S.I.
1) Verify fasteners are in conformance with approved submittal		Periodic	S.I.
2) Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations		Periodic	S.I.
<b>1705.3 Concrete Construction</b>			
1. Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection	Periodic	S.I.
3. Inspection of anchors cast in concrete	Shop (3) and field inspection	Periodic	S.I.
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Periodic or as required by the research report issued by an approved source	S.I.
5. Verify use of approved design mix	Shop (3) and field inspection	Periodic	S.I.
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection	Continuous	S.I.
7. Inspection of concrete placement for proper application techniques	Shop (3) and field inspection	Continuous	S.I.
8. Inspection for maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Periodic	S.I.
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Periodic	S.I.
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Periodic	S.I.
<b>1705.4 Masonry Construction</b>			
<b>Level - B - Quality Assurance</b>			
<b>(A) Level A, B and C Quality Assurance:</b>			
1. Verify compliance with approved submittals	Field Inspection	Periodic	S.I.
<b>(B) Level B Quality Assurance:</b>			
1. Verification of $f_m$ and $f_{AC}$ prior to construction	Testing by unit strength method or prism test method	Periodic	S.I.
<b>(D) Levels B and C Quality Assurance:</b>			
1. Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout as delivered to the project	Field testing	Continuous	S.I.
2. Verify compliance with approved submittals	Field inspection	Periodic	S.I.
3. Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Field Inspection	Periodic	S.I.
4. Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages	Field Inspection	Periodic	S.I.
5. Verify construction of mortar joints	Field Inspection	Periodic	S.I.
7. Verify grout space prior to grouting	Field Inspection	Level B - Periodic Level C - Continuous	S.I.
9. Verify size and location of structural masonry elements	Field Inspection	Periodic	S.I.
10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction.	Field inspection	Level B - Periodic Level C - Continuous	S.I.
12. Verify preparation, construction, and protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F)	Field inspection	Periodic	S.I.
18. Prepare grout and mortar specimens	Field testing	Level B - Periodic Level C - Continuous	S.I.
19. Observe preparation of prisms	Field inspection	Level B - Periodic Level C - Continuous	S.I.
20. Inspection of anchors and reinforcing steel post-installed (epoxy, expansion, etc.): Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, location to masonry joints, masonry minimum thickness, anchor embedment and tightening torque	Field inspection	Periodic or as required by the research report issued by an approved source	S.I.
<b>* INSPECTION AGENTS</b>			
<b>FIRM</b>		<b>ADDRESS</b>	
1. G.E. Geotechnical Engineer			
2. S.I. Special Inspector - Not Yet Selected			
3.			
4.			
<p>Notes: 1. The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.</p> <p>2. If the list of Inspection Agents is noted as "Not Yet Selected" the General Contractor shall coordinate submittal of special inspection agencies for approval by the Building Official and the Design Professional.</p> <p>3. Special Inspections as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2</p> <p>4. Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks for each welded joint, bolted connection, or steel element.</p> <p>5. NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, N7.</p>			

STATEMENT OF SPECIAL INSPECTIONS

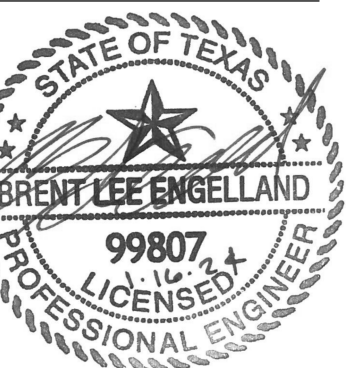
- THIS STRUCTURAL STATEMENT OF SPECIAL INSPECTIONS IS INCLUDED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE BUILDING CODE. THE TABLE OF REQUIRED SPECIAL INSPECTIONS IDENTIFIES THE STRUCTURAL ITEMS TO BE INSPECTED APPLICABLE TO THIS PROJECT AS WELL AS IDENTIFYING THE APPROVED AGENCIES TO BE RETAINED FOR CONDUCTING THESE INSPECTIONS AND TESTS.
- SPECIAL INSPECTIONS SHALL BE PERFORMED BY PERSONNEL AS INDICATED IN THE TABLE.  
 S.I. SPECIAL INSPECTOR  
 G.E. GEOTECHNICAL ENGINEER
- IF SPECIAL INSPECTION IS WAIVED BY THE GOVERNING AUTHORITIES, THE GENERAL CONTRACTOR SHALL PROVIDE THE ENGINEER OF RECORD A COPY OF THE BUILDING OFFICIALS WRITTEN EXEMPTION FOR SPECIAL INSPECTION PRIOR TO STARTING WORK.

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ROOSEVELT LOFTS  
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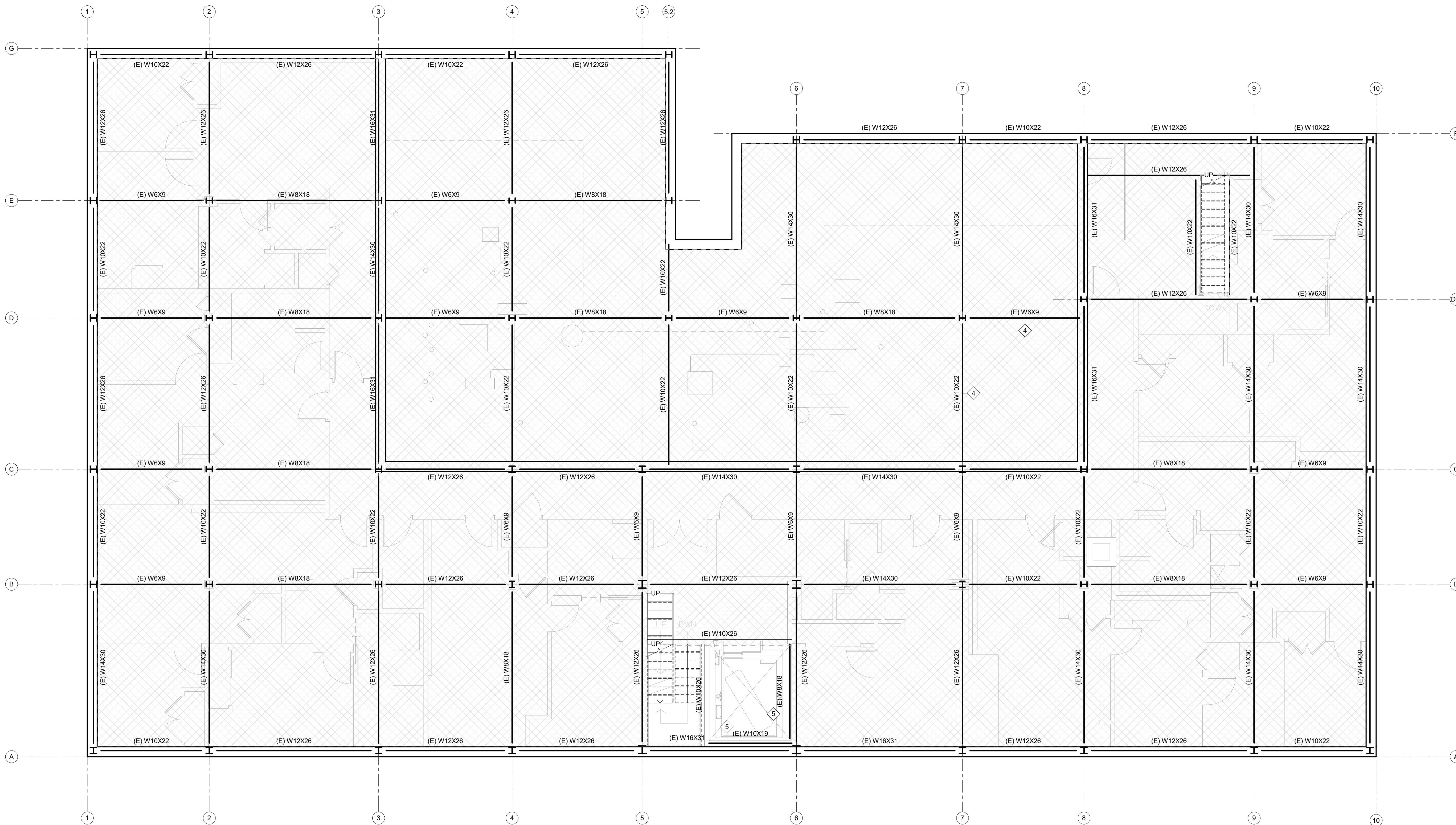
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**PLAN NOTES**

1. DEMO ALL FLOOR AND ROOF SLAB/DECK AND METAL JOISTS IN HATCHED AREA.
2. EXISTING BEAMS MARKED (E) TO REMAIN U.N.O.
3. CONTRACTOR TO SHORE AND BRACE EXISTING ELEMENTS DURING DEMO. BRACING TO REMAIN IN PLACE UNTIL NEW METAL DECK IS IN PLACE AND ATTACHED.
4. DEMO EXISTING BEAM AND REPLACE WITH NEW BEAM PER S1.1
5. DEMO EXISTING BEAM TO ACCOMMODATE NEW CMU ELEVATOR SHAFT



**2D** 2ND FLOOR DEMO PLAN  
 3/16" = 1'-0"

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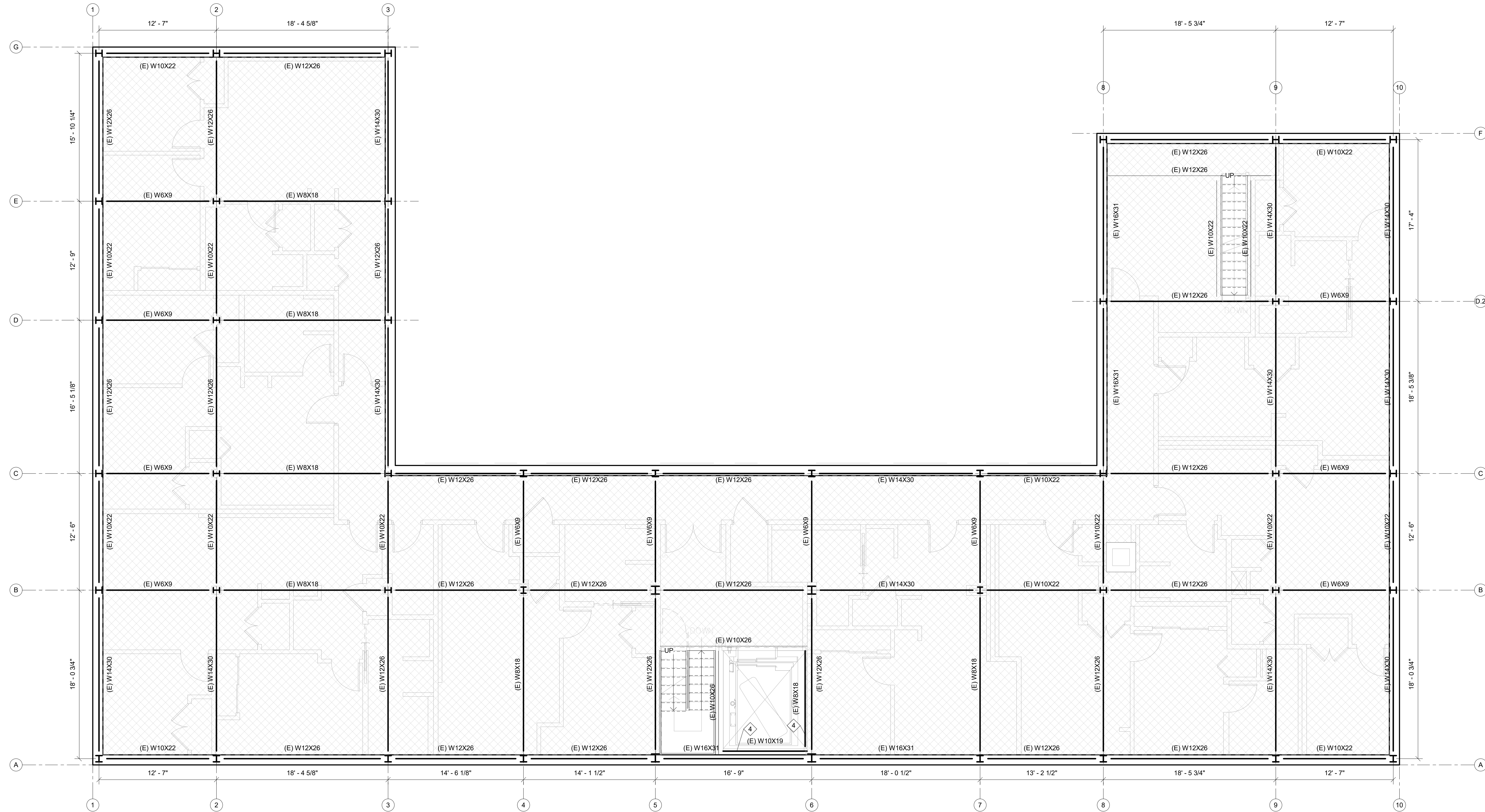
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**PLAN NOTES**

1. DEMO ALL FLOOR SLAB/DECK AND METAL JOISTS IN HATCHED AREA.
2. EXISTING BEAMS MARKED (E) TO REMAIN U.N.O.
3. CONTRACTOR TO SHORE AND BRACE EXISTING ELEMENTS DURING DEMO. BRACING TO REMAIN IN PLACE UNTIL NEW METAL DECK IS IN PLACE AND ATTACHED.
4. DEMO EXISTING BEAM TO ACCOMMODATE NEW CMU ELEVATOR SHAFT



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3D 3RD FLOOR DEMO PLAN  
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 NORTH

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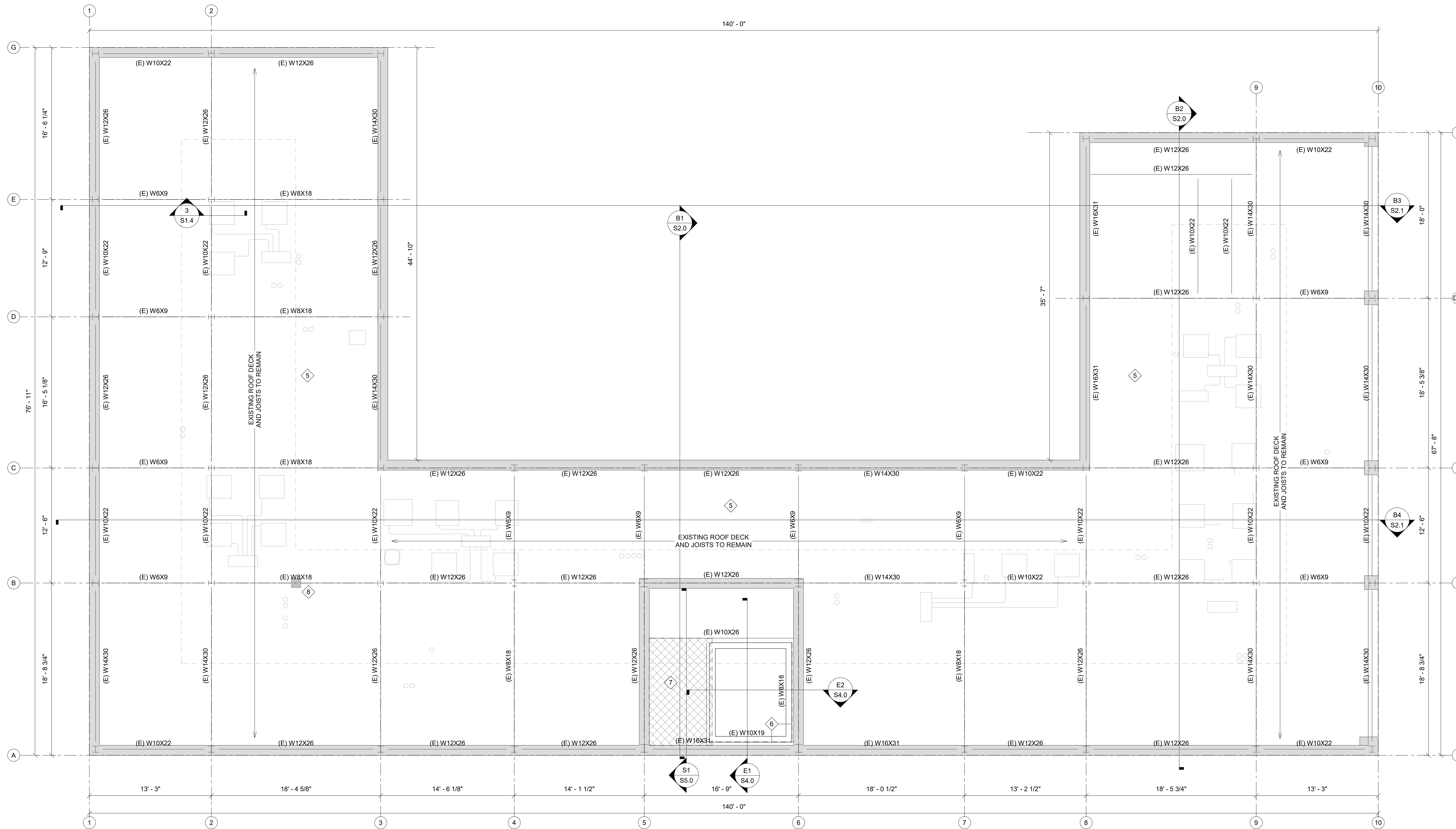
**PLAN NOTES**

1. FOR GENERAL STRUCTURAL NOTES (GSN) SEE SHEET S0.0
2. DIMENSIONS SHOWN HERE APPLY TO STRUCTURAL ELEMENTS ONLY. SEE ARCHITECTURAL FOR ANY DIMENSIONS NOT NOTED HERE.
3. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
4. EXISTING ROOF JOISTS & DECK TO REMAIN.
5. DEMO AND REPLACE ROOFING, COVER BOARDS, INSULATION, AND MEMBRANE PER ARCHITECTURAL DRAWINGS. TAKE CARE TO PROTECT EXISTING CONCRETE ROOF DECK DURING RE-ROOF WORK.
6. DEMO EXISTING BEAM TO ACCOMMODATE NEW CMU ELEVATOR SHAFT
7. DEMO EXISTING CONCRETE FLOOR DECK IN HATCHED AREA.
8. DEMO EXISTING CONCRETE STUB COLUMN ABOVE ROOF TO ACCOMMODATE NEW RTU SUPPORT BEAM.

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**RD** ROOF DEMO PLAN  
 3/16" = 1'-0"

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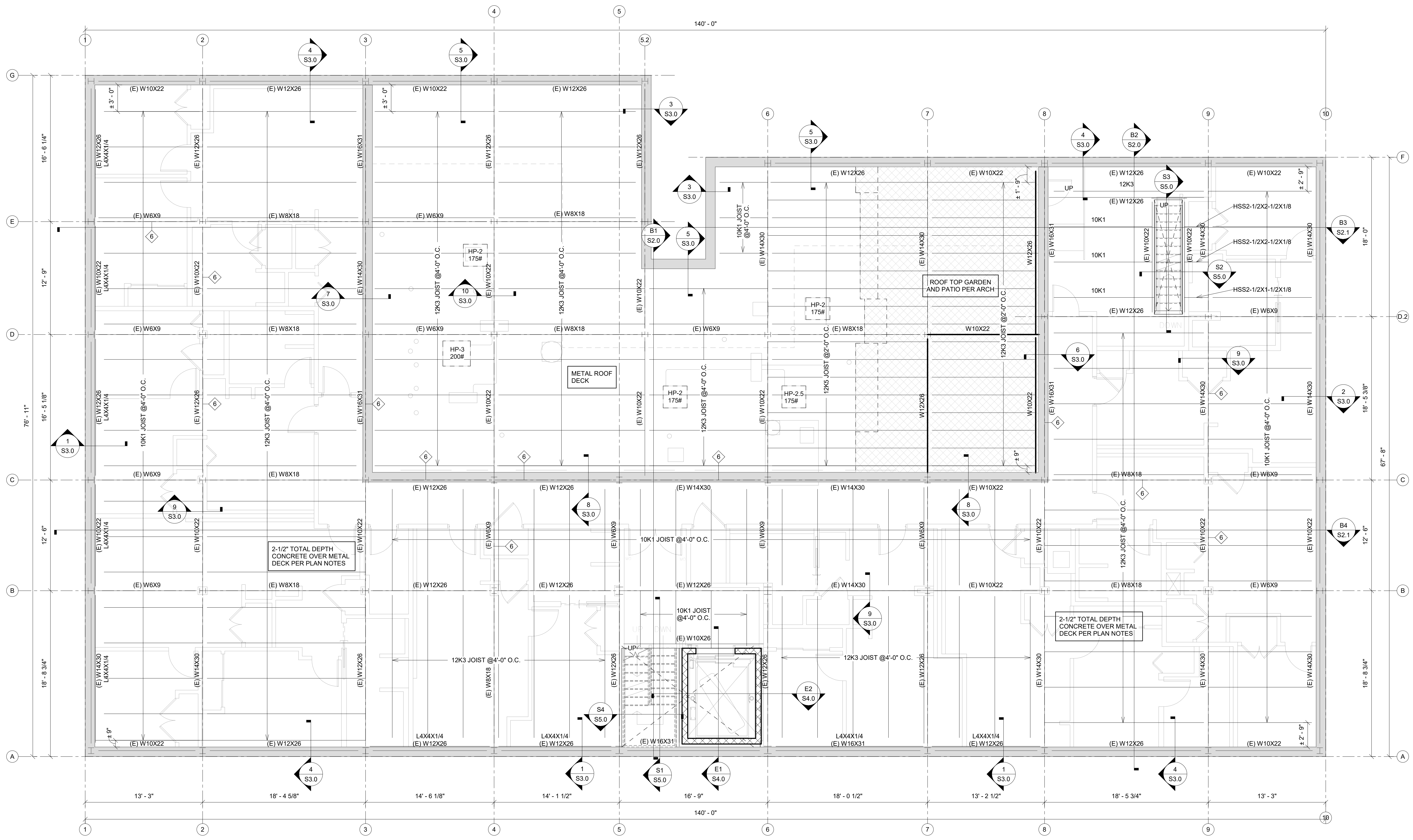




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- PLAN NOTES**
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  - DIMENSIONS SHOWN HERE APPLY TO STRUCTURAL ELEMENTS ONLY. SEE ARCHITECTURAL FOR ANY DIMENSIONS NOT NOTED HERE.
  - ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
  - 2-1/2" TOTAL DEPTH CONCRETE SLAB OVER METAL DECK:  
 NORMAL WEIGHT CONCRETE W/ 6x6 W2.9xW2.9 WWF AT MIDHEIGHT OF CONCRETE W/ 1.0C24 METAL DECK.  
 ATTACH DECK TO PERPENDICULAR SUPPORTS W/ #12 TEK SCREWS IN 3/4 PATTERN. ATTACH DECK TO PARALLEL SUPPORTS W/ #12 TEK SCREWS @ 12" O.C. DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.
  - METAL ROOF DECK:  
 NEW ROOF DECK TO BE 1.5C24 METAL DECK.  
 ATTACH DECK TO PERPENDICULAR SUPPORTS WITH #12 TEK SCREWS AT EVERY FLUTE. ATTACH DECK TO PARALLEL SUPPORTS WITH #12 TEK SCREWS AT 12" O.C. DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.

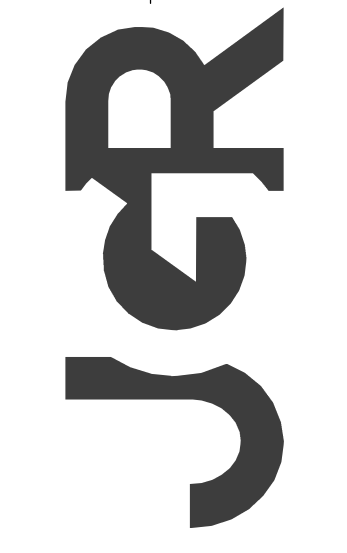
- DELAMINATION ON BEAM
  - CLEAN AND REMOVE RUST AND SCALE.
  - CHECK REMAINING THICKNESS.
  - WHERE MORE THAN 25% OF ORIGINAL THICKNESS IS MISSING, WELD 5/16" COVER PLATE W/ MIN. 6" ON EACH SIDE OF DAMAGE
  - PRIME AND PAINT REPAIRED AREA AFTER REPAIR IS COMPLETE
- CONTRACTOR TO FIELD VERIFY CONDITION OF ALL EXISTING BEAMS. ADDITIONAL REPAIRS MAY BE REQUIRED BEYOND THOSE NOTED.



2ND FLOOR AND LOW ROOF FRAMING PLAN  
 3/16" = 1'-0"

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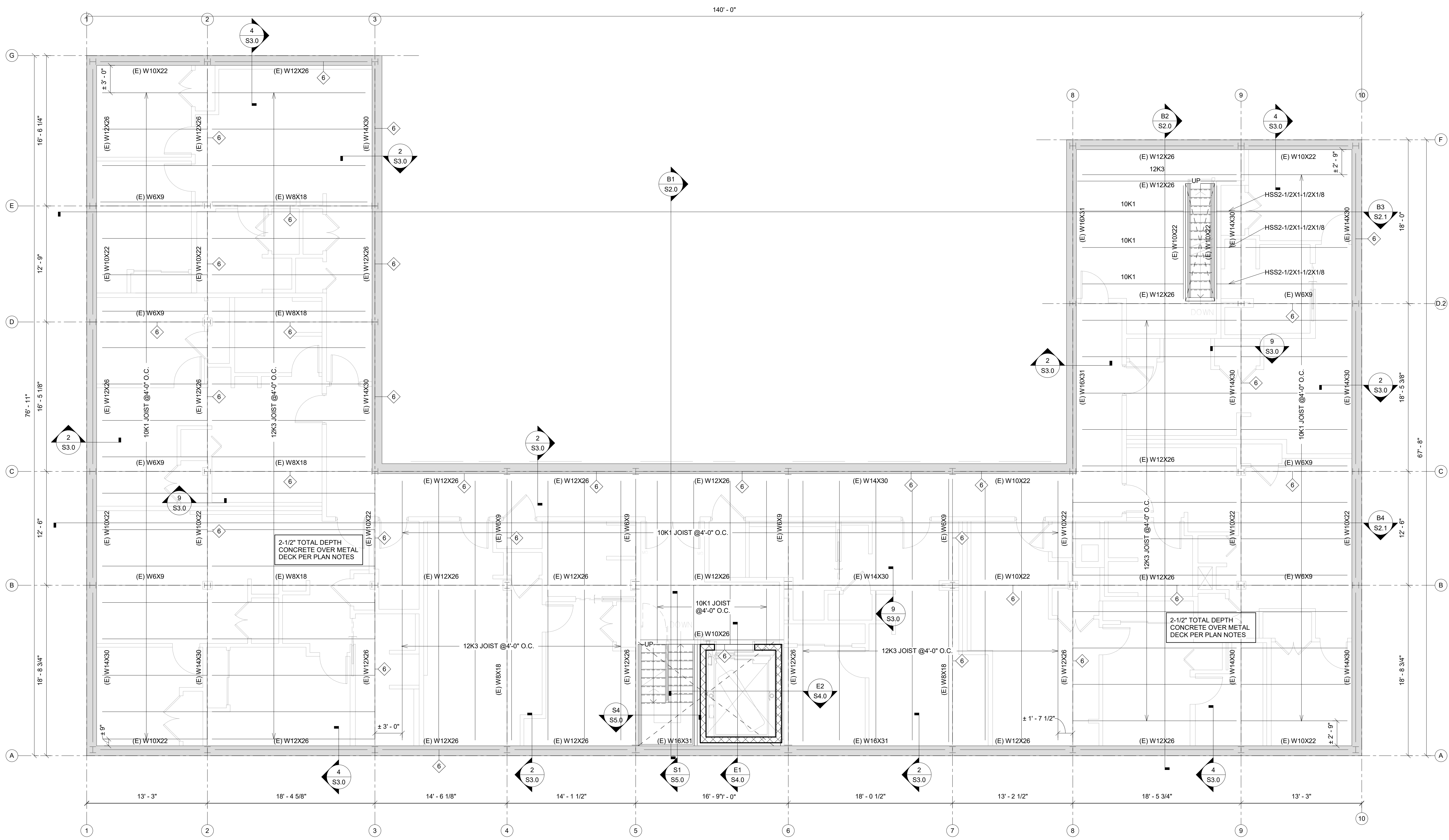
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  - ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
  - 2-1/2" TOTAL DEPTH CONCRETE SLAB OVER METAL DECK:  
 NORMAL WEIGHT CONCRETE W/ 6x6 W2.9xW2.9 WWF AT MIDHEIGHT OF CONCRETE W/ 1.0C24 METAL DECK.  
 ATTACH DECK TO PERPENDICULAR SUPPORTS W/ #12 TEK SCREWS IN 3/4 PATTERN. ATTACH DECK TO PARALLEL SUPPORTS W/ #12 TEK SCREWS @ 12" O.C.  
 DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.
  - METAL ROOF DECK:  
 NEW ROOF DECK TO BE 1.5C24 METAL DECK.  
 ATTACH DECK TO PERPENDICULAR SUPPORTS WITH #12 TEK SCREWS AT EVERY FLUTE. ATTACH DECK TO PARALLEL SUPPORTS WITH #12 TEK SCREWS AT 12" O.C.  
 DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.

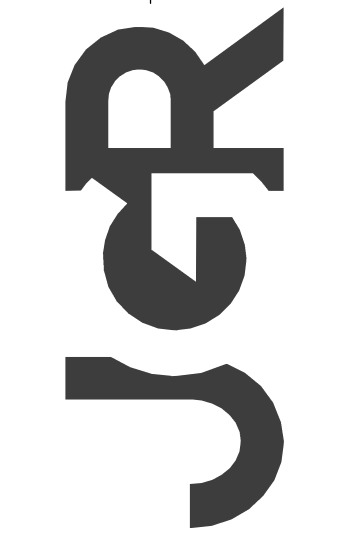
- DELAMINATION ON BEAM  
 A. CLEAN AND REMOVE RUST AND SCALE.  
 B. CHECK REMAINING THICKNESS.  
 C. WHERE MORE THAN 25% OF ORIGINAL THICKNESS IS MISSING, WELD 5/16" COVER PLATE W/ MIN. 6" ON EACH SIDE OF DAMAGE.  
 D. PRIME AND PAINT REPAIRED AREA AFTER REPAIR IS COMPLETE
- CONTRACTOR TO FIELD VERIFY CONDITION OF ALL EXISTING BEAMS. ADDITIONAL REPAIRS MAY BE REQUIRED BEYOND THOSE NOTED.



3 3RD FLOOR FRAMING PLAN  
 3/16" = 1'-0"

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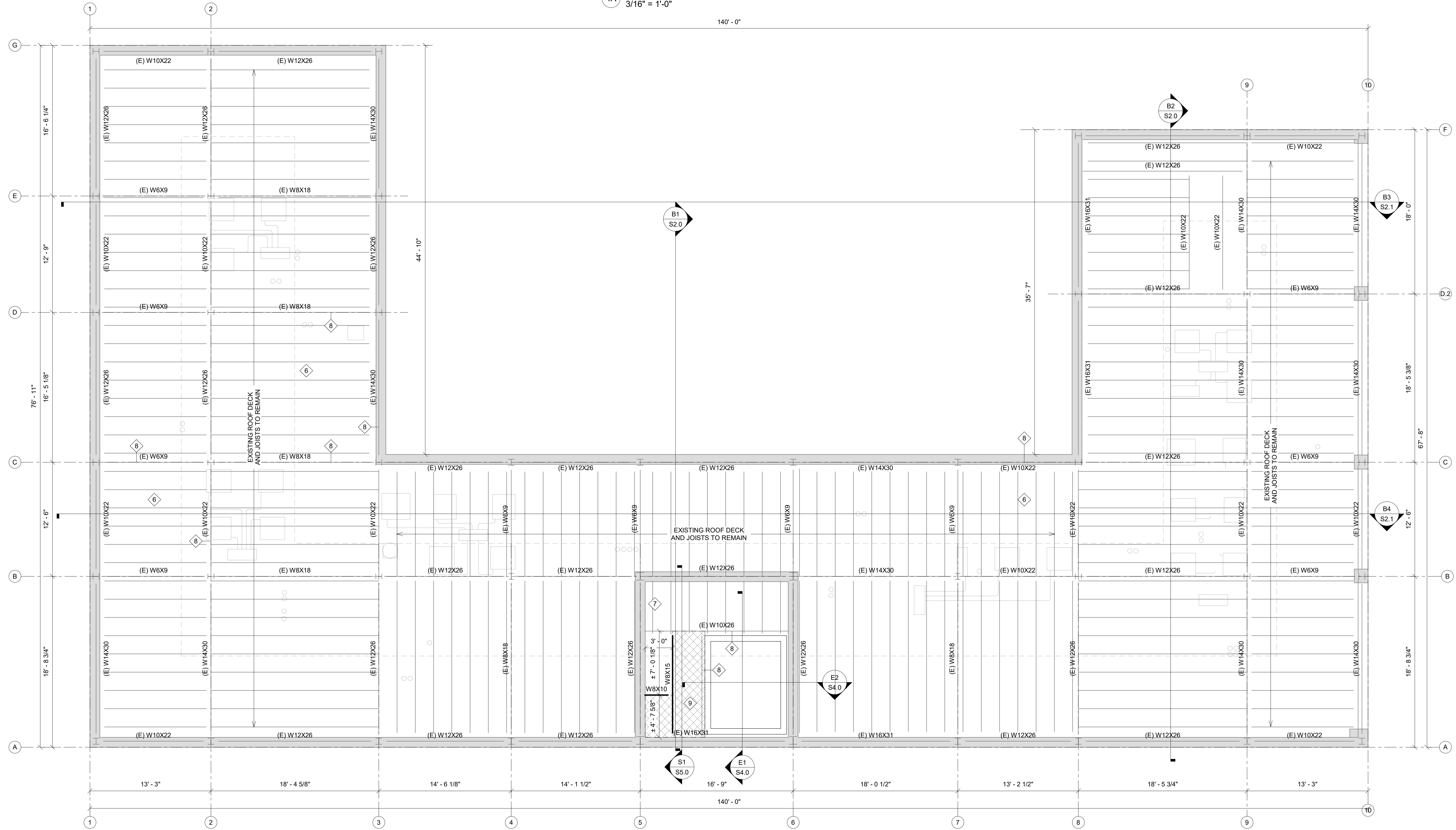
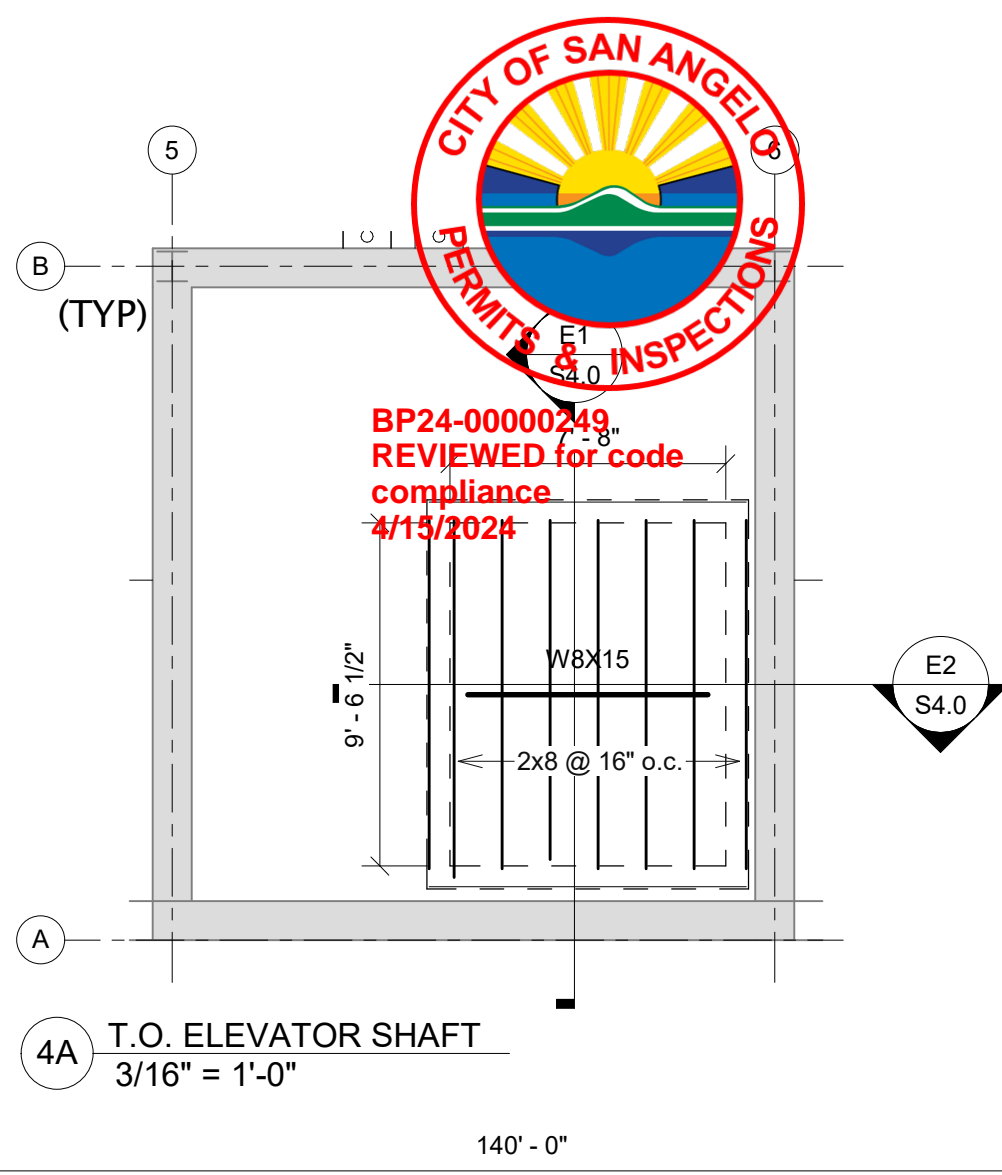
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- ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
- EXISTING ROOF JOISTS & DECK TO REMAIN.
- REPLACE EXISTING ROOFING, COVER BOARDS, INSULATION, AND MEMBRANE PER ARCHITECTURAL DRAWINGS. TAKE CARE TO PROTECT EXISTING CONCRETE ROOF DECK DURING RE-ROOF WORK.
- REPAIR CORRODED JOISTS IN THIS BAY PER DETAIL 11 / S3.0 ANY JOIST ELEMENT THAT HAS LOST MORE THAN 25% OF ITS ORIGINAL CROSS-SECTION DUE TO CORROSION OR DAMAGE NEEDS TO BE REPAIRED. G.C. TO REVIEW ALL JOISTS IN ALL BAYS FOR DAMAGE OR CORRODED JOISTS.
- REPAIR DAMAGED CONCRETE ROOF DECK THIS BAY BY CAREFULLY REMOVING EXISTING DECK TO THE NEXT ADJACENT JOIST BEYOND THE DAMAGED AREA. REPLACE WITH 2-1/2" TOTAL DEPTH N.W. CONCRETE SLAB OVER 1.0C20 METAL DECK W/ 6x6 W2.9xW2.9 WWF AT MIDHEIGHT OF CONCRETE. TACK WELD DECK TO EXISTING JOISTS ON BOTH SIDES AT EACH FLUTE.

- DELAMINATION ON BEAM
  - CLEAN AND REMOVE RUST AND SCALE.
  - CHECK REMAINING THICKNESS.
  - WHERE MORE THAN 25% OF ORIGINAL THICKNESS IS MISSING, WELD 5/16" COVER PLATE W/ MIN. 6" ON EACH SIDE OF DAMAGE.
  - PRIME AND PAINT REPAIRED AREA AFTER REPAIR IS COMPLETE.
- REPLACE EXISTING FLOOR IN HATCHED AREA W/ 2-1/2" TOTAL DEPTH CONCRETE SLAB OVER METAL DECK. NORMAL WEIGHT CONCRETE W/ 6x6 W2.9xW2.9 WWF AT MIDHEIGHT OF CONCRETE W/ 1.0C24 METAL DECK. ATTACH DECK TO PERPENDICULAR SUPPORTS W/ #12 TEK SCREWS IN 3/4" PATTERN. ATTACH DECK TO PARALLEL SUPPORTS W/ #12 TEK SCREWS @ 12" O.C. DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.
- CONTRACTOR TO FIELD VERIFY CONDITION OF ALL EXISTING BEAMS. ADDITIONAL REPAIRS MAY BE REQUIRED BEYOND THOSE NOTED.



**4 ROOF FRAMING PLAN**  
3/16" = 1'-0"

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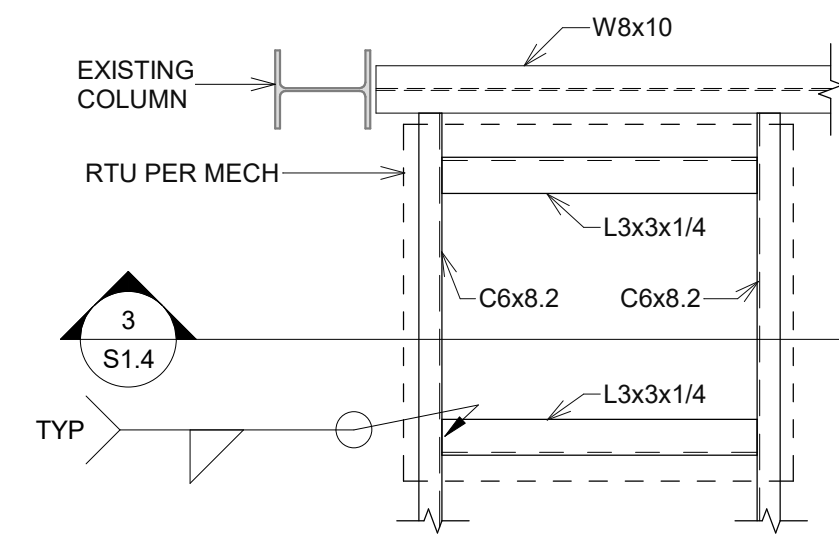


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**PLAN NOTES**

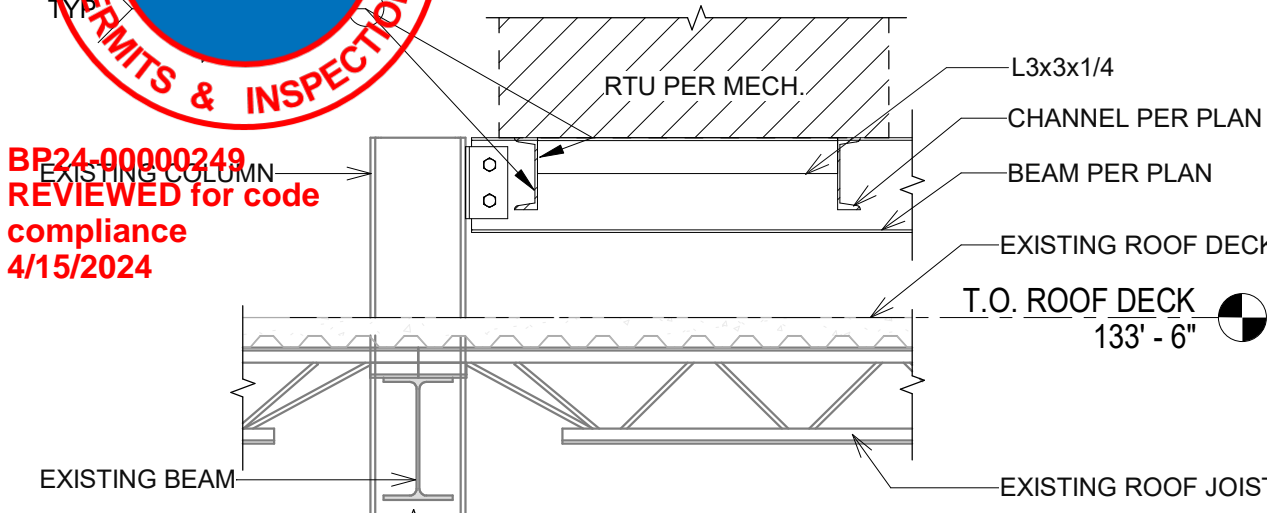
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- ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
- COORDINATE LOCATION OF MECHANICAL ROOFTOP UNITS WITH MECHANICAL DRAWINGS.
- ALL STEEL EXPOSED TO WEATHER TO BE PRIMED AND PAINTED.



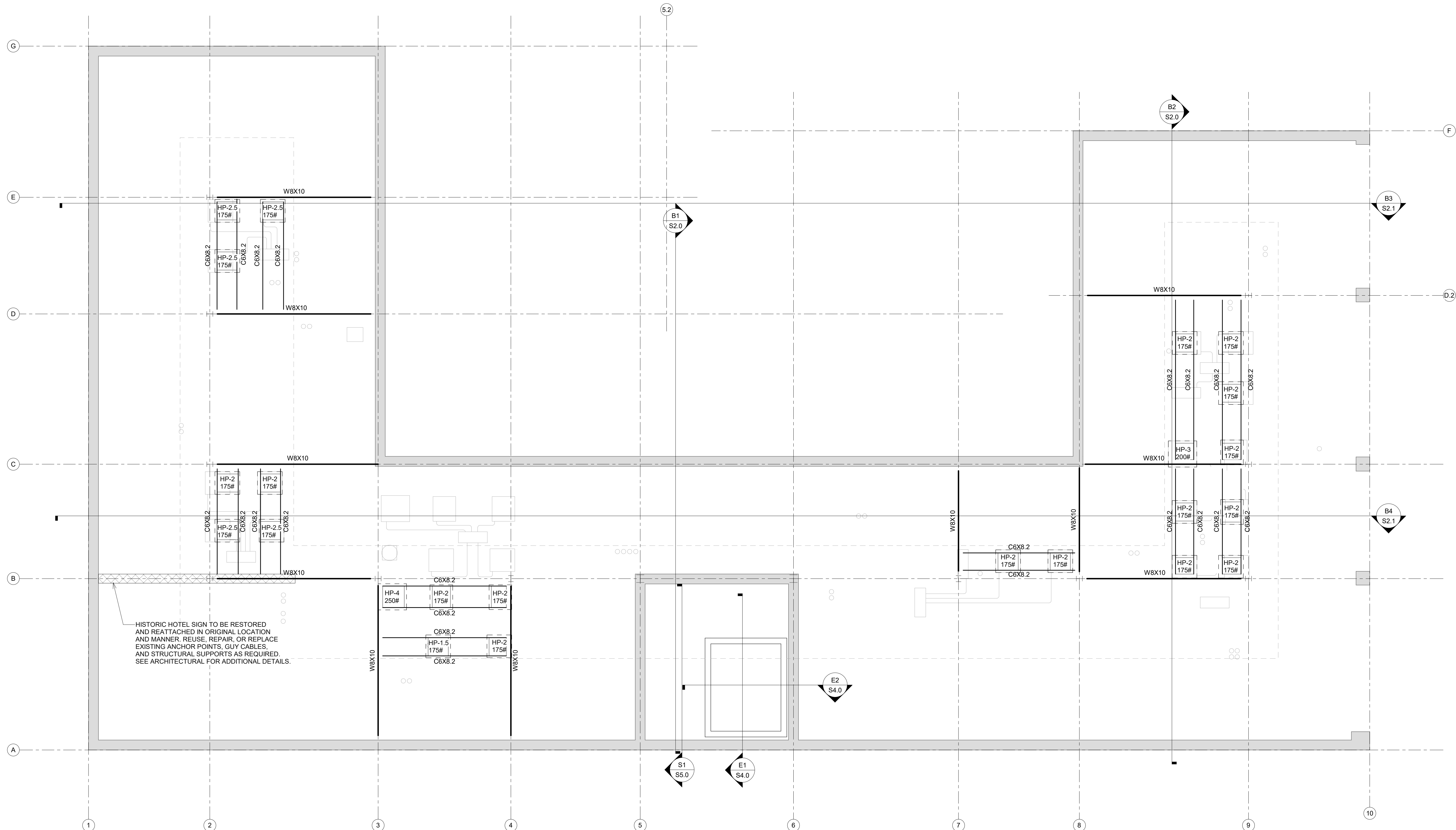
2 TYPICAL RTU SUPPORT PLAN  
3/4" = 1'-0"



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3 TYPICAL RTU SUPPORT SECTION  
3/4" = 1'-0"



1 MECHANICAL FRAMING PLAN  
3/16" = 1'-0"



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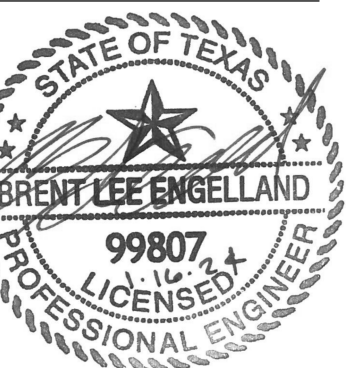
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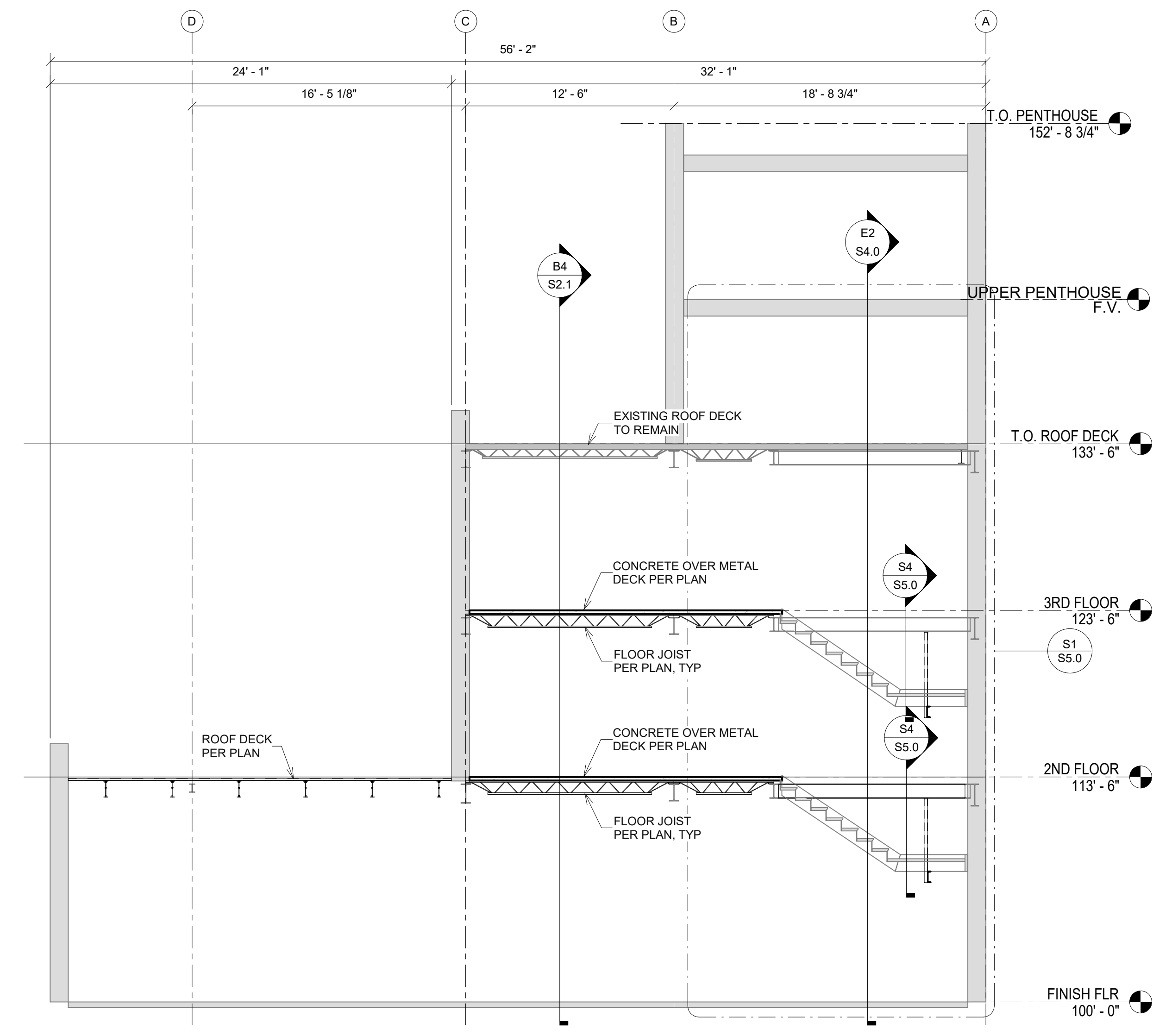


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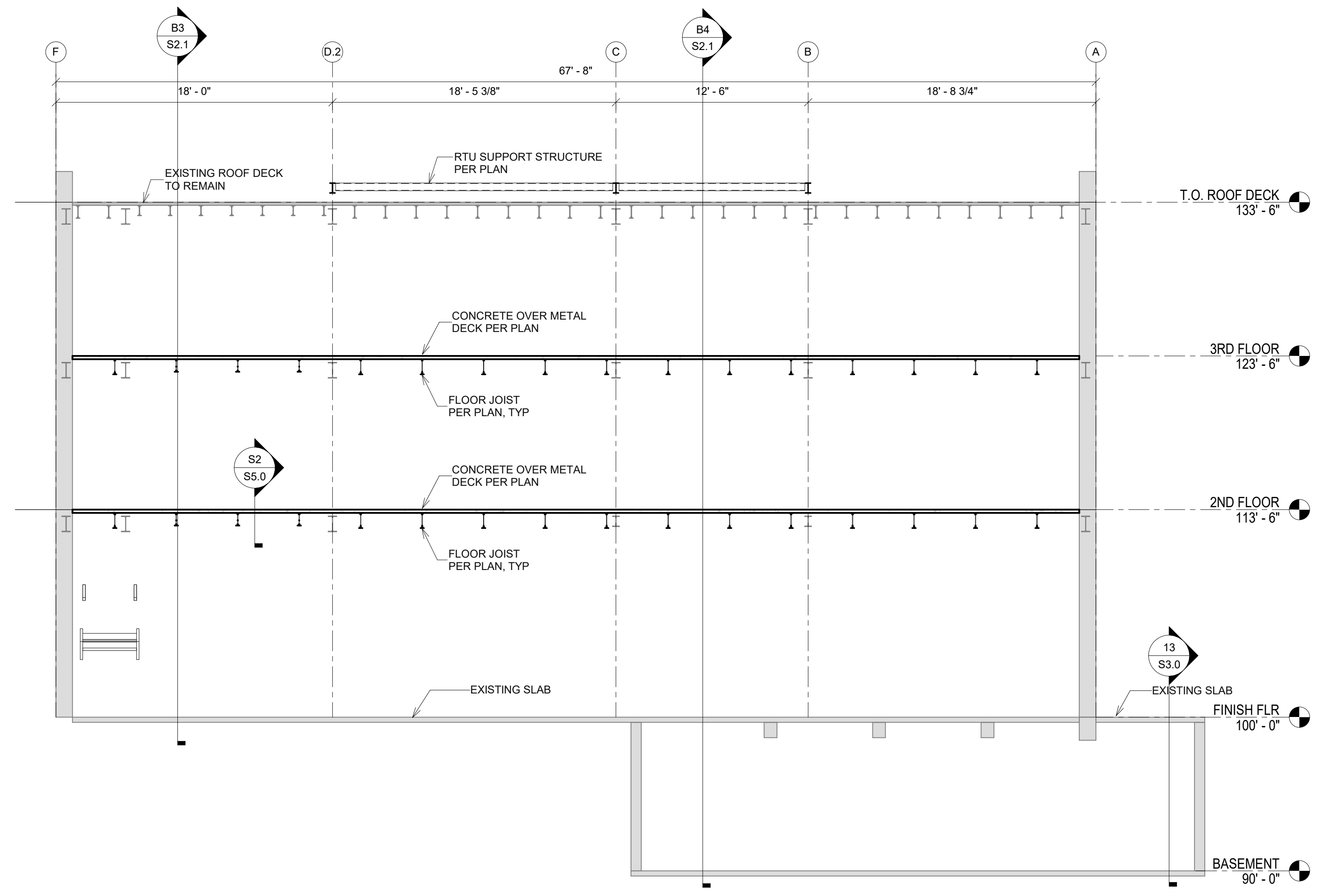

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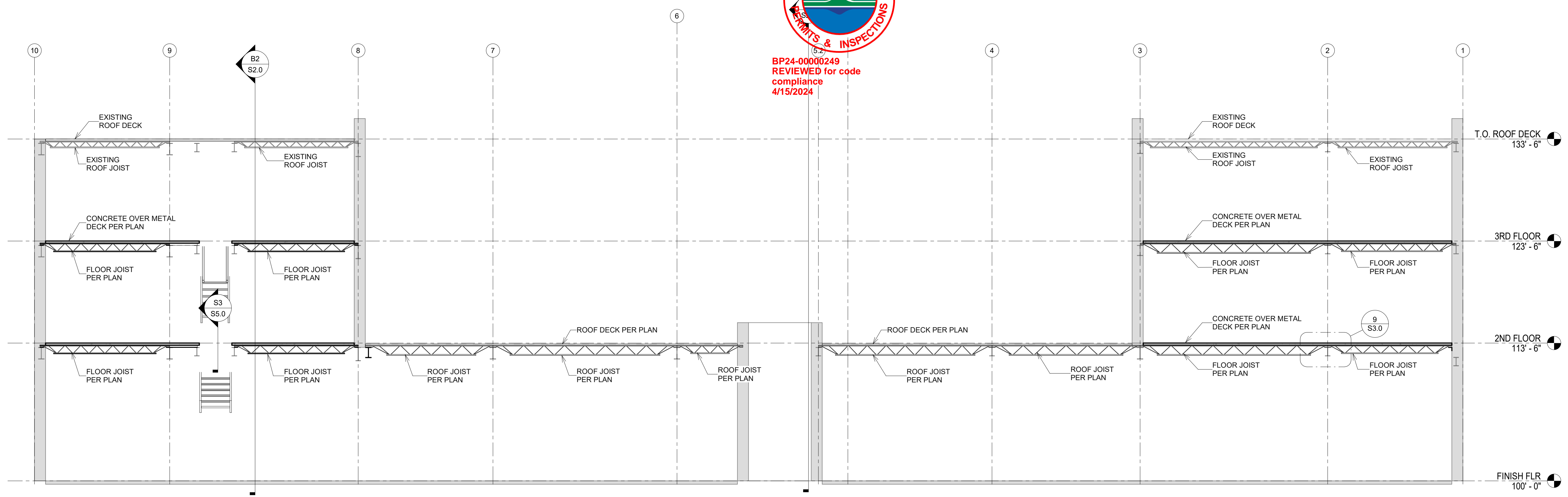
B1 Section B1  
 3/16" = 1'-0"



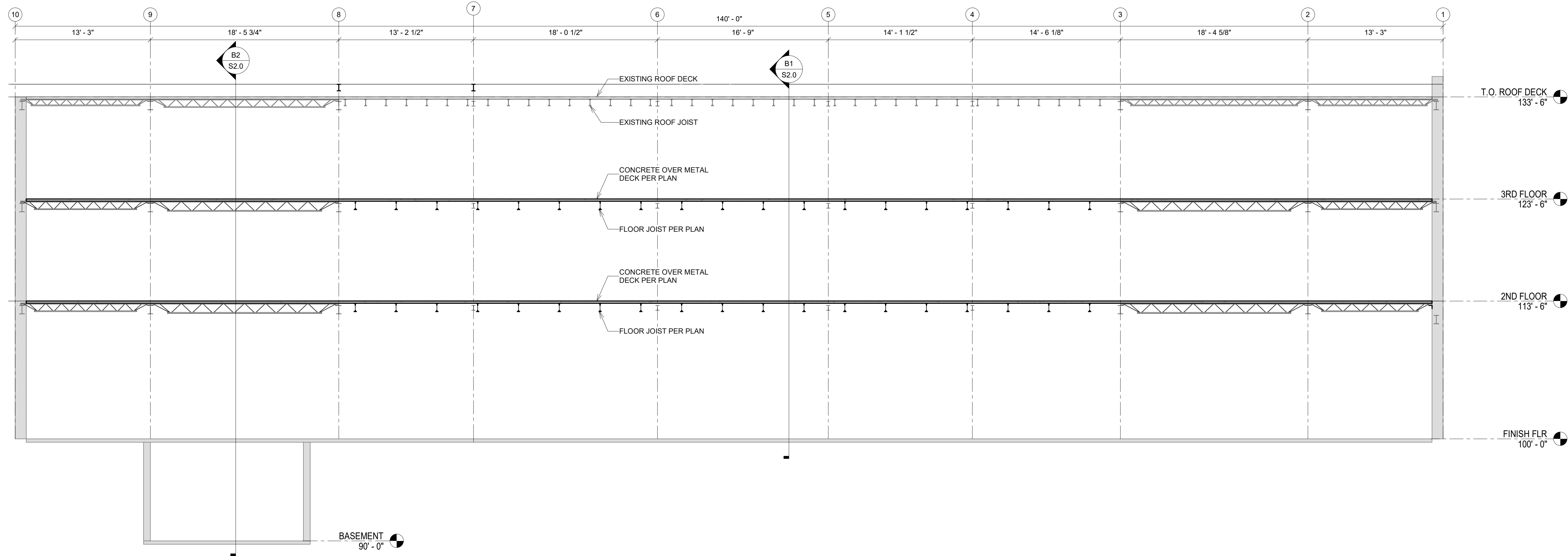
B2 Section B2  
 3/16" = 1'-0"



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B3 Section B3  
3/16" = 1'-0"

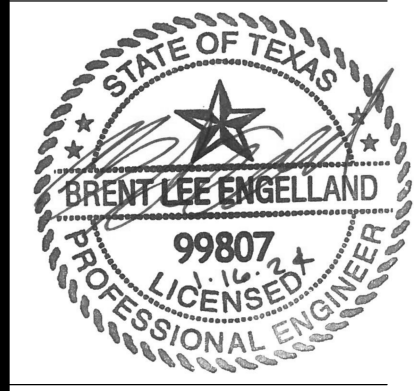


B4 Section B4  
3/16" = 1'-0"

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NO.	DESCRIPTION	DATE

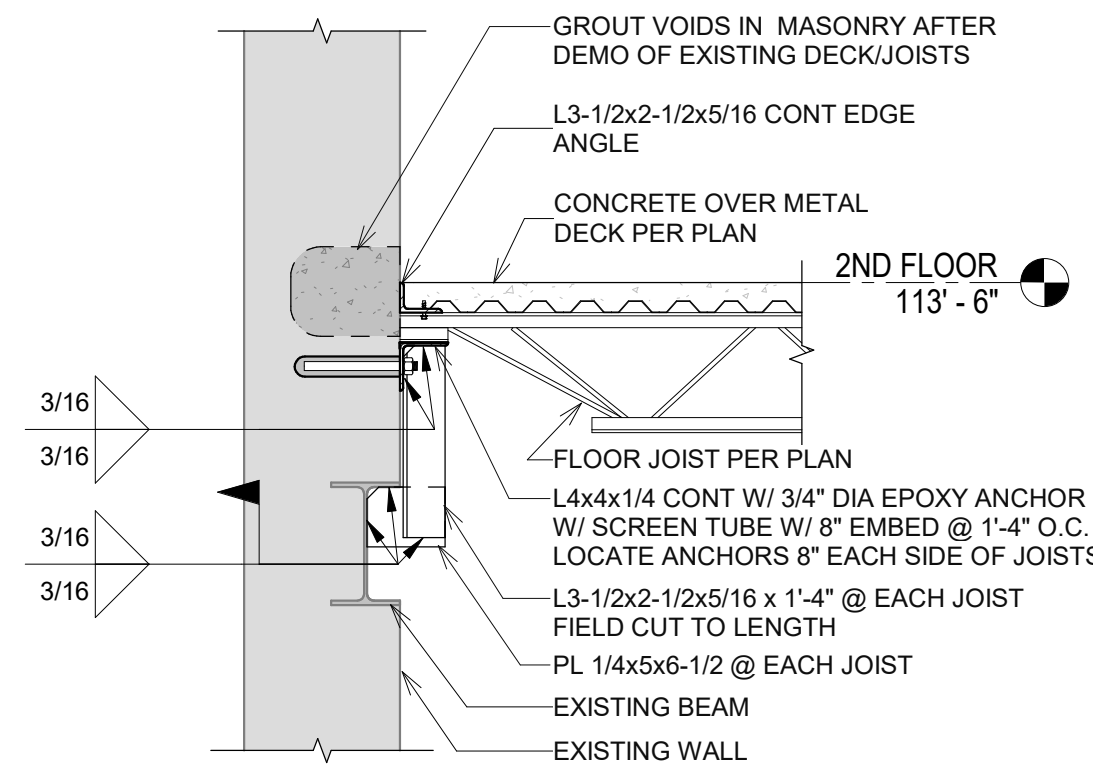
DATE: 1-16-2024  
JOB: 22-3281  
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**S2.1**

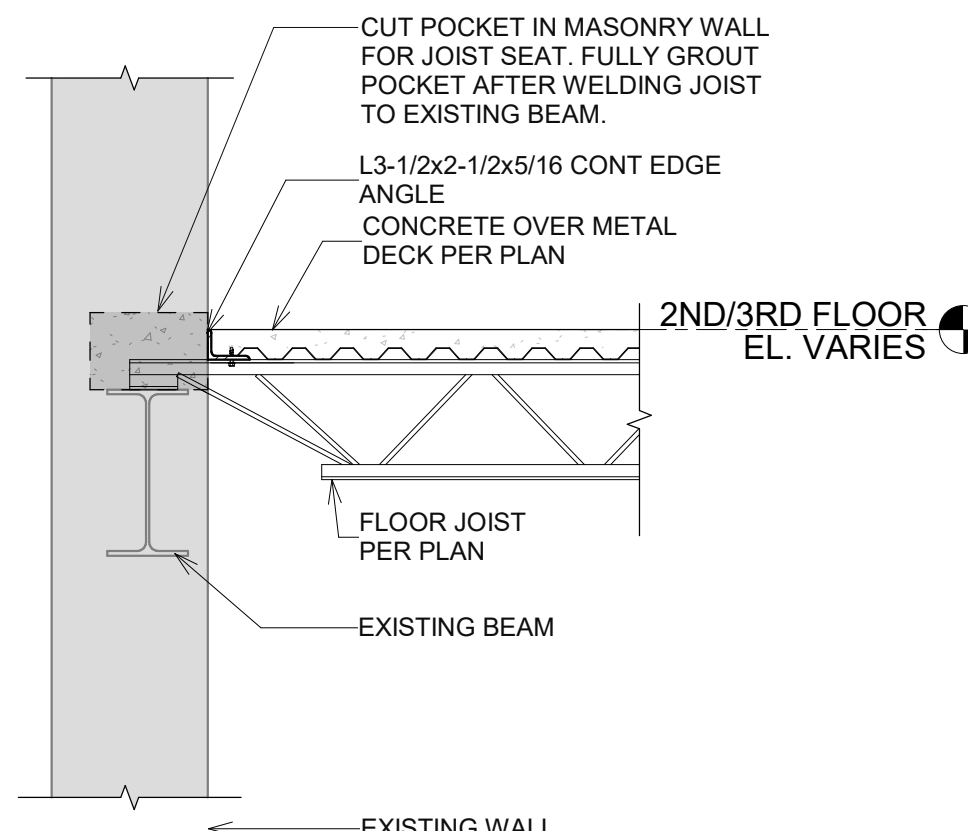
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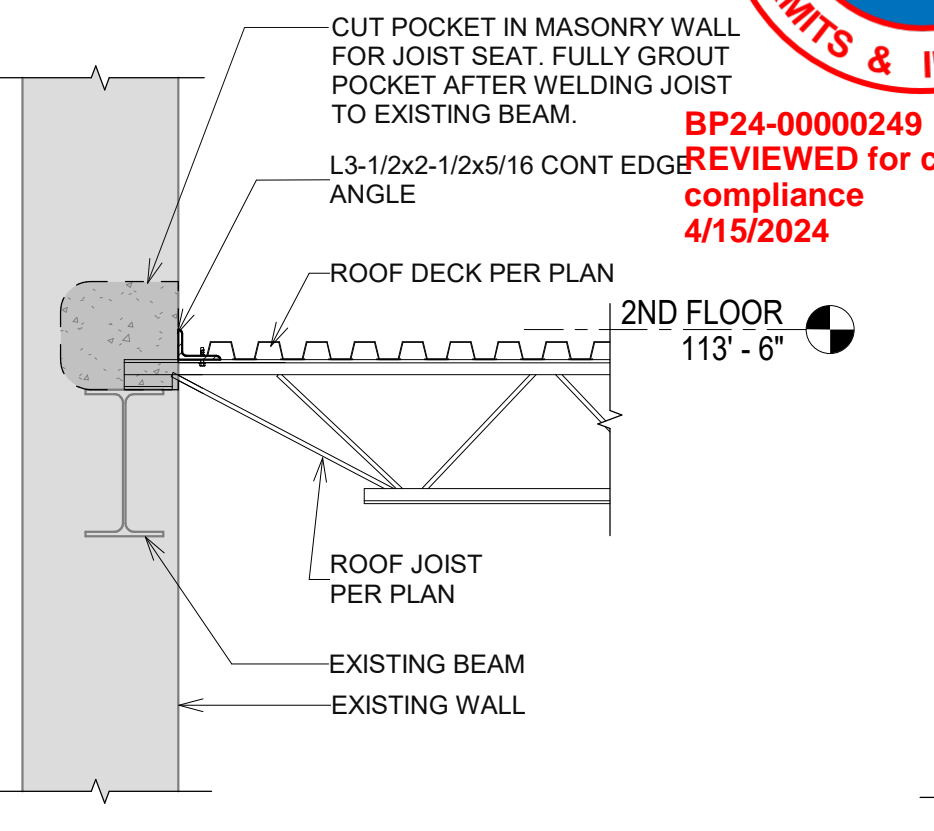
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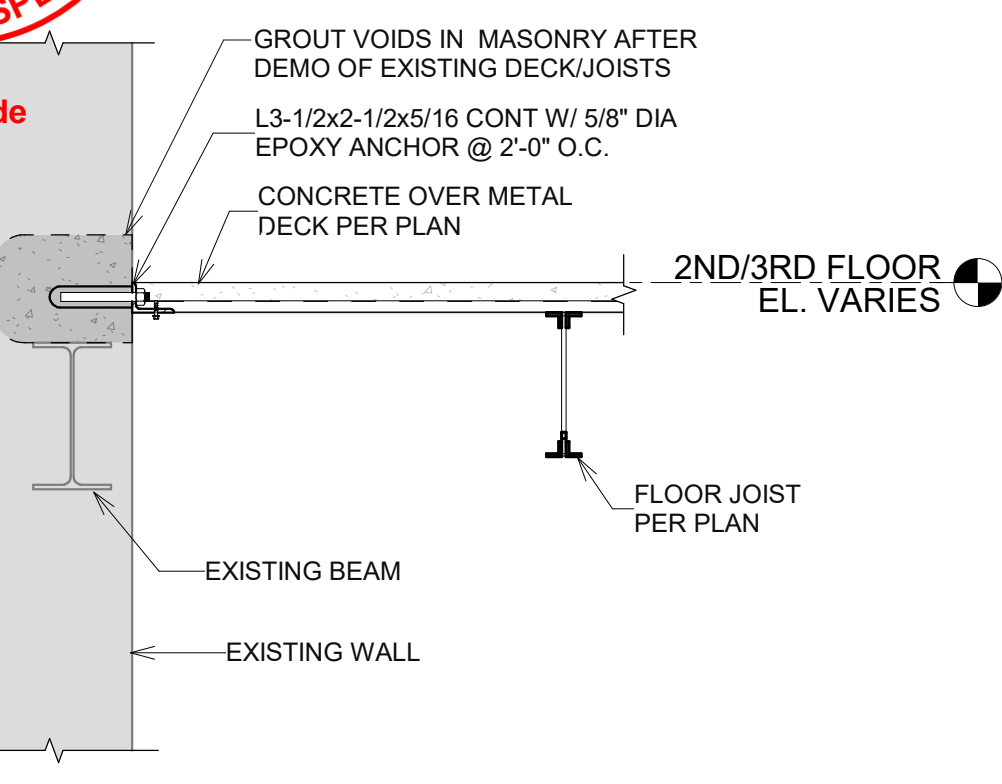
1 Section 1  
3/4" = 1'-0"



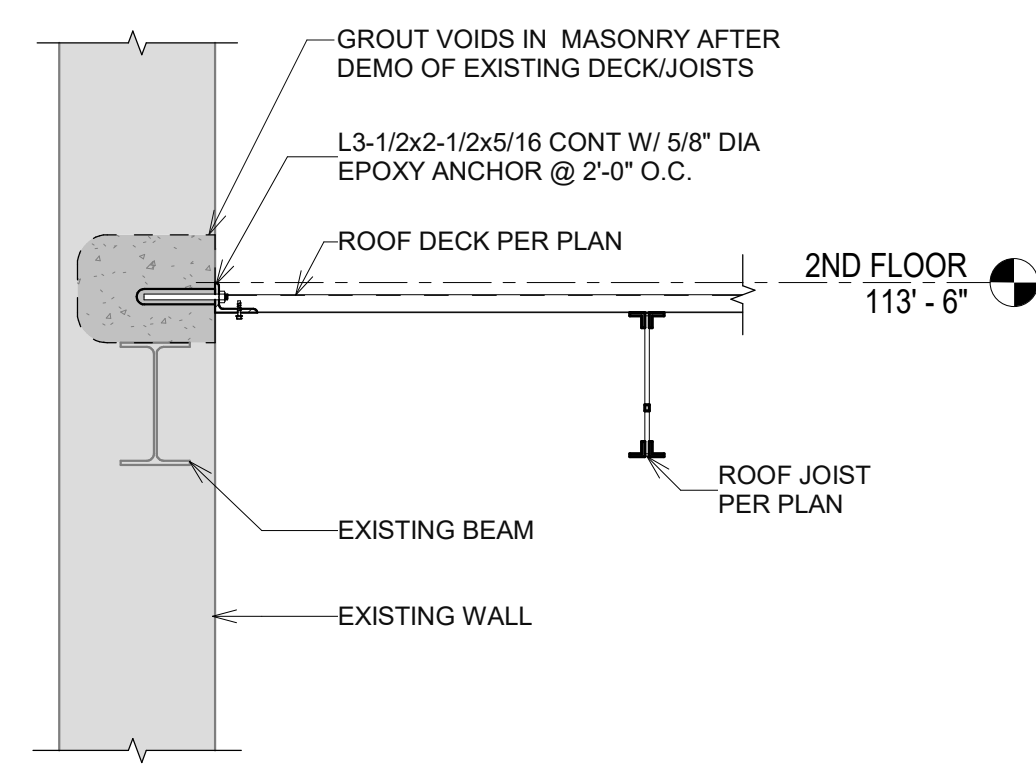
2 Section 2  
3/4" = 1'-0"



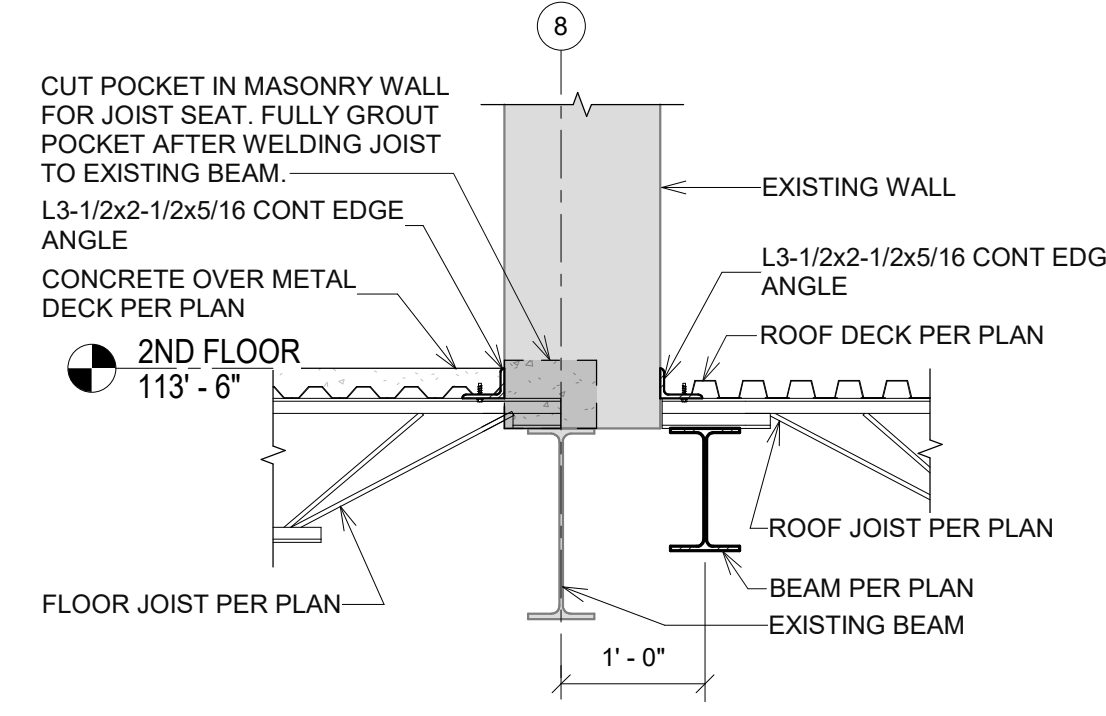
3 Section 3  
3/4" = 1'-0"



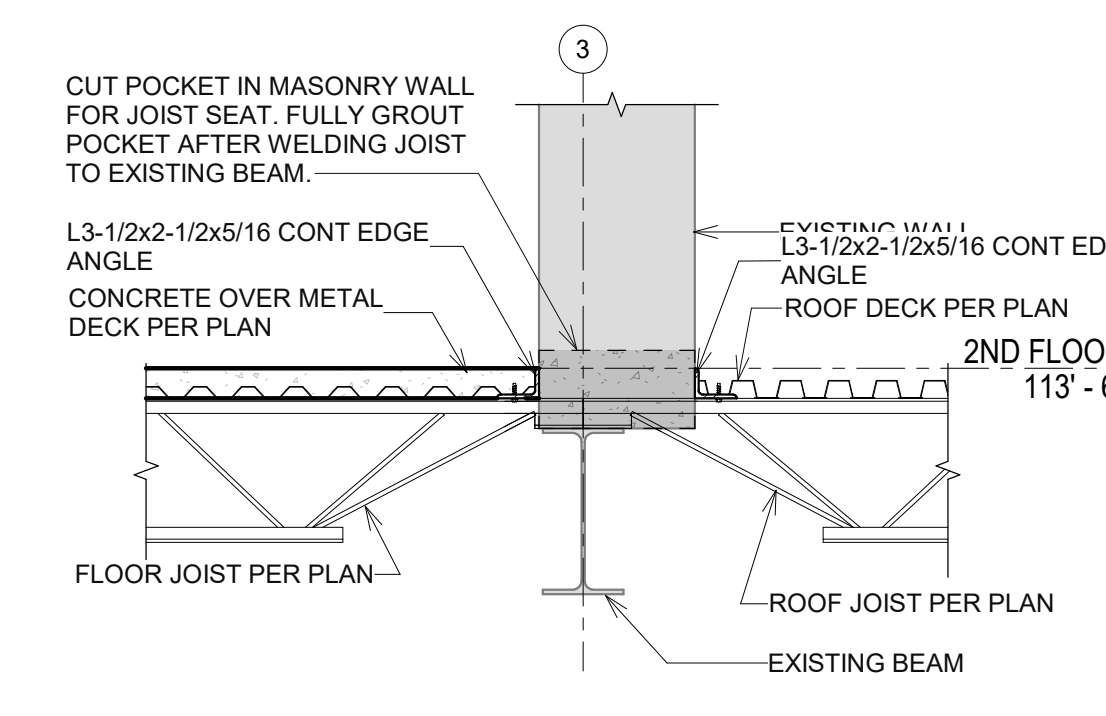
4 Section 4  
3/4" = 1'-0"



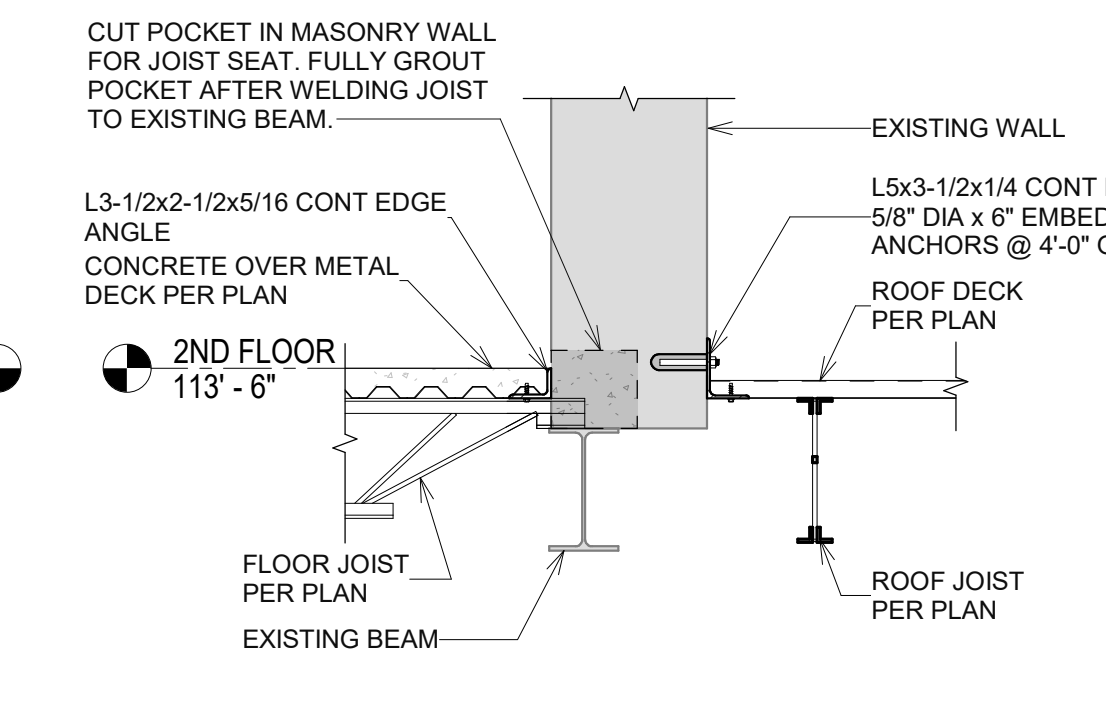
5 Section 5  
3/4" = 1'-0"



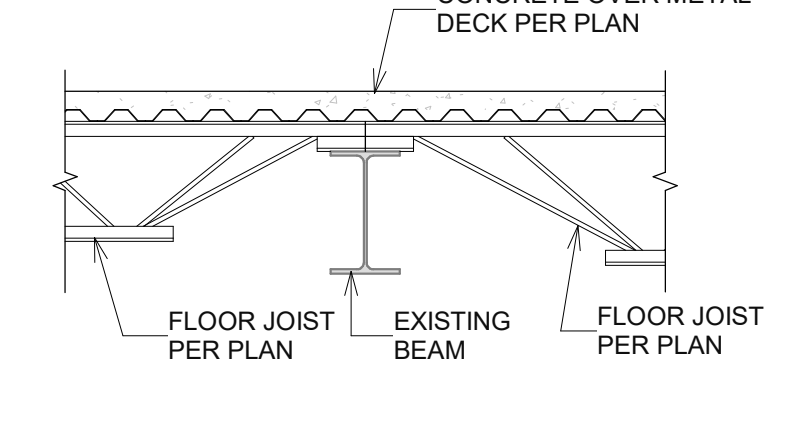
6 Section 6  
3/4" = 1'-0"



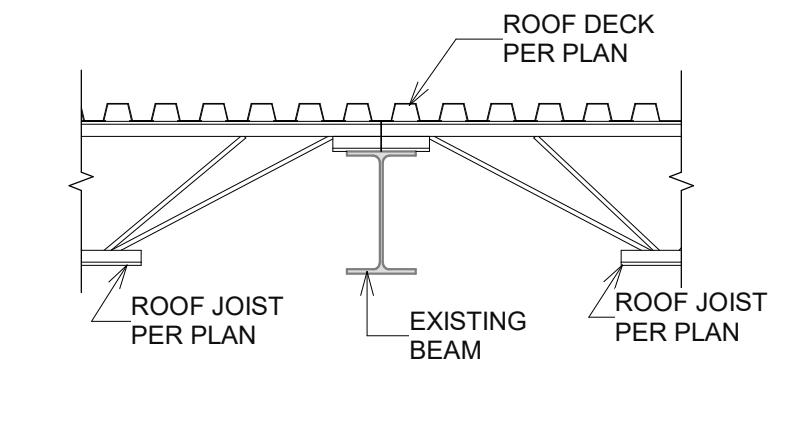
7 Section 7  
3/4" = 1'-0"



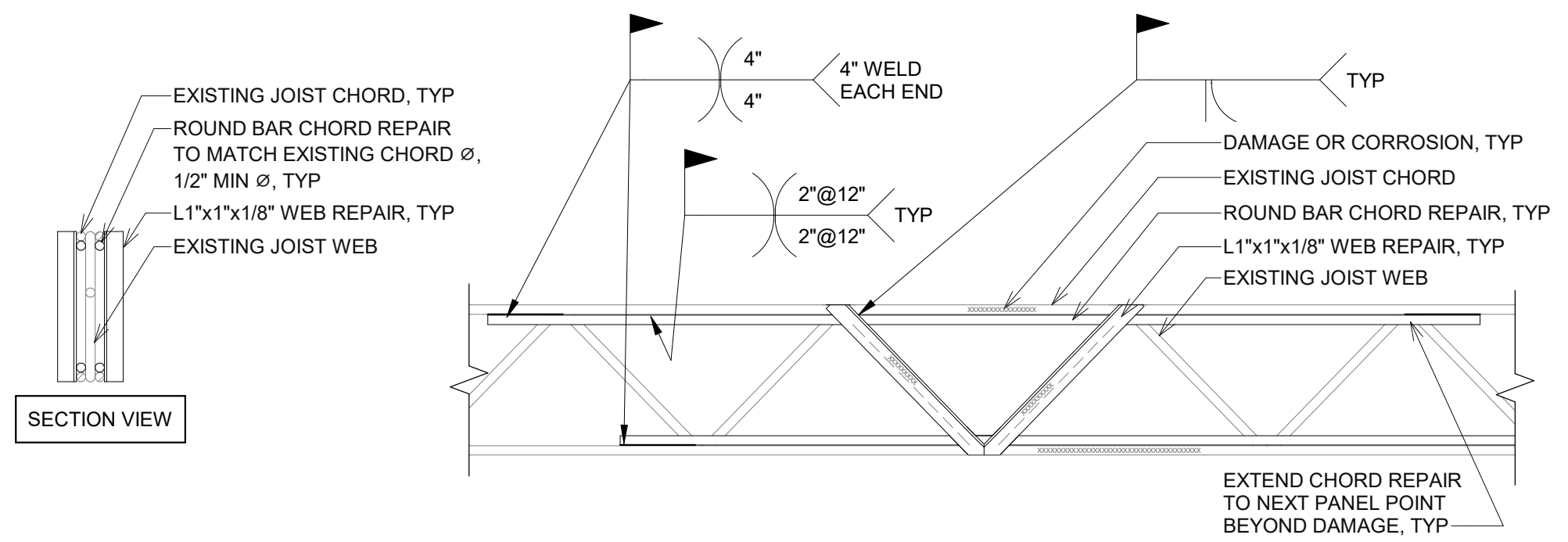
8 Section 8  
3/4" = 1'-0"



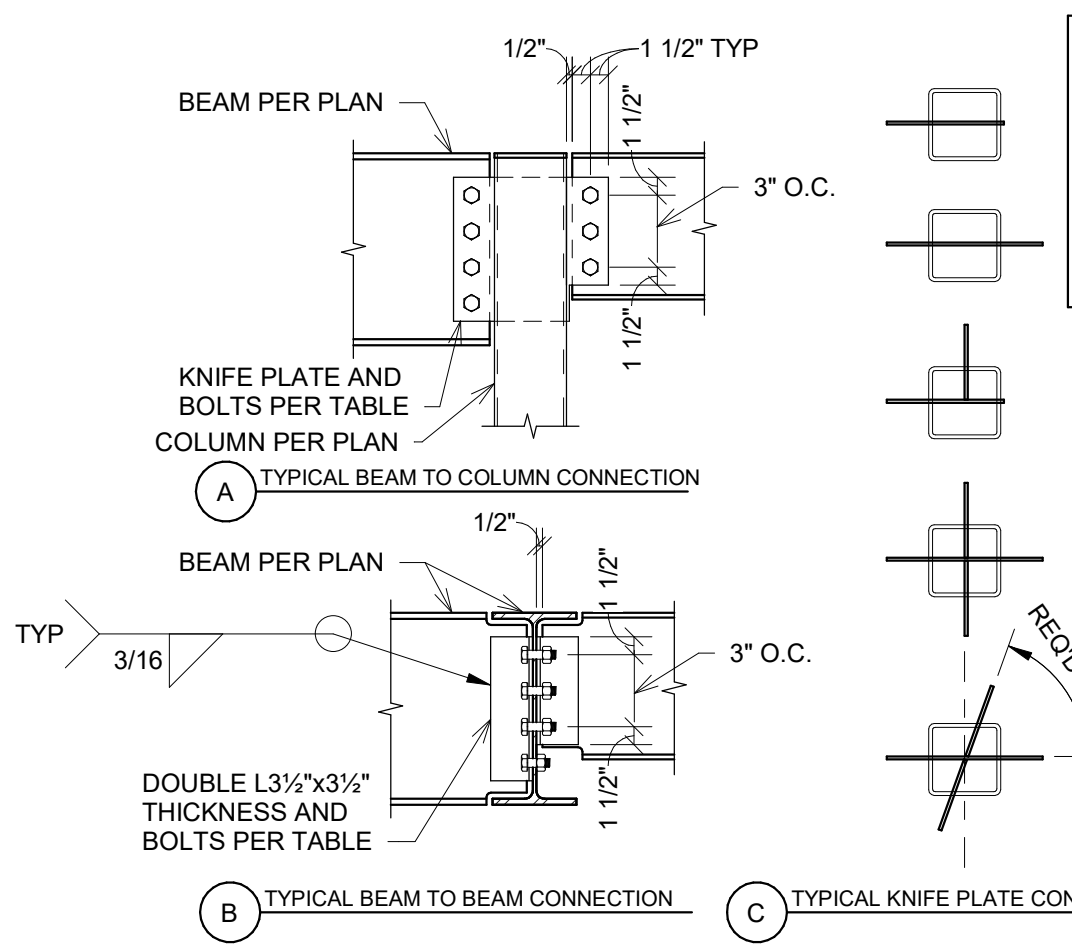
9 Section 9  
3/4" = 1'-0"



10 Section 10  
3/4" = 1'-0"



11 TYPICAL JOIST REPAIR DETAIL  
1 1/2" = 1'-0"



12 TYPICAL BEAM CONNECTION  
3/4" = 1'-0"

NOTES  
1. AT COLUMNS WITH MULTIPLE BEAMS, USE LARGER KNIFE PLATE SIZE.  
2. IN LIEU OF KNIFE PLATE IN COLUMN, FOR W8 AND W10 USE SHEAR TAB. FOR W12 AND LARGER USE ST4x11.5 WITH SAME QUANTITY OF BOLTS WITH FLANGE WELDED TO COLUMN TO BE ACCEPTED BY EOR ON SHOP DRAWINGS

BEAM SIZE	PLATE / L THICKNESS	QUANTITY 3/4" A325 BOLTS *
C8	5/16"	(2)
W8	5/16"	(2)
W10	5/16"	(2)
W12	5/16"	(3)
W14	5/16"	(3)
W16	3/8"	(4)
W18	3/8"	(5)
W21	7/16"	(6)

\*DOUBLE QUANTITY OF BOLTS AT DOUBLE ANGLE CONNECTIONS



REBAR AND CONCRETE REPAIR:  
1. REMOVE LOOSE AND SPALLING CONCRETE. ROUGHEN SURFACE TO REPAIR, AND CLEAN THOROUGHLY.  
2. REMOVE RUST AND CORROSION FROM EXISTING REINFORCING.  
3. COAT REPAIR AREA AND ALL SIDES OF REINFORCING WITH SIKA ARMATEC-110 EPOCEM BONDING PRIMER AND CORROSION PROTECTION.  
4. PATCH REPAIR AREA WITH SIKA SIKAREPAIR 223 PATCHING MATERIAL.

13 BASEMENT REPAIR DETAIL  
NTS

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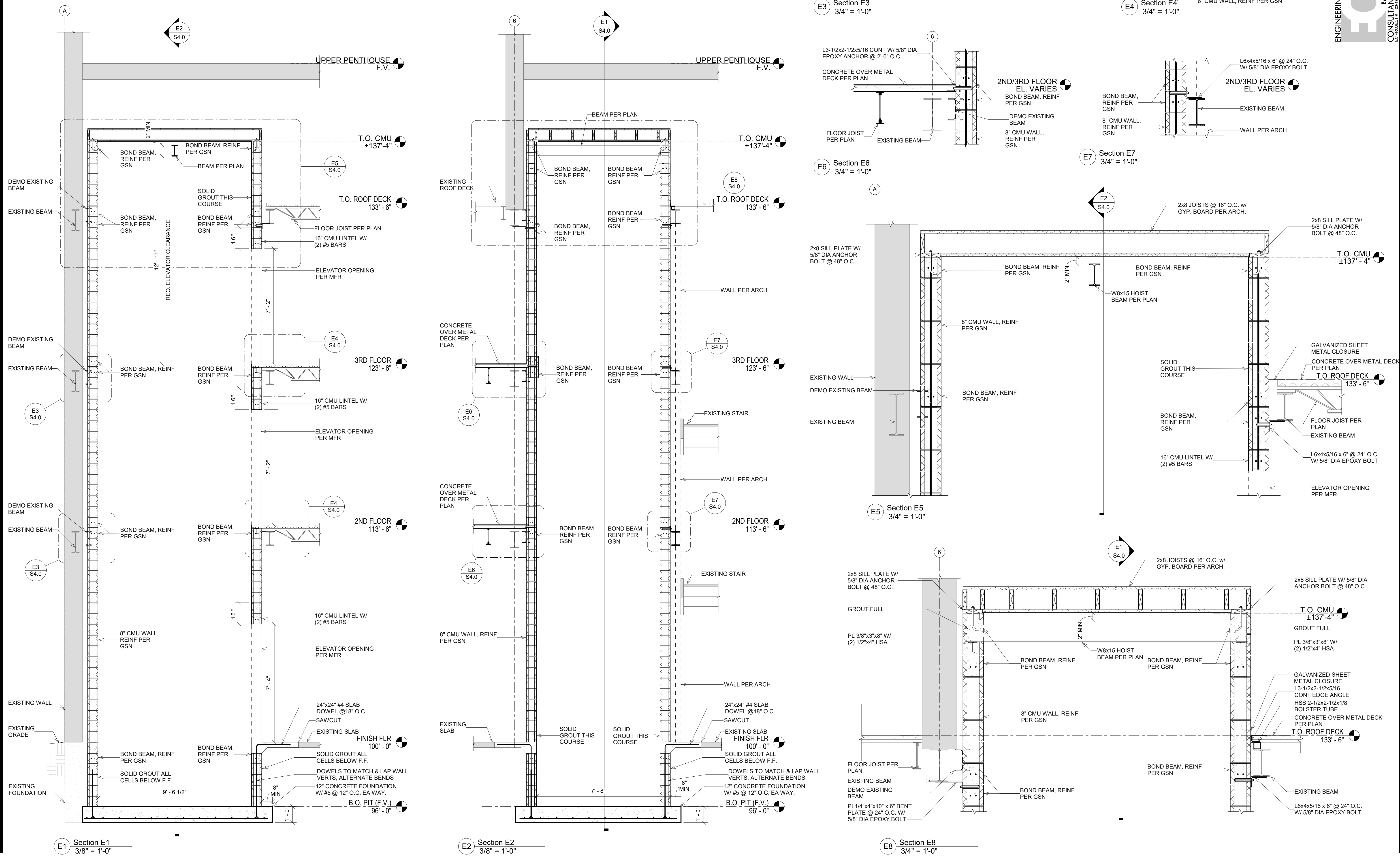

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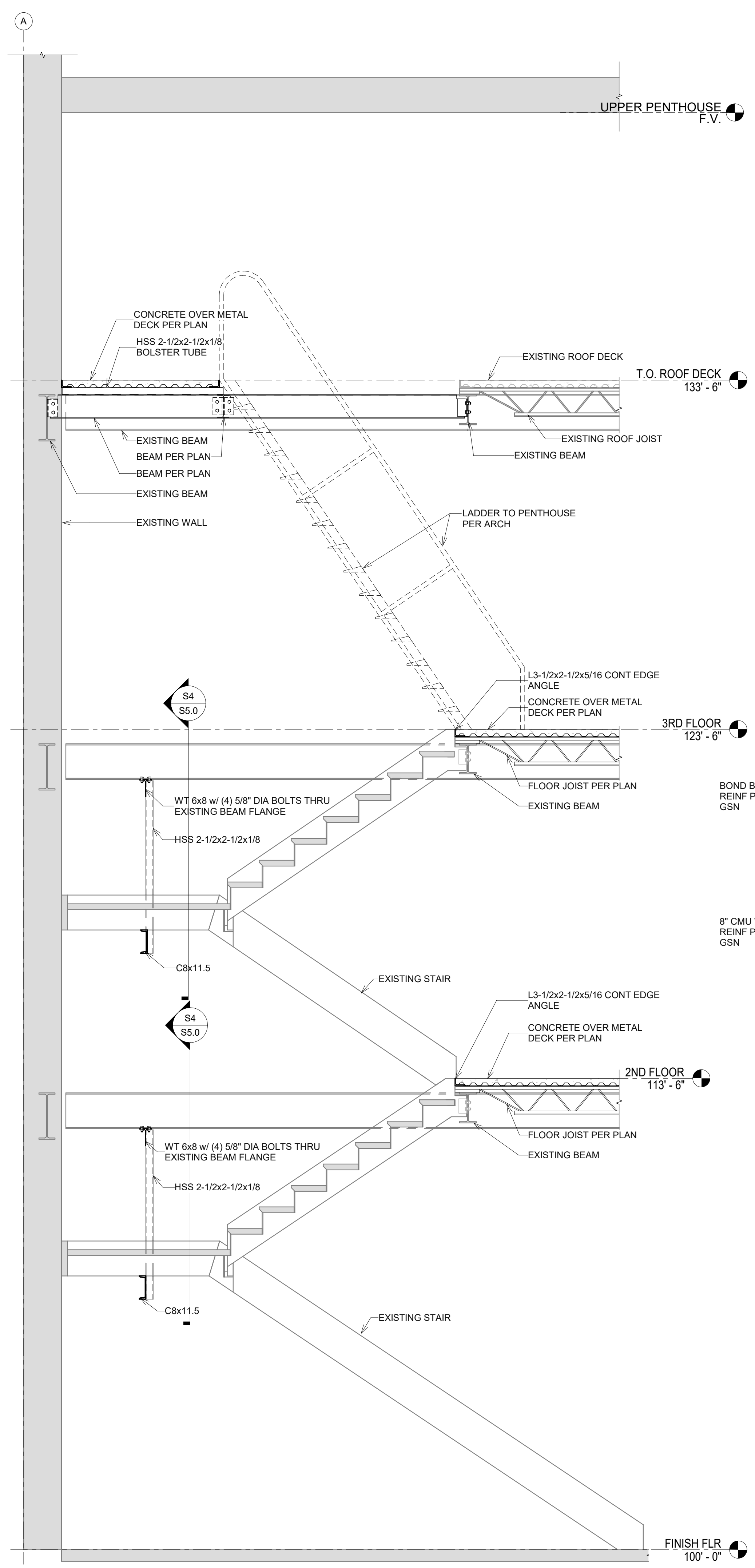

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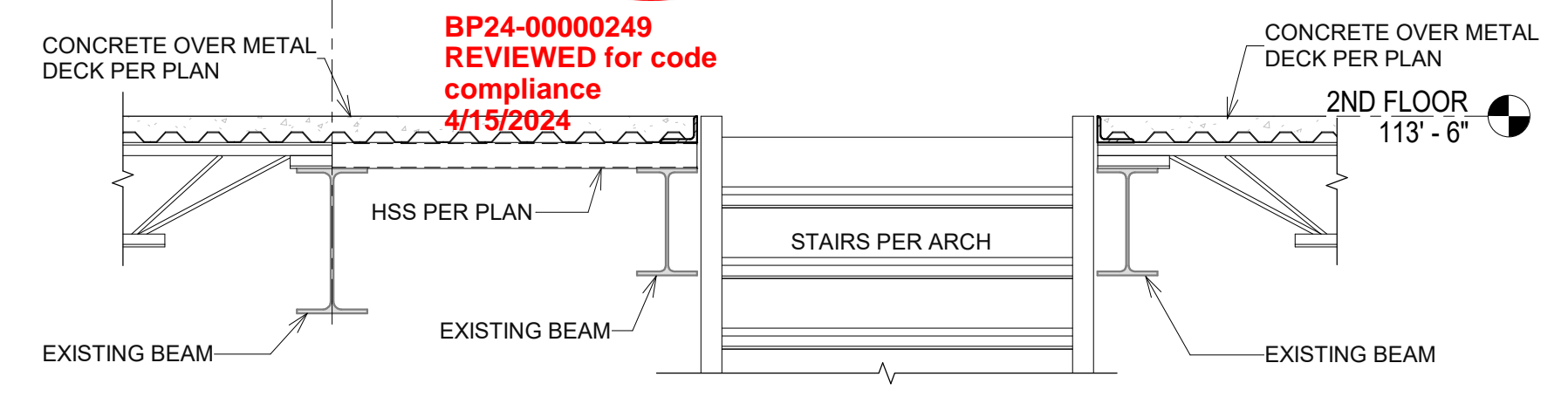
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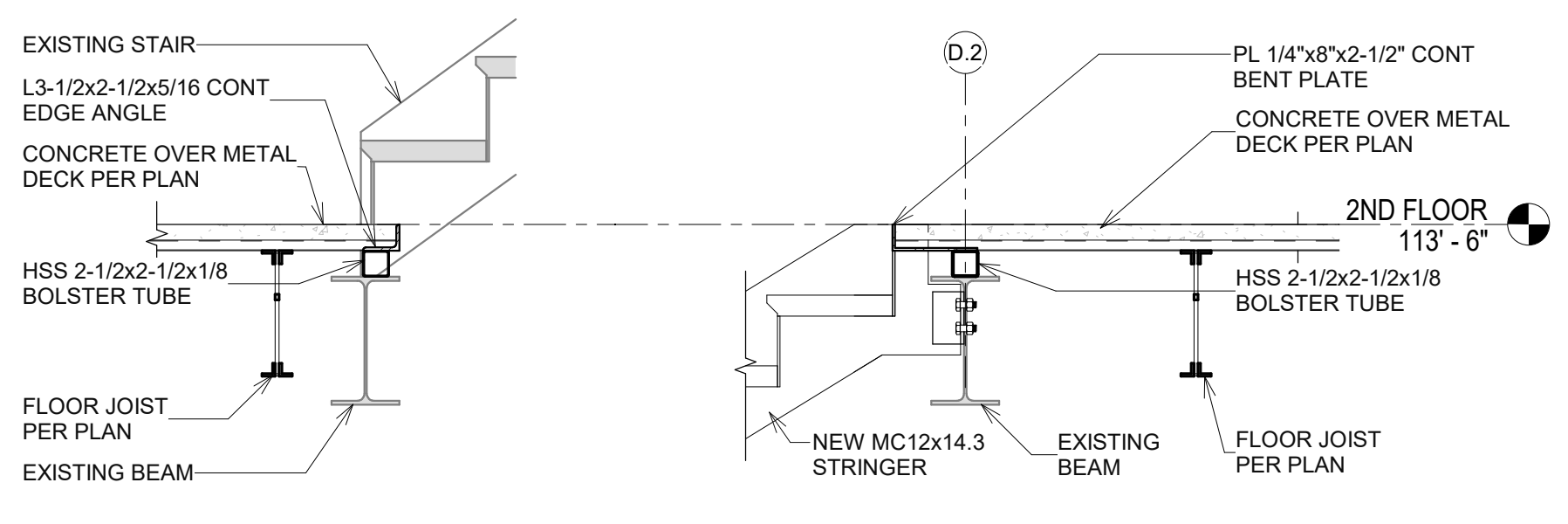
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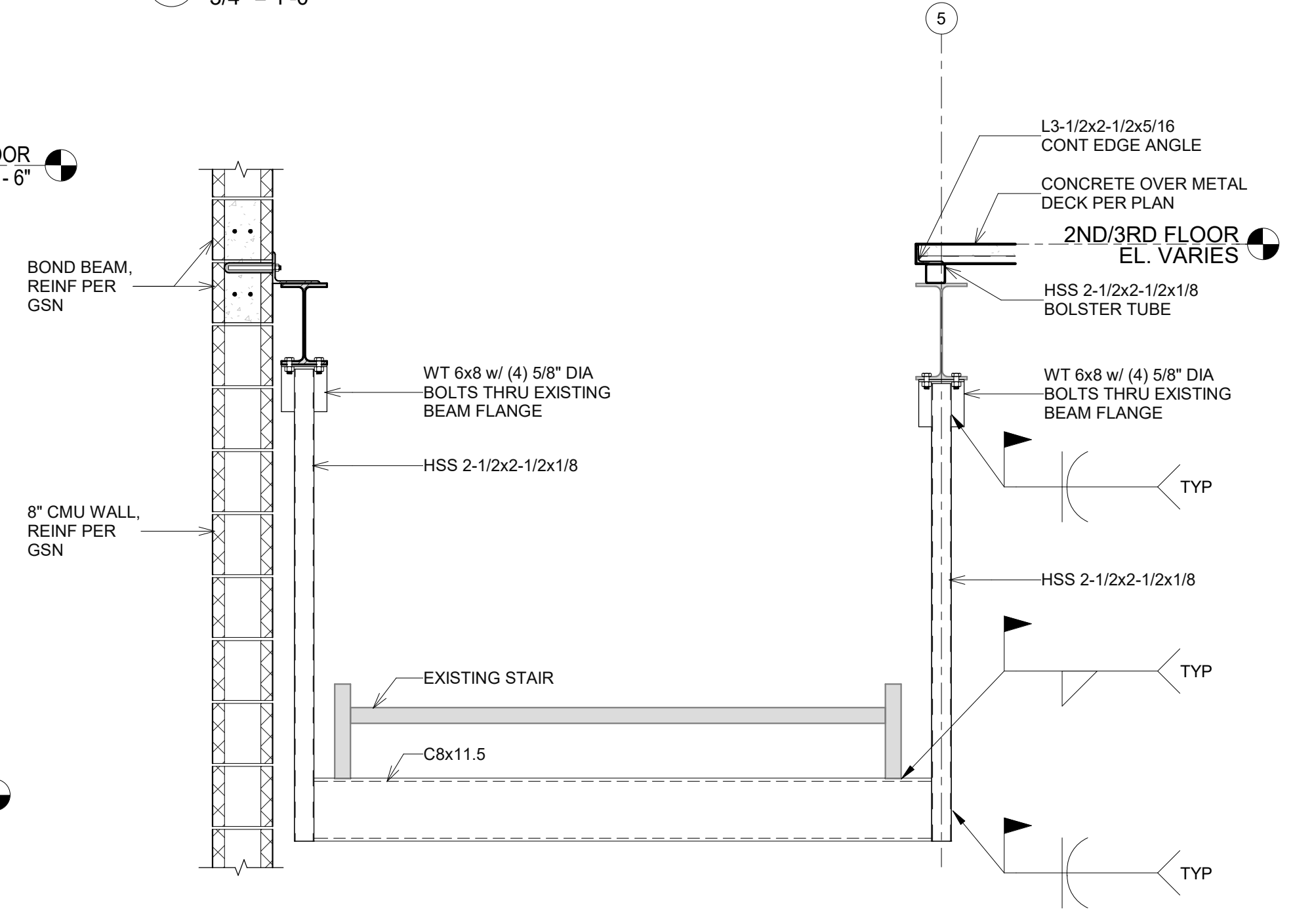
S1 Section S1  
1/2" = 1'-0"



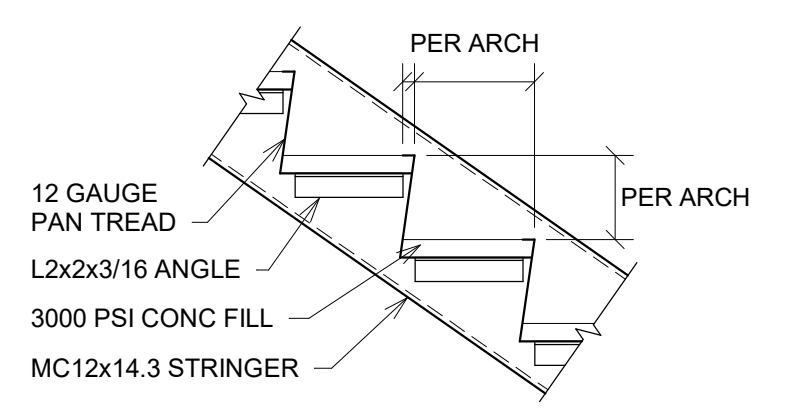
S2 Section S2  
3/4" = 1'-0"



S3 Section S3  
3/4" = 1'-0"



S4 Section S4  
3/4" = 1'-0"

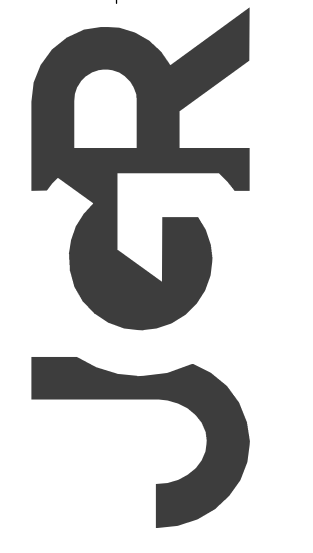


**STAIR NOTES**  
1. FINAL STAIR DETAILING PER STEEL FABRICATOR SUBJECT TO APPROVAL OF ARCHITECT AND ENGINEER OF RECORD.  
2. LANDING SUPPORT POSTS TO BE HSS3x3x1/4.

T1 TYPICAL STAIR DETAIL  
3/4" = 1'-0"

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