

GENERAL

- ### DESIGN LOADS

### EXISTING CONSTRUCTION

1. DESIGN ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF HAS BEEN ASSUMED. ALL EXTERIOR FOOTINGS TO BE 3'-0" BELOW FINISH GRADE UNO.
2. UNLESS NOTED OTHERWISE; CENTER COLUMN FOOTINGS ON COLUMN CENTERLINES, CENTER WALL FOOTINGS ON FOUNDATION WALLS.
3. SLAB ON GRADE SHALL BE UNDERLAIN BY VAPOR BARRIER AND 6 INCHES MINIMUM OF CRUSHED ROCK OR CONCRETE. REINFORCE ALL SLABS ON GRADE WITH #3 AT 18 INCHES EACH WAY TOP 1/3 OF SLAB UNLESS NOTED OTHERWISE. AT DROPPED OR DERESSED SLABS ON GRADE MAINTAIN GRAVEL THICKNESS, SLAB DEPTH, REINFORCEMENT AND REINFORCEMENT POSITION.
4. BACK FILL AROUND THE EXTERIOR FOUNDATION WALLS WITH A FREE DRAINING GRANULAR MATERIAL TO THE ELEVATION OF THE ROUGH GRADE. PLACEMENT OF BACKFILL IS NOT ALLOWED UNTIL THE SLAB ON GRADE IS IN PLACE AND THE MAIN FLOOR DIAPHRAGM IS COMPLETED. TUNNEL CONCRETE WORK, FREE STANDING AND FOUNDATION WALLS SHALL BE COMPLETE AND AT DESIGN STRENGTH BEFORE BACKFILL IS PLACED.

1. MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS  

FOOTINGS	3500 PSI MAX. W/C RATIO OF 0.50
INTERIOR SLABS ON GRADE	4000 PSI MAX. W/C RATIO OF 0.45
SLABS OVER STEEL DECK	3500 PSI MAX. W/C RATIO OF 0.45
EXPOSED CONCRETE SLABS AND GARAGE SLABS	4000 PSI MAX. W/C RATIO OF 0.45
FOUNDATION WALLS, WALLS, COLUMNS AND BEAMS	4000 PSI MAX. W/C RATIO OF 0.45
2. EXTERIOR EXPOSED CONCRETE SHALL HAVE 4 TO 6% ENTRAINMENT AIR. SLABS WITH HARD TROWELLED FINISH TO HAVE NO AIR ENTRAINMENT ADDED. COORDINATE WITH ARCHITECTURAL FOR FINISHES.
3. AGGREGATE FOR NORMAL WEIGHT CONCRETE SHALL MEET ASTM C33. MIX DESIGN SHALL CONTAIN A MINIMUM OF 30% COARSE AGGREGATES BY GRADATION REQUIREMENTS (NO.67 GRADING) SET FORTH IN ASTM C33.
4. NO ALUMINUM SHALL BE PLACED IN THE CONCRETE.

5. CONSTRUCTION TO BE IN ACCORDANCE WITH ACI 318-05 (R-05), "CHAPTER 3 FOR STANDARDS FOR TESTS & MATERIALS, CHAPTERS 4, 5, 6 & 7 FOR CONSTRUCTION REQUIREMENTS". REFER TO ACI 302.1R-04 FOR SLAB ON GRADE MIX DESIGN.
6. PIPE OR ELECTRICAL CONDUIT EMBEDDED IN CONCRETE SHALL NOT BE LARGER IN OUTSIDE DIAMETER AT ITS WIDEST (OR FITTING) THAN 1/3 THE THICKNESS OF THE SLAB OR WALL. SLEEVES, CONDUIT, OR PIPES THROUGH SLABS AND WALLS SHALL BE PLACED NO CLOSER THAN THREE DIAMETERS ON CENTER AND THEY DO NOT DISPLACE REINFORCING. DO NOT CUT HOLES IN CONCRETE SLABS, BEAMS, COLUMNS, OR WALLS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
7. LOCATION OF ALL CONSTRUCTION AND CONTROL JOINTS SHALL BE LOCATED AND DETAILED ON SHOP DRAWINGS AND ARE SUBJECT TO ENGINEERS APPROVAL. IF SLAB ON GRADE CONTROL JOINTS ARE NOT SPECIFICALLY LOCATED ON DRAWINGS, PROVIDE CONTROL JOINTS AT 10'-0" ON CENTER MAXIMUM WITH A LENGTH TO WIDTH RATIO OF 1.5. PROVIDE (2) #4x4'-0" AT ALL NON-CONTINUOUS CONTROL JOINTS. PROVIDE (2) #4x4'-0" AND (1) #4x24"x24" CORNER BAR AT ALL REINTRANT CORNERS OF SLAB ON GRADE.
8. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATION AND DIMENSION OF CONCRETE REVEALS, NOTCHES, REGULETS, DRIPS, PADS, CURBS, CHAMFERS BLOCKOUTS AT DOORWAYS, AND ALL OTHER PROJECT REQUIREMENTS NOT SHOWN ON STRUCTURAL DRAWINGS. CHAMFER ALL EXPOSED CORNERS OF BEAMS, COLUMNS, JOISTS AND WALLS, SUBJECT TO ARCHITECTS APPROVAL.

1. TRUSS TYPE MASONRY JOINT REINFORCEMENT: W1.7 (9 GAGE), ASTM A1064, FY=70,000 PSI
2. WELDED WIRE REINFORCING (WWR): ASTM A82 AND A185
3. DEFORMED BARS (REBAR): ASTM A615, GRADE 40 FOR #3; GRADE 60 FOR #4 AND LARGER; ASTM A706 FOR WELDED CONDITIONS.
4. LAP SPLICES:
 

MASONRY:	48-BAR DIAMETERS AT CELLS WITH SINGLE BAR (HORIZ. AND VERT.)
CONCRETE:	64-BAR DIAMETERS AT CELLS WITH TWO BARS (HORIZ. AND VERT.)
WELDED WIRE FABRIC:	CLASS "B" LAP SPLICE, TYPICAL UNLESS NOTED OTHERWISE.
	WIRE SPACING +2"
5. CONCRETE COVER FOR CAST-IN-PLACE AND NON-PRE-STRESSED CONCRETE SHALL BE AS SPECIFIED BELOW U.N.O. ON THESE DRAWINGS:

6. SECURELY TIE ALL REINFORCING IN PLACE WITH DOUBLE ANNEALED 16-GAUGE IRON WIRE OR APPROVED CLIPS PRIOR TO CONCRETE OR GROUT PLACEMENT.
7. SUBMIT SHOP DRAWINGS OF REINFORCING STEEL FOR REVIEW BY THE ARCHITECT AND ENGINEER PRIOR TO FABRICATION.

1. UNLESS NOTED OTHERWISE ON THESE DRAWINGS, REINFORCE CMU WALLS AS FOLLOWS:

- 8" CMU WALLS - INTERIOR  
#5 VERT @ 48" O.C. CENTER OF WALL  
(2) #5 VERT AT EACH CORNER OF WALL  
TRUSS TYPE HORIZ JOINT REINFORCEMENT @ 16" O.C.
- SOLID GROUT AT REINFORCED CELLS ONLY UNO  
PROVIDE (2) TYPICAL VERT FULL HEIGHT JAMB STEEL EACH SIDE OF OPENINGS  
PROVIDE (2) #4 HORIZ BOND BEAM AT FLOOR AND ROOF LEVELS AND TOP OF WALL

1. LIGHT WEIGHT CONCRETE SHALL BE 1500 PSI. ASTM C90 CONCRETE MASONRY UNITS WITH NET AREA MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI. ASTM C270 MORTAR TYPE S, MINIMUM COMPRESSIVE STRENGTH OF 28 DAYS. ASTM C476 GROUT WITH MINIMUM COMPRESSIVE STRENGTH OF 28 DAYS. AREA COMPRESSIVE STRENGTH OF MASONRY,  $F_m = 1,500$  PSI.

2. FILL ALL CELLS WITH GROUTING AND FORCING WITH GROUT IN LIFTS NOT EXCEEDING 4'-0" IN HEIGHT. FILL OTHER CELLS WITH GROUT AS INDICATED ON DRAWINGS. ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING WITH VERTICAL BARS HELD AT TOP, BOTTOM AND 120 BAR DIAMETERS MAXIMUM. ON CENTER GROUT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION DURING PLACING.

3. CONCRETE SHALL PROVIDE BRACING FOR MASONRY WALLS, AS REQUIRED, UNTIL CONNECTION TO FLOOR AND/OR ROOF DIAPHRAGMS ARE COMPLETED.

4. STRENGTH OF MASONRY ASSEMBLY SHALL BE DETERMINED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH SECTION 2105.2.2.1 OF THE 2006 IBC.
5. PROVIDE HORIZONTAL TRUSS-TYPE REINFORCING AT 16" ON CENTER MAXIMUM UNO.
6. NON-BEARING INTERIOR PARTITIONS SHALL STOP 1" BELOW STRUCTURAL SLABS OR STEEL FRAMING U.N.O.
7. WHERE BOND BEAMS INTERSECT AT CORNERS AT DIFFERENT ELEVATIONS, RUN EACH BOND BEAM AROUND CORNER FOR TWO BLOCK LENGTHS MINIMUM.
8. WHERE BOND BEAMS INTERSECT PARALLEL AT DIFFERENT ELEVATIONS, LAP BOND BEAMS FOUR BLOCK LENGTHS MINIMUM.
9. PROVIDE CORNER AND INTERSECTION BARS IN ALL BOND BEAMS.
10. CONTROL AND EXPANSION JOINTS SHALL BE PROVIDED IN MASONRY WALLS AT 30' MAXIMUM PER TYPICAL MASONRY DETAILS. SEE ARCHITECTURAL FOR LOCATIONS.
11. PROVIDE (2) #4 VERTICAL EACH SIDE OF ALL OPENINGS IN MASONRY WALLS UNO. COORDINATE WITH LINTEL SCHEDULE AND PROVIDE GREATER REINFORCING REQUIREMENTS.
12. PROVIDE (2) #4 VERTICAL AT ALL WALL CORNERS, ENDS AND INTERSECTIONS UNO. COORDINATE WITH LINTEL SCHEDULE AND PROVIDE GREATER REINFORCING REQUIREMENTS.
13. PROVIDE BOND BEAM WITH (2) #4 CONTINUOUS BENEATH ALL SLAB AND BEAM BEARINGS UNO.
14. PROVIDE 1/2" AIR GAP AROUND SIDES, TOP AND END OF WOOD STRUCTURAL MEMBERS BEARING ON MASONRY.

1. STEEL JOISTS BEARING CONNECTIONS SHALL BE BY WELDED UNO. PROVIDE ERECTION BOLTS AT LOCATION REQUIRED BY SJI SPECIFICATIONS. WHERE JOIST BEARING CONDITIONS REQUIRE NON-STANDARD BEARING ENDS, JOIST FABRICATOR SHALL PROVIDE SPECIAL BEARING ENDS AS REQUIRED TO ACCOMMODATE SUCH CONDITIONS.

2. SUSPENSION OF ANY MISCELLANEOUS ITEMS FROM THE JOISTS SHALL BE ONLY AT TOP AND BOTTOM CHORD PANEL POINTS UNLESS SPECIFICALLY DETAILED OTHERWISE.
3. JOIST FABRICATOR SHALL PROVIDE JOIST BRIDGING PER SJI RECOMMENDATIONS. ALL JOISTS AND JOIST BRIDGING SHALL BE DESIGNED TO RESIST THE UPLIFT PRESSURES SHOWN ON THE PLANS.
4. UNLESS NOTED OTHERWISE:  
ALL ROOF JOISTS TO BE DESIGNED FOR A 150 LB ADD-LOAD AND 150 LB BEND-CHECK  
ALL FLOOR JOISTS TO BE DESIGNED FOR A 300 LB ADD-LOAD AND 300 LB BEND-CHECK
5. JOIST MANUFACTURER MAY NOT DESIGN JOISTS FOR LESS THAN LOADS SPECIFIED IN THE SJI CAPACITY TABLES FOR JOIST DESIGNATIONS SHOWN ON PLANS.

1.	STEEL CONSTRUCTION MANUAL, 14TH EDITION MATERIAL SPECIFICATIONS U.N.O.	
WIDE FLANGE AND S SHAPES		ASTM A992, FY=50KSI
CHANNELS, ANGLES, PLATES AND BARS		ASTM A36, FY=36KSI
HOLLOW STRUCTURAL SHAPES (HSS)		ASTM A500 GR. B, FY=46KSI
PIPE		ASTM A53, GR. B, FY=35KSI
STRUCTURAL BOLTS (U.N.O.)		ASTM A325
MACHINE BOLTS (WHERE NOTED)		ASTM A307
ANCHOR BOLTS AND RODS AND THREADED RODS		ASTM A1554 GRADE 36KSI
HIGH STRENGTH ANCHOR BOLTS AND RODS (AS NOTED)		ASTM A1554 GRADE 105KSI
HEADED OR THREADED STUD ANCHORS (H.S.A. OR T.S.A.)		ASTM A108-69T
DEFORMED BAR ANCHORS (D.B.A.)		ASTM A476 OR ASTM A706
WELDING ELECTRODES		E70XX
NON-SHRINK GROUT (7,000 PSI)		ASTM C1107, GR. A
POWDER ACTUATED FASTENER (PAF OR PDF)		HILTI X-U (0.157" DIA)
EXPANSION BOLTS (CONCRETE)		HILTI KWIK BOLT TZ
EXPANSION BOLTS (MASONRY)		HILTI KWIK BOLT W3
EPOXY ADHESIVE - CONCRETE		HILTI HIT-HY 700
EPOXY ADHESIVE - MASONRY		HILTI HIT-HY 70 W SCREEN TUBE

2. ALL STRUCTURAL STEEL ERECTION AND FABRICATION SHALL BE ACCORDING TO THE CURRENT EDITION OF AISC "SPECIFICATIONS FOR DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS".
3. ALL STRUCTURAL BOLTED CONNECTIONS SHALL BE ACCORDING TO THE CURRENT EDITION OF RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS" FOR SNUG TIGHTENED, PRETENSIONED, OR SLIP-CRITICAL JOINTS. ALL STRUCTURAL BOLTED CONNECTIONS TO BE SNUG TIGHTENED UNO. FOR SLIP-CRITICAL JOINTS, AS NOTED, THE USE OF TENSION INDICATING WASHERS OR TWIST-OFF BOLT ASSEMBLIES SHALL BE PERMITTED ONLY ACCORDING TO THE ABOVE MENTIONED STANDARD.
4. ALL WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.1. ALL WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS. ALL WELDING OF STRUCTURAL STEEL SHALL BE PERFORMED IN THE SHOP WHENEVER PRACTICAL. AN EFFORT HAS BEEN MADE TO INDICATE WELDS THAT CAN BE OR SHOULD BE FIELD WELDED. IT IS, HOWEVER, THE FABRICATORS RESPONSIBILITY TO DECIDE WHERE AND HOW THE WELDING IS TO BE ACCOMPLISHED TO ACHIEVE THE INTENDED RESULT.
5. COMPLETE JOINT PENETRATION (CJP) WELDING. PROVIDE BACKER BARS. RUN OFF TABS, AND ACCESS HOLES PER AWS D1.1. BACKER BARS SHALL BE REMOVED AFTER WELDING. THE ROOT WELD BACK GOUNG AND REPAIRED IF NECESSARY AND REINFORCED WITH A FILLET. RUN OFF TABS SHALL BE REMOVED AFTER WELDING WITH THE FLANGE EDGE GROUND SMOOTH.
6. STEEL FABRICATOR SHALL BE AN AISC CERTIFIED SHOP FOR CATEGORY 1 STEEL STRUCTURES AND SHALL MAINTAIN DETAILED QUALITY CONTROL PROCEDURES.

BEAMS SHALL BE FABRICATED FOR PLACEMENT OF NATURAL CAMBER UP.
7. STRUCTURAL STEEL SUPPLIER SHALL FURNISH COLUMN ANCHOR RODS.
8. HOLES IN STEEL SHALL BE DRILLED OR PUNCHED. ALL SLOTTED HOLES SHALL BE PROVIDED WITH SMOOTH EDGES. BURNING OF HOLES AND TORCH CUTTING AT THE SITE IS NOT PERMITTED. PROVIDE CONNECTIONS REQUIRED FOR ATTACHMENT OF WOOD AND STEEL MEMBERS.
9. USE CONNECTIONS AS DETAILED ON PLANS. WHEREVER CONNECTIONS ARE NOT DETAILED, FABRICATOR SHALL REQUEST ENGINEER TO SUPPLY CONNECTION DETAIL.
10. ALL COLUMNS, ANCHOR BOLTS, BASE PLATES, ETC., HAVE BEEN DESIGNED FOR THE FINAL COMPLETED CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADING OR OVERLOAD DURING STEEL ERECTION AND CONSTRUCTION. CONFORMANCE TO OR DEVIATION FROM ALLOWABLE CAPACITIES DURING STEEL ERECTION IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR (SEE GENERAL SECTION OF G.S.N.)
11. PRIOR TO GROUTING, COLUMNS SHALL BE ERECTED AND ALIGNED AS TO PLUMBNESS AND ELEVATION BY MEANS OF STEEL SHIMS OR LEVELING NUTS UNDER THE BASE PLATES. SETTING PLATE SHALL ONLY BE USED AS TEMPLATES TO LOCATE ANCHOR BOLTS DURING CONCRETE PLACEMENT.
12. STRUCTURAL STEEL PERMANENTLY EXPOSED TO VIEW SHALL RECEIVE COMMERCIAL BLAST CLEANING. PRIME WITH 5 MILS EPOXY PRIMER, TOP COAT SHALL BE 3 DRY MILS ALIPHATIC URETHANE THIN FILM. ARCHITECT TO APPROVE COATING AND COLOR. DAMAGE DURING TRANSPORT, ERECTION, AND FIELD WELDING PROCESSES SHALL BE REPAIRED TO MATCH THE SHOP APPLIED COATING.

1. ALL WOOD BEARING ON CONCRETE OR MASONRY, IF LESS THAN 4'-0" ABOVE FINISH GRADE, SHALL BE PRESSURE TREATED. WHEREVER NECESSARY TO CUT OR DRILL TREATED LUMBER, TREAT THE CUT OR BORED SURFACES WITH TWO HEAVY COATS OF THE SAME PRESERVATIVE AS USED IN ORIGINAL TREATMENT.

2. PLYWOOD SHEATHING SHALL CONFORM TO THE CURRENT EDITION OF THE U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARD 1 OR 2 (DOC PS 1 OR 2) OR THE APA PANEL DESIGN SPECIFICATION (PDS) AND SHALL BE INSTALLED IN STAGGERED PATTERN.

3. BOLT HOLES IN WOOD SHALL BE DRILLED 1/16" MAXIMUM OVERSIZE. HOLES FOR SCREWS AND LAG SCREWS SHALL BE FIRST BORED FOR THE SAME DEPTH AND DIAMETER OF THE SHANK, THEN THE REMAINDER OCCUPIED BY THE THREAD PORTION SHALL BE BORED NOT LARGER IN DIAMETER THAN THE ROOF OF THE THREAD. ALL SCREWS SHALL BE SCREWED NOT DRIVEN INTO PLACE. PROVIDE WASHERS UNDER ALL NUTS AND HEADS OF BOLTS AND LAG SCREWS.
4. PROVIDE SOLID BLOCKING AT MID-HEIGHT OF ALL WALLS U.N.O.
5. PROVIDE SOLID BLOCKING BETWEEN JOISTS AT ALL SUPPORTS.
6. WOOD FRAMING AND CONSTRUCTION SHALL CONFORM TO THE CURRENT EDITION OF THE NATIONAL DESIGN SPECIFICATION (NDS) FOR WOOD CONSTRUCTION.
7. ALL COLUMNS SHOWN ON STRUCTURAL DRAWINGS SHALL BE CONTINUOUS U.N.O.
8. SET ALL JOISTS WITH CROWN UP.
9. TYPICAL FRAMING ANCHORS SHALL BE "SIMPSON STRONG TIE" OR APPROVED EQUIVALENT AS INDICATED ON DRAWINGS. INSTALL AND CONNECT PER MANUFACTURER RECOMMENDATIONS.
10. CONNECTORS, ANCHORS, AND FASTENERS ATTACHED TO PRESSURE TREATED WOOD TREATED WITH ACQ-C OR ACQ-D SHALL BE GALVANIZED AND SHALL MEET ONE OF THE FOLLOWING SPECIFICATIONS: ASTM-A653-G-018S OR GREATER, ASTM-A123-2.0 OZ/F2 MIN, ASTM-A153, ASTM-B695-CLASS 110.
11. IF WOOD TREATMENT IS OTHER THAN LISTED ABOVE, CONTACT THE TREATMENT SUPPLIER FOR CORROSION PROTECTION REQUIREMENTS AND SUBMIT TO ARCHITECT/ENGINEER FOR APPROVAL. CONNECTIONS SHOULD NOT INCORPORATE DISSIMILAR METALS OR METALLIC COATINGS IN CONTACT WITH EACH OTHER.

#	AT	#
AB	ANCHOR	
ADBL	ANCHOR BOLT	
AESS	ADDITIONAL	
	ARCHITECTUALLY EXPOSED	
	STRUCTURAL STEEL	
ALT	ALTERNATE	
ARCH	ARCHITECTURAL	
ATTM	ATTACHMENT	
BLDG	BUILDING	
BLKG	BLOCKING	
BOT	BOTTOM	
BSMT	BASEMENT	
BTWN	BETWEEN	
CFS	COLD FORMED STEEL	
CJ	CONTROL OR CONSTRUCTION JOINT	
CJP	COMPLETE JOINT PENETRATION WELD	
CLR	CENTERLINE	
CMU	CONCRETE MASONRY UNITS	
COL	COLUMN	
CONC	CONCRETE	
CONN	CONNECTION	
CONT	CONTINUOUS	
COORD	COORDINATE	
DBA	DEFORMED BAR ANCHOR	
DET	DETAIL	
DIA	DIAMETER	
DIM	DIMENSION	
DIR	DIRECTION	
DF-L	DOUGLAS FIR-LARCH	
EA	EACH	
EF	EACH FACE	
EMBED	EMBEDDED	
EN	EDGE NAILING	
EOR	ENGINEER OF RECORD	
EQ	EQUAL	
EW	EACH WAY	
EXIST	EXISTING	
EXP	EXPANSION	
FDN	FOUNDATION	
FIN	FINISH	
FLR	FLOOR	
FV	FIELD NAILING	
FRP	FIBER-REINFORCED POLYMER	
FTG	FOOTING	
GA	GAUGE	
GR	GRADE	
HK	HOOK	
HORIZ	HORIZONTAL	
HS	HIGH STRENGTH	
HSA	HEADED STUD ANCHOR	
HSS	HOLLOW STRUCTURAL SHAPE	
IBC	INTERNATIONAL BUILDING CODE	
ID	INSIDE DIAMETER	
INFO	INFORMATION	
LBS	POUNDS	
LG	LONG	
LLH	LONG LEG HORIZONTAL	
LLV	LONG LEG VERTICAL	
LSL	LAMINATED STRAND LUMBER	
LVL	LAMINATED VENEER LUMBER	
MAX	MAXIMUM	
MECH	MECHANICAL	
MFR	MANUFACTURER	
MIN	MINIMUM	
MTL	METAL	
NIT	NOT IN CONTRACT	
NS	NON-SHRINK	
OC	ON CENTER	
OD	OUTSIDE DIAMETER	
OPP	OPPOSITE	
OSB	ORIENTED STRAND BOARD	
PAER	PARTIALLY ACTUATED FASTENER	
PEMB	PRE-ENGINEERED METAL BUILDING	
PL	PLATE	
PLF	POUNDS PER LINEAR FOOT	
PSF	POUNDS PER SQUARE FOOT	
PSI	POUNDS PER SQUARE INCH	
PSL	PARALLEL STRAND LUMBER	
PT	POINT	
QTY	QUANTITY	
REINF	REINFORCING	
REIN	REINFORCER	
REQ'D	REQUIRED	
RTU	ROOF TOP UNIT	
SCHD	SCHEDULE	
SIM	SIMILAR	
SLV	SHORT LEG VERTICAL	
SOG	SLAB-ON-GRADE	
SPF	SPRUCE-PINE-FIR	
SQ	SQUARE	
STD	STANDARD	
T&B	TOP AND BOTTOM	
THK	THICK	
TOP	TOP OF FOOTING	
TOM	TOP OF MASONRY	
TOS	TOP OF STEEL	
TOW	TOP OF WALL	
TSA	THREADED STUD ANCHOR	
TYP	TYPICAL	
VERT	VERTICAL	
UNO	UNLESS NOTED OTHERWISE	
WF	WIDE FLANGE	
WWR	WELDED WIRE REINFORCING	

DATE:	1-16-2024
JOB:	22-3281
SHEET NO.:	







BP24-00000249  
REVIEWED for code  
compliance  
5/30/2024

STATEMENT OF SPECIAL INSPECTIONS

1.

THIS STRUCTURAL STATEMENT OF SPECIAL INSPECTIONS IS INCLUDED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE BUILDING CODE. THE TABLE OF REQUIRED SPECIAL INSPECTIONS IDENTIFIES THE STRUCTURAL ITEMS TO BE INSPECTED APPLICABLE TO THIS PROJECT AS WELL AS IDENTIFYING THE APPROVED AGENCIES TO BE RETAINED FOR CONDUCTING THESE INSPECTIONS AND TESTS.
2.

SPECIAL INSPECTIONS SHALL BE PERFORMED BY PERSONNEL AS INDICATED IN THE TABLE.  
  
S.I. SPECIAL INSPECTOR  
G.E. GEOTECHNICAL ENGINEER
3.

IF SPECIAL INSPECTION IS WAIVED BY THE GOVERNING AUTHORITIES, THE GENERAL CONTRACTOR SHALL PROVIDE THE ENGINEER OF RECORD A COPY OF THE BUILDING OFFICIALS WRITTEN EXEMPTION FOR SPECIAL INSPECTION PRIOR TO STARTING WORK.

SCHEDULE OF SPECIAL INSPECTION SERVICES			
MATERIAL / ACTIVITY	SERVICE	INSPECTION	
		EXTENT	AGENT*
1705.2 Steel Construction			
1. Fabricator and erector documents (Verify reports and certificates as listed in AISC 360, chapter N, paragraph 3.2 for compliance with construction documents)	Submittal Review	Each submittal	S.I.
2. Material verification of structural steel	Shop (3) and field inspection	Periodic	S.I.
3. Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Periodic	S.I.
4. Verify member locations, braces, stiffeners, and application of joint details at each connection comply with construction documents	Field inspection	Periodic	S.I.
5. Structural steel welding:			S.I.
a. Inspection tasks Prior to Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-1)	Shop (3) and field inspection	Observe or Perform as noted (4)	S.I.
b. Inspection tasks During Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-2)	Shop (3) and field inspection	Observe (4)	S.I.
c. Inspection tasks After Welding (Observe, or perform for each welded joint or member, the QA tasks listed in AISC 360, Table N5.4-3)	Shop (3) and field inspection	Observe or Perform as noted (4)	S.I.
6. Structural steel bolting:	Shop (3) and field inspection		S.I.
a. Inspection tasks Prior to Bolting (Observe, or perform tasks for each bolted connection, in accordance with QA tasks listed in AISC 360, Table N5.6-1)		Observe or Perform as noted (4)	S.I.
b. Inspection tasks During Bolting (Observe the QA tasks listed in AISC 360, Table N5.6-2)		Observe (4)	S.I.
2) Snug-tight joints		Periodic	S.I.
c. Inspection tasks After Bolting (Perform tasks for each bolted connection in accordance with QA tasks listed in AISC 360, Table N5.6-3)		Perform (4)	S.I.
1705.2.2 Steel Construction Other Than Structural Steel			
1. Material verification of cold-formed steel deck:			S.I.
a. Identification markings	Field inspection	Periodic	S.I.
b. Manufacturer's certified test reports	Submittal Review	Each submittal	S.I.
2. Connection of cold-formed steel deck to supporting	Shop (3) and field inspection		S.I.
a. Welding		Periodic	S.I.
b. Other fasteners (in accordance with AISC 360, Section N6)			S.I.
1) Verify fasteners are in conformance with approved submittal		Periodic	S.I.
2) Verify fastener installation is in conformance with approved submittal and manufacturer's recommendations		Periodic	S.I.
1705.3 Concrete Construction			
1. Inspection of reinforcing steel installation (see 1705.2.2 for welding)	Shop (3) and field inspection	Periodic	S.I.
3. Inspection of anchors cast in concrete	Shop (3) and field inspection	Periodic	S.I.
4. Inspection of anchors and reinforcing steel post-installed in hardened concrete: Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, concrete minimum thickness, anchor embedment and tightening torque	Field inspection	Periodic or as required by the research report issued by an approved source	S.I.
5. Verify use of approved design mix	Shop (3) and field inspection	Periodic	S.I.
6. Fresh concrete sampling, perform slump and air content tests and determine temperature of concrete	Shop (3) and field inspection	Continuous	S.I.
7. Inspection of concrete placement for proper application techniques	Shop (3) and field inspection	Continuous	S.I.
8. Inspection for maintenance of specified curing temperature and techniques	Shop (3) and field inspection	Periodic	S.I.
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection	Periodic	S.I.
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports	Periodic	S.I.
1705.4 Masonry Construction			
Level - B - Quality Assurance			
(A) Level A, B and C Quality Assurance:			
1. Verify compliance with approved submittals	Field Inspection	Periodic	S.I.
(B) Level B Quality Assurance:			
1. Verification of f <sub>m</sub> and f <sub>AC</sub> prior to construction	Testing by unit strength method or prism test method	Periodic	S.I.
(D) Levels B and C Quality Assurance:			
1. Verification of Slump Flow and Visual Stability Index (VSI) of self-consolidating grout as delivered to the project	Field testing	Continuous	S.I.
2. Verify compliance with approved submittals	Field inspection	Periodic	S.I.
3. Verify proportions of site-mixed mortar, grout and prestressing grout for bonded tendons	Field Inspection	Periodic	S.I.
4. Verify grade, type, and size of reinforcement and anchor bolts, and prestressing tendons and anchorages	Field Inspection	Periodic	S.I.
5. Verify construction of mortar joints	Field Inspection	Periodic	S.I.
7. Verify grout space prior to grouting	Field Inspection	Level B - Periodic Level C - Continuous	S.I. S.I.
9. Verify size and location of structural masonry elements	Field Inspection	Periodic	S.I.
10. Verify type, size, and location of anchors, including details of anchorage of masonry to structural members, frames, or other construction	Field inspection	Level B - Periodic Level C - Continuous	S.I. S.I.
12. Verify preparation, construction, and protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F)	Field inspection	Periodic	S.I.
18. Prepare grout and mortar specimens	Field testing	Level B - Periodic Level C - Continuous	S.I. S.I.
19. Observe preparation of prisms	Field inspection	Level B - Periodic Level C - Continuous	S.I. S.I.
20. Inspection of anchors and reinforcing steel post-installed (epoxy, expansion, etc.): Per research reports including verification of anchor type, anchor dimensions, hole dimensions, hole cleaning procedures, anchor spacing, edge distances, location to masonry joints, masonry minimum thickness, anchor embedment and tightening torque	Field inspection	Periodic or as required by the research report issued by an approved source	S.I.
* INSPECTION AGENTS			
FIRM		ADDRESS	
1. G.E. Geotechnical Engineer			
2. S.I. Special Inspector - Not Yet Selected			
3.			
4.			
Notes: 1. The inspection and testing agent(s) shall be engaged by the Owner or the Owner's Agent, and not by the Contractor or Subcontractor whose work is to be inspected or tested. Any conflict of interest must be disclosed to the Building Official prior to commencing work. The qualifications of the Special Inspector(s) and/or testing agencies may be subject to the approval of the Building Official and/or the Design Professional.			
2. If the list of Inspection Agents is noted as "Not Yet Selected" the General Contractor shall coordinate submittal of special inspection agencies for approval by the Building Official and the Design Professional.			
3. Special Inspections as required by Section 1704.2.5 are not required where the fabricator is approved in accordance with IBC Section 1704.2.5.2			
4. Observe on a random basis, operations need not be delayed pending these inspections. Perform these tasks for each welded joint, bolted connection, or steel element.			
5. NDT of welds completed in an approved fabricator's shop may be performed by that fabricator when approved by the AHJ. Refer to AISC 360, N7.			

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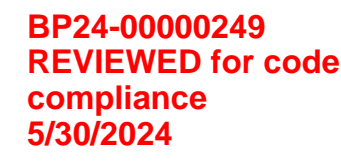
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HISTORIC REHABILITATION - APARTMENTS  
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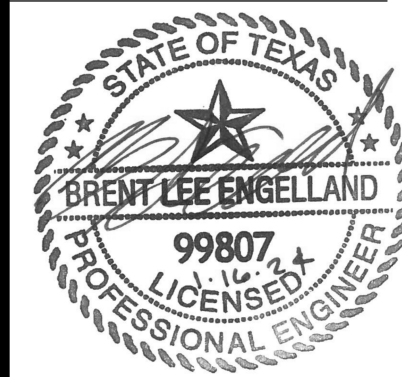

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1. DEMO ALL FLOOR AND ROOF SLAB/DECK AND METAL JOISTS IN HATCHED AREA.
2. EXISTING BEAMS MARKED (E) TO REMAIN U.N.O.
3. CONTRACTOR TO SHORE AND BRACE EXISTING ELEMENTS DURING DEMO. BRACING TO REMAIN IN PLACE UNTIL NEW METAL DECK IS IN PLACE AND ATTACHED.


-  NORTH
- 



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# **ROOSEVELT LOFTS**

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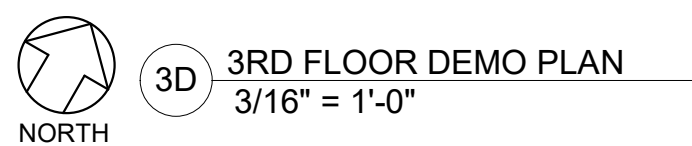
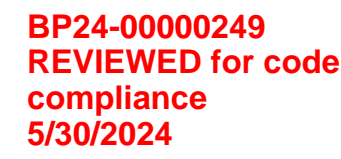
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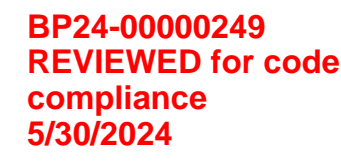
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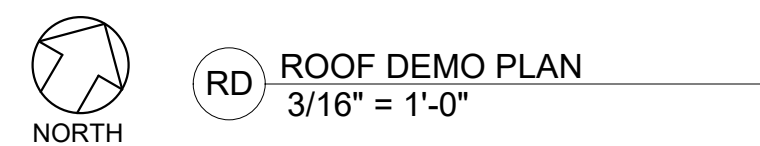
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2. EXISTING BEAMS MARKED (E) TO REMAIN U.N.O.
3. CONTRACTOR TO SHORE AND BRACE EXISTING ELEMENTS DURING DEMO. BRACING TO REMAIN IN PLACE UNTIL NEW METAL DECK IS IN PLACE AND ATTACHED.
4. DEMO EXISTING BEAM TO ACCOMMODATE NEW CMU ELEVATOR SHAFT







1. FOR GENERAL STRUCTURAL NOTES (GSN) SEE SHEET S0.0
2. DIMENSIONS SHOWN HERE APPLY TO STRUCTURAL ELEMENTS ONLY. SEE ARCHITECTURAL FOR ANY DIMENSIONS NOT NOTED HERE.
3. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
4. EXISTING ROOF JOISTS & DECK TO REMAIN.
5. DEMO AND REPLACE ROOFING, COVER BOARDS, INSULATION, AND MEMBRANE PER ARCHITECTURAL DRAWINGS. TAKE CARE TO PROTECT EXISTING CONCRETE ROOF DECK DURING RE-ROOF WORK.
6. DEMO EXISTING BEAM TO ACCOMMODATE NEW CMU ELEVATOR SHAFT
7. DEMO EXISTING CONCRETE FLOOR DECK IN HATCHED AREA.
8. DEMO EXISTING CONCRETE STUB COLUMN ABOVE ROOF TO ACCOMMODATE NEW RTU SUPPORT BEAM.



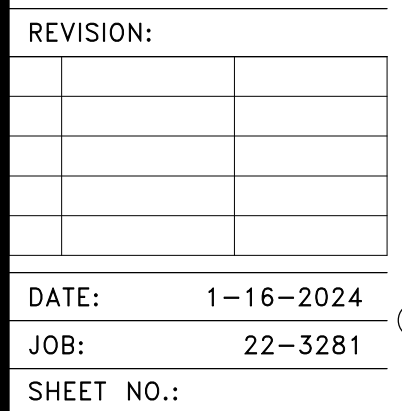
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- PLAN NOTES**
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  - ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
  - 2-1/2" TOTAL DEPTH CONCRETE SLAB OVER METAL DECK:  
NORMAL WEIGHT CONCRETE W/ 6x6 W2.9xW2.9 WWF AT MIDHEIGHT OF CONCRETE W/ 1.0C24 METAL DECK.  
ATTACH DECK TO PERPENDICULAR SUPPORTS W/ #12 TEK SCREWS IN 3/4" PATTERN. ATTACH DECK TO PARALLEL SUPPORTS W/ #12 TEK SCREWS @ 12" O.C.  
DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.
  - METAL ROOF DECK:  
NEW ROOF DECK TO BE 1.5C24 METAL DECK.  
ATTACH DECK TO PERPENDICULAR SUPPORTS WITH #12 TEK SCREWS AT EVERY FLUTE. ATTACH DECK TO PARALLEL SUPPORTS WITH #12 TEK SCREWS AT 12" O.C.  
DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.

- 6 DELAMINATION ON BEAM
- CLEAN AND REMOVE RUST AND SCALE.
  - CHECK REMAINING THICKNESS.
  - WHERE MORE THAN 25% OF ORIGINAL THICKNESS IS MISSING, WELD 5/16" COVER PLATE W/ MIN. 6" ON EACH SIDE OF DAMAGE.
  - PRIME AND PAINT REPAIRED AREA AFTER REPAIR IS COMPLETE
7. CONTRACTOR TO FIELD VERIFY CONDITION OF ALL EXISTING BEAMS. ADDITIONAL REPAIRS MAY BE REQUIRED BEYOND THOSE NOTED.



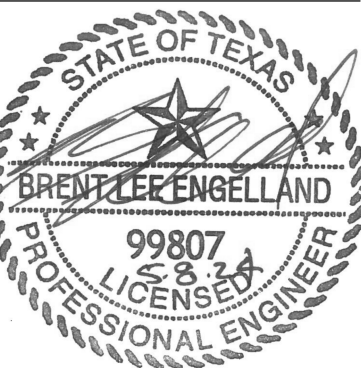
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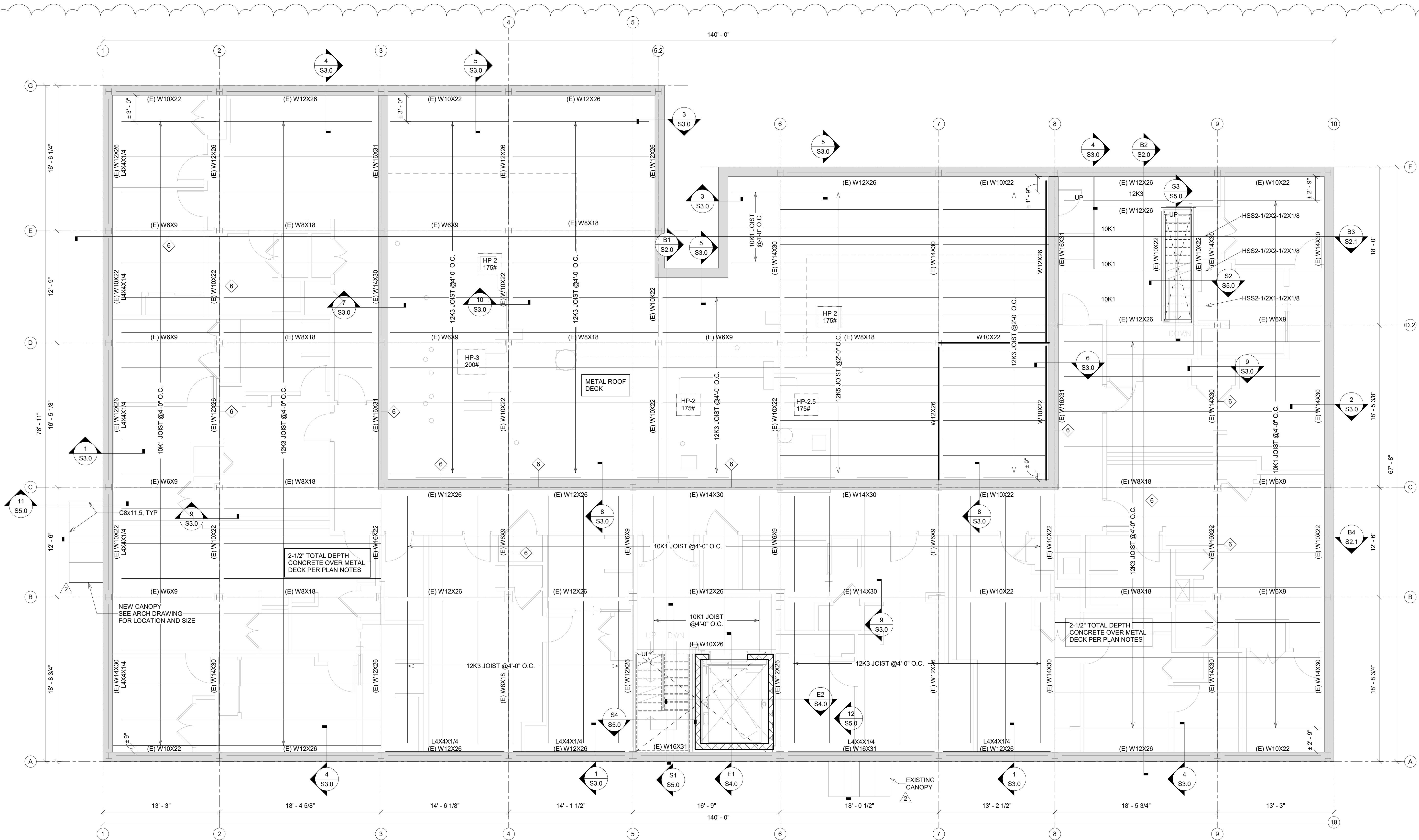
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2 2ND FLOOR AND LOW ROOF FRAMING PLAN  
3/16" = 1'-0"

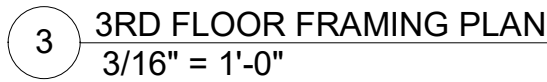
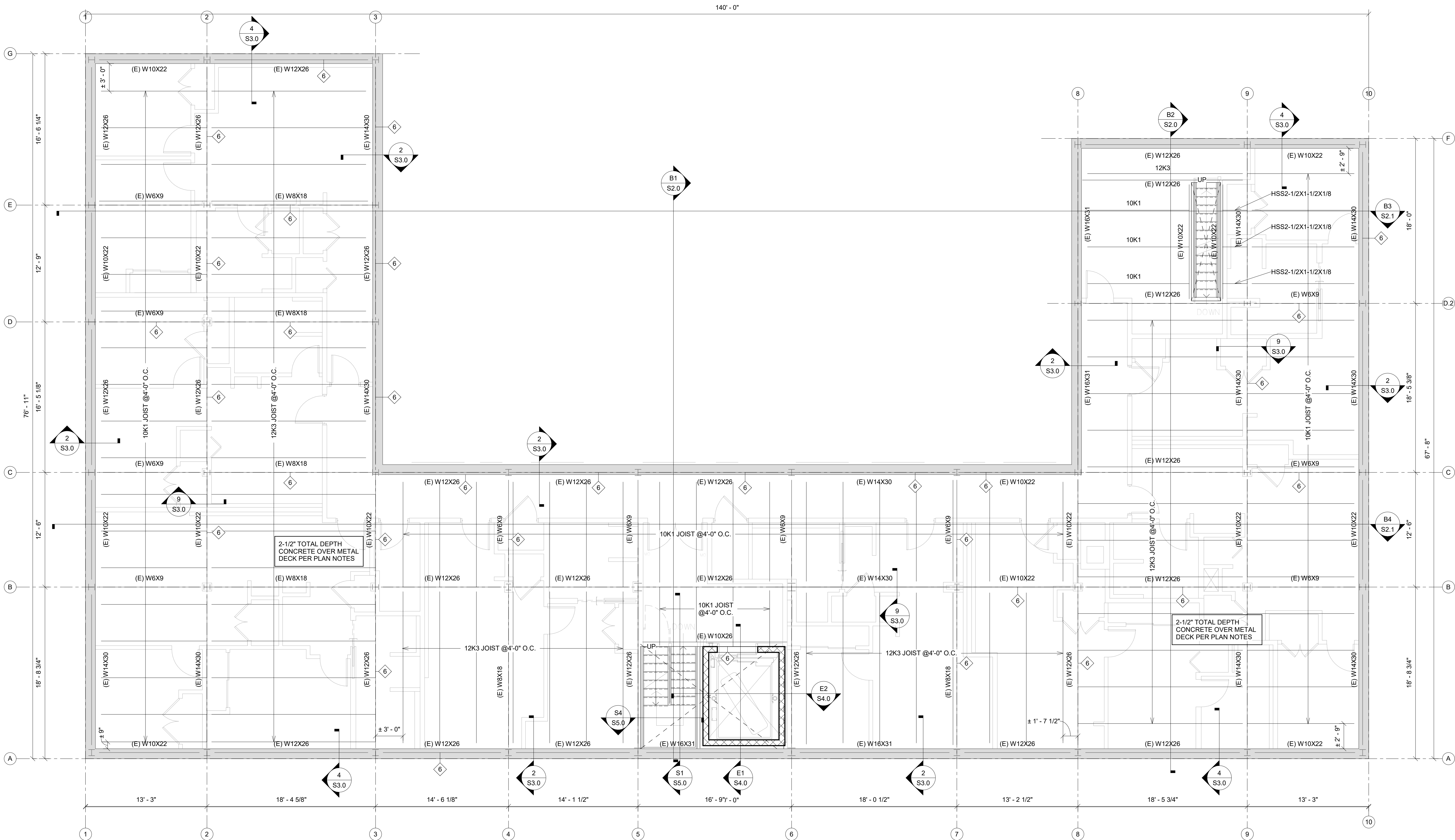
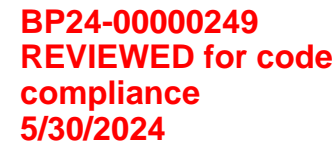
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3. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
4. 2-1/2" TOTAL DEPTH CONCRETE SLAB OVER METAL DECK;  
NORMAL WEIGHT CONCRETE W/ 6x6 W2.9xW2.9 W/WF AT MIDHEIGHT OF DECK SLAB W/ 1.0c24 METAL DECK;  
ATTACH DECK TO PERPENDICULAR SUPPORTS W/ #12 TEK SCREWS IN 3/4" PATTERN. ATTACH DECK TO PARALLEL SUPPORTS W/ #12 TEK SCREWS @ 12" O.C. DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.
5. METAL ROOF DECK;  
NEW ROOF DECK TO BE 1.5C24 METAL DECK.  
ATTACH DECK TO PERPENDICULAR SUPPORTS WITH #12 TEK SCREWS AT EVERY FLUTE. ATTACH DECK TO PARALLEL SUPPORTS WITH #12 TEK SCREWS AT 12" O.C. DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.

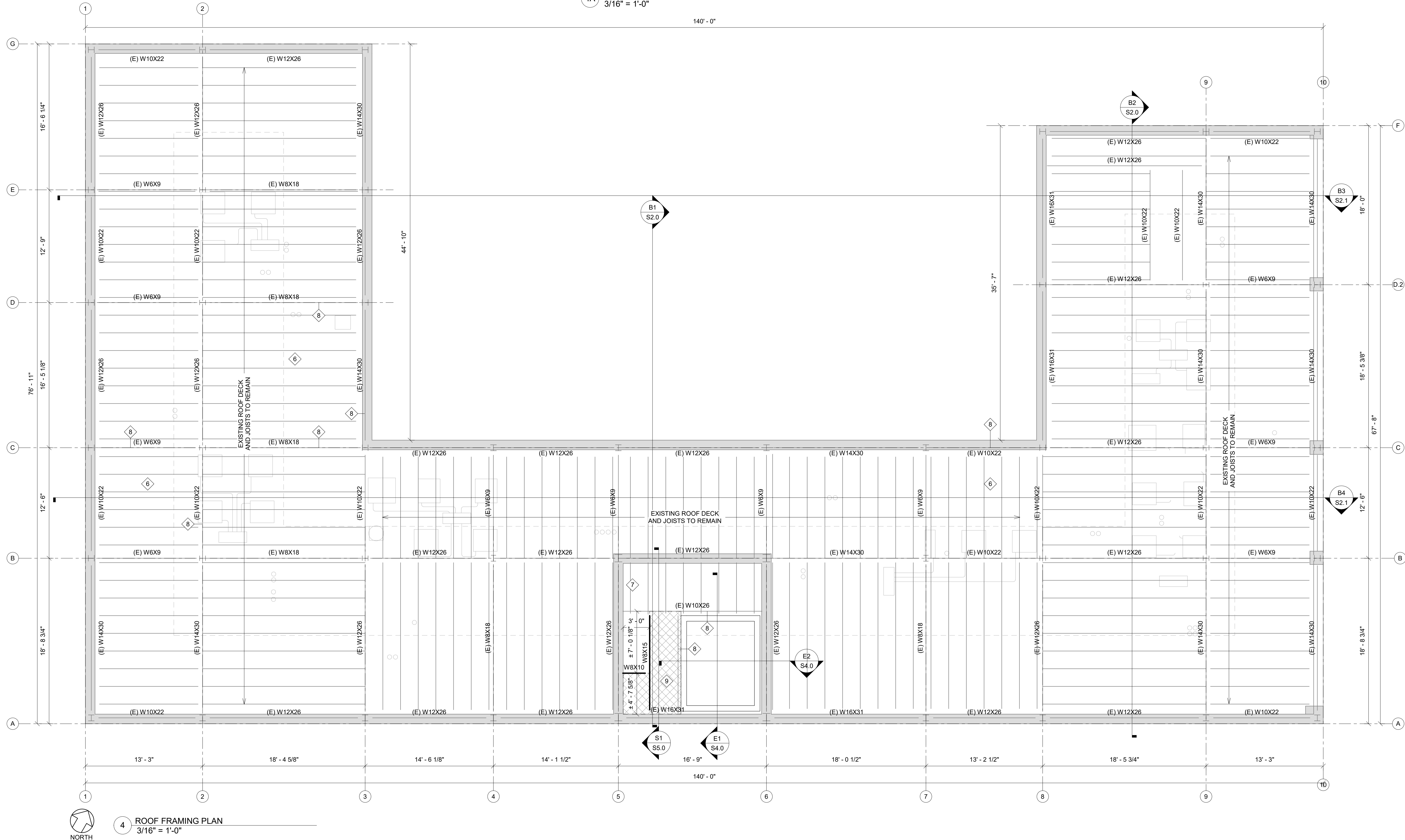
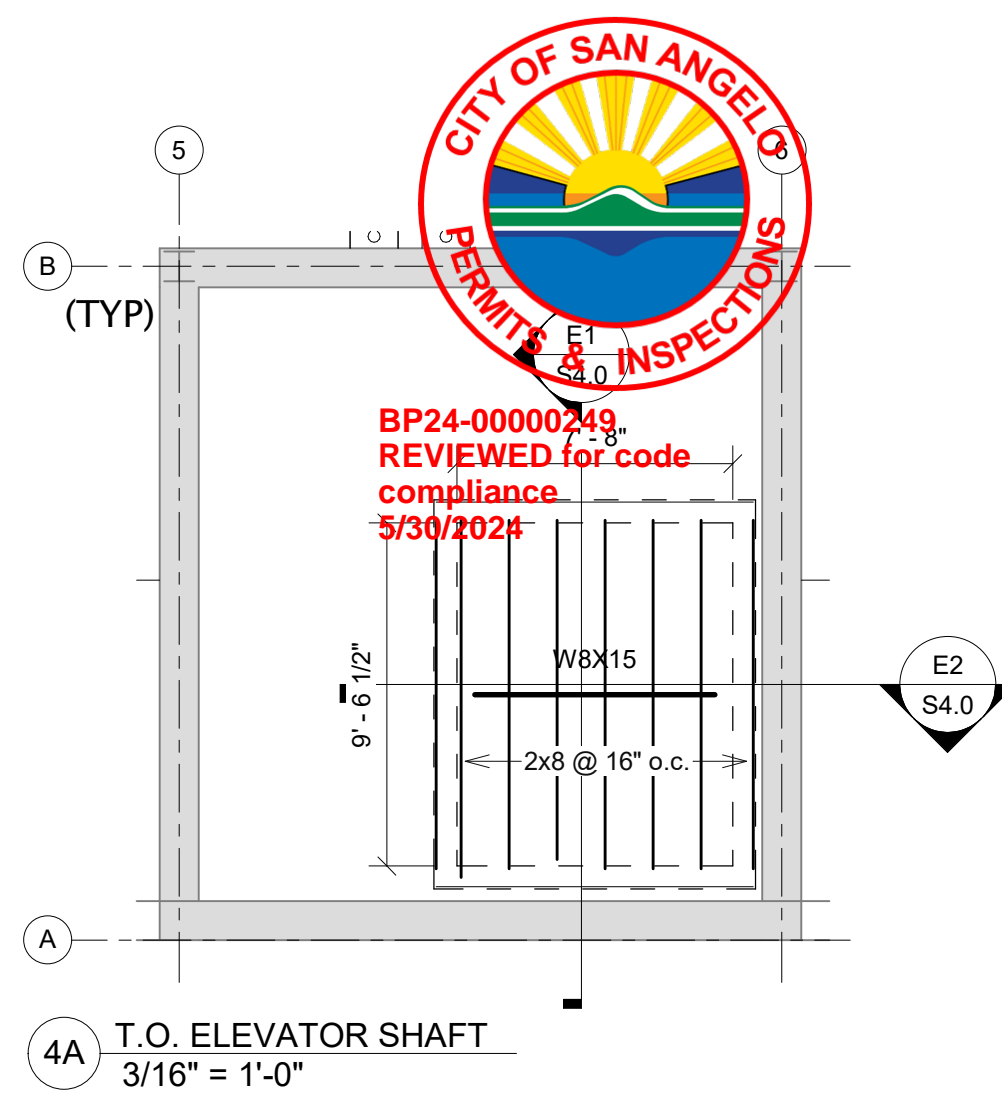
6. DELAMINATION ON BEAM
- A. CLEAN AND REMOVE RUST AND SCALE.
  - B. CHECK REMAINING THICKNESS.
  - C. WHERE MORE THAN 25% OF ORIGINAL THICKNESS IS MISSING, WELD 5/16" COVER PLATE WITH ANGLE ON EACH SIDE OF DAMAGE.
  - D. PRIME AND PAINT REPAIRED AREA AFTER REPAIR IS COMPLETE
7. CONTRACTOR TO FIELD VERIFY CONDITION OF ALL EXISTING BEAMS. ADDITIONAL REPAIRS MAY BE REQUIRED BEYOND THOSE NOTED.





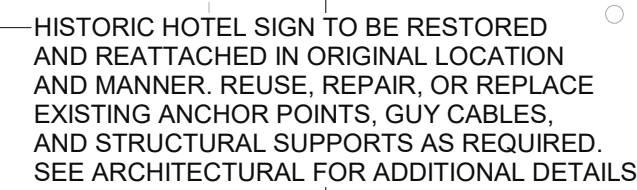
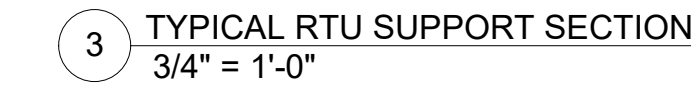
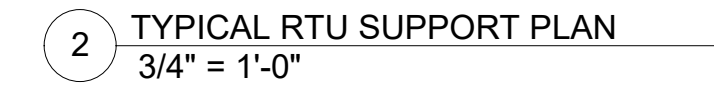
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3. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
4. EXISTING ROOF JOISTS & DECK TO REMAIN.
5. REPLACE EXISTING ROOFING, COVER BOARDS, INSULATION, AND MEMBRANE PER ARCHITECTURAL DRAWINGS. TAKE CARE TO PROTECT EXISTING CONCRETE ROOF DECK DURING RE-ROOF WORK.
6. REPAIR CORRODED JOISTS IN THIS BAY PER DETAIL 11 / S3.0  
ANY JOIST ELEMENT THAT HAS LOST MORE THAN 25% OF ITS ORIGINAL CROSS-SECTION DUE TO CORROSION OR DAMAGE NEEDS TO BE REPAIRED.  
G.C. TO REVIEW ALL JOISS IN ALL BAYS FOR DAMAGE OR CORRODED JOISTS.
7. REPAIR DAMAGED CONCRETE ROOF DECK THIS BAY BY CAREFULLY REMOVING EXISTING DECK TO THE NEXT ADJACENT JOIST BEYOND THE DAMAGED AREA.  
REPLACE WITH 2'-1/2" TOTAL DEPTH N.W. CONCRETE SLAB OVER 1.0C20 METAL DECK W/ 6x6 W2.9xW2.9 WWF AT MIDGEIGHT OF CONCRETE. TACK WELD DECK TO EXISTING JOISTS ON BOTH SIDES AT EACH FLUTE.

8. DELAMINATION ON BEAM
- A. CLEAN AND REMOVE RUST AND SCALE.
  - B. CHECK REMAINING THICKNESS.
  - C. WHERE MORE THAN 25% OF ORIGINAL THICKNESS IS MISSING, WELD 5/16" COVER PLATE W/ MIN. 6" ON EACH SIDE OF DAMAGE.
  - D. PRIME AND PAINT REPAIRED AREA AFTER REPAIR IS COMPLETE
9. REPLACE EXISTING FLOOR IN HATCHED AREA W/ 2-1/2" TOTAL DEPTH CONCRETE SLAB OVER METAL DECK:  
NORMAL WEIGHT CONCRETE W/ 6x6 W2.9xW2.9 WWF AT MIDHEIGHT OF CONCRETE W/ 1.0C24 METAL DECK.  
ATTACH DECK TO PERPENDICULAR SUPPORTS W/ #12 TEK SCREWS IN 3/4" PATTERN AT TYPICAL SPACING TO PARALLEL SUPPORTS W/ #12 TEK SCREWS W/ 12" O.C. DECK SIDE LAP CONNECTION TO BE (1) #10 TEK SCREW PER DECK SPAN. 3 SPAN MINIMUM FOR STEEL DECK.
10. CONTRACTOR TO FIELD VERIFY CONDITION OF ALL EXISTING BEAMS. ADDITIONAL REPAIRS MAY BE REQUIRED BEYOND THOSE NOTED.





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3. ALL DIMENSIONS SHALL BE FIELD VERIFIED PRIOR TO STARTING WORK. DIMENSIONS SHOWN HERE ARE FOR REFERENCE ONLY.
4. COORDINATE LOCATION OF MECHANICAL ROOFTOP UNITS WITH MECHANICAL DRAWINGS.
5. ALL STEEL EXPOSED TO WEATHER TO BE PRIMED AND PAINTED.







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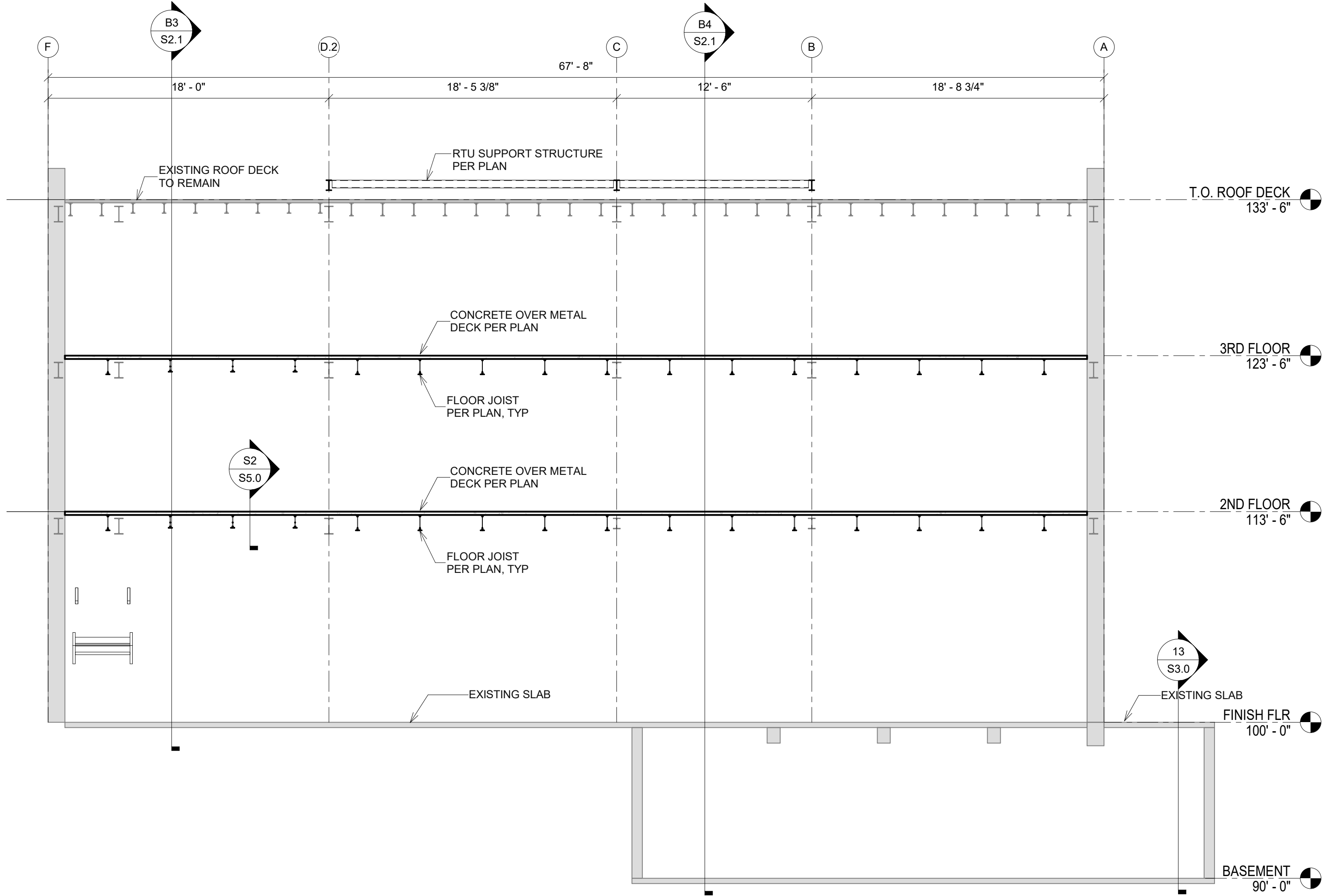
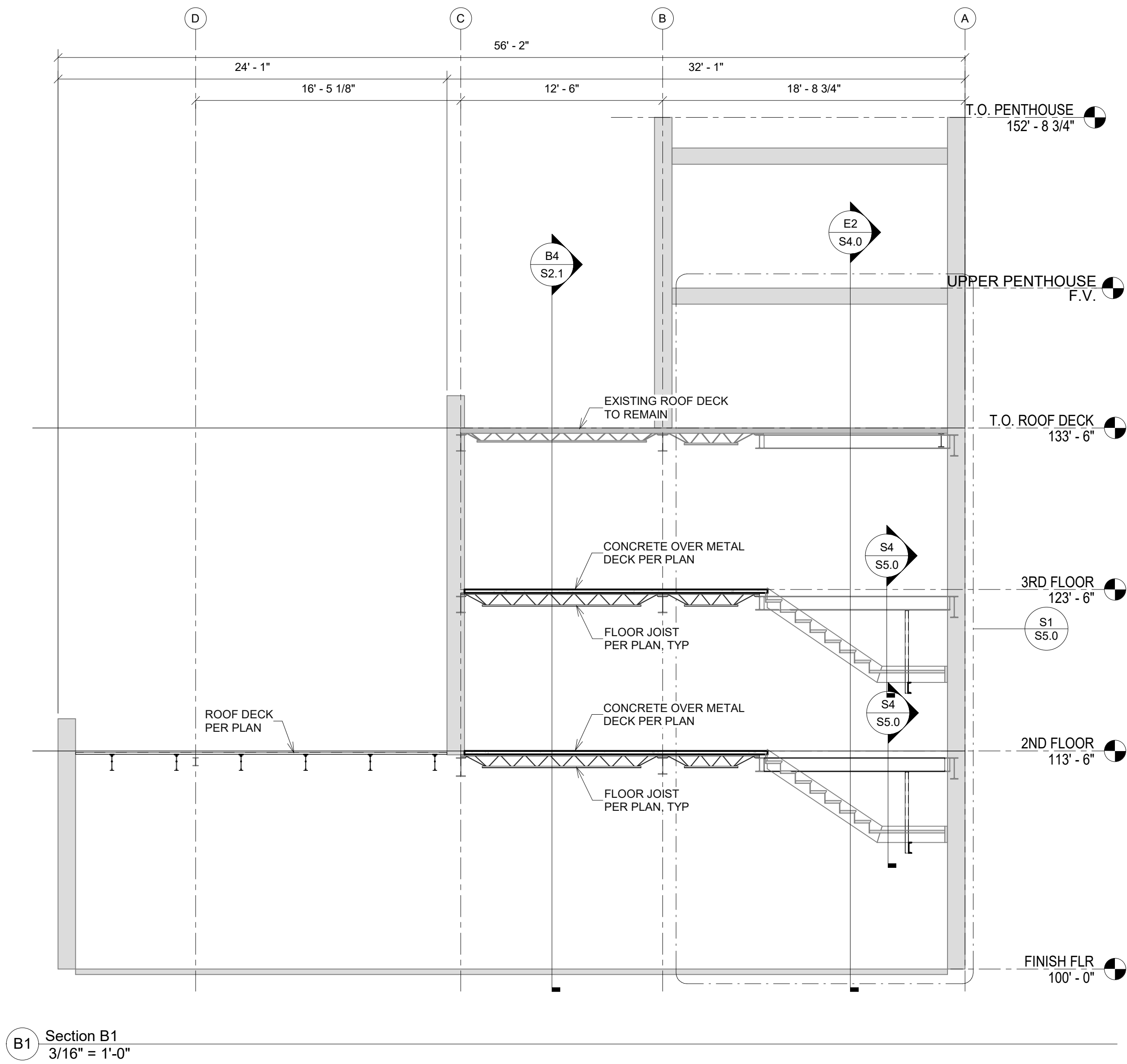
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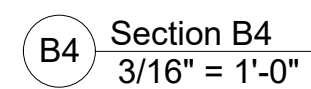
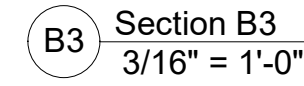
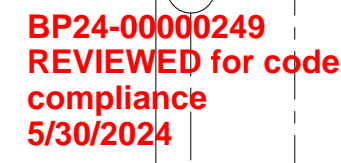
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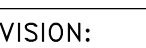






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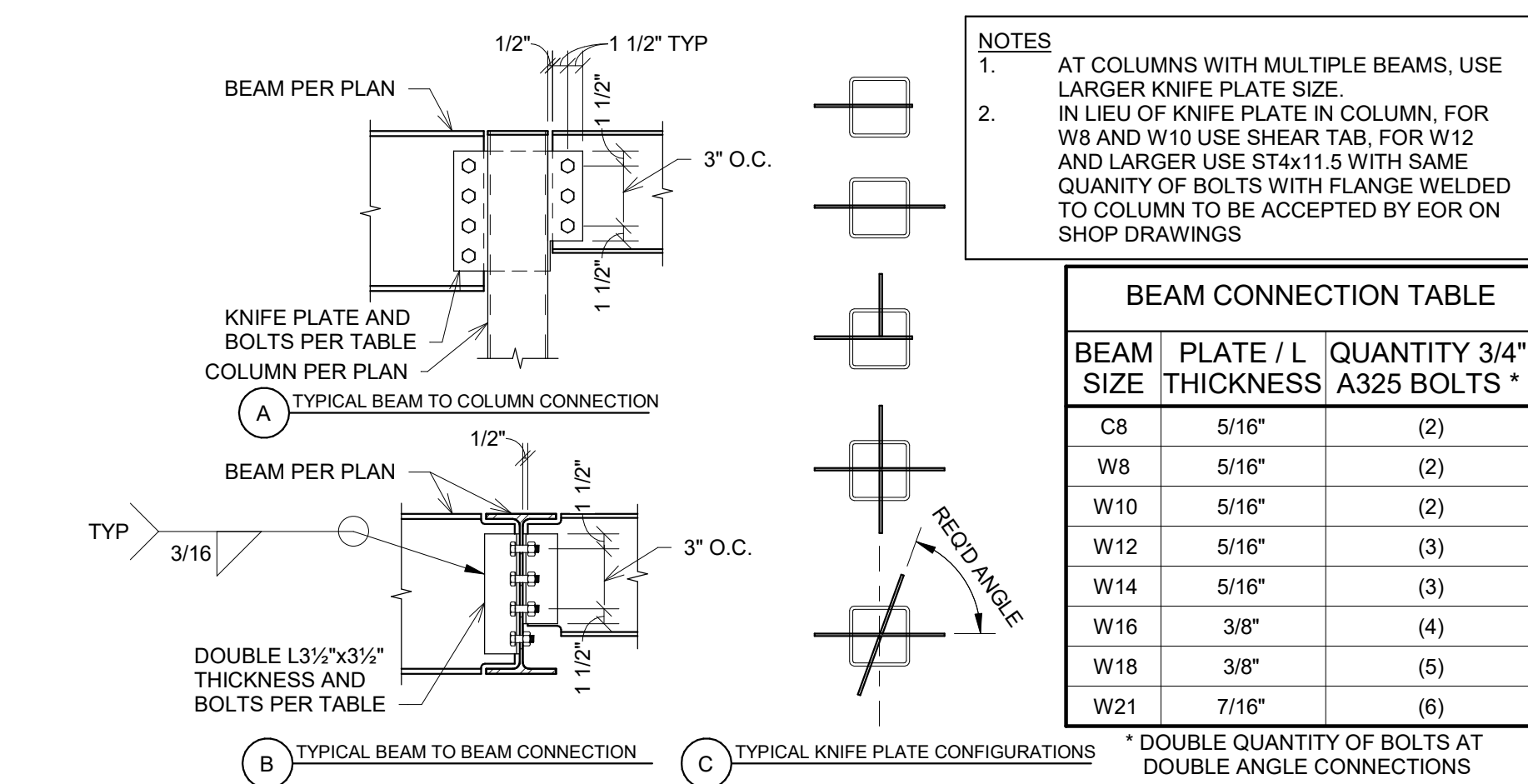
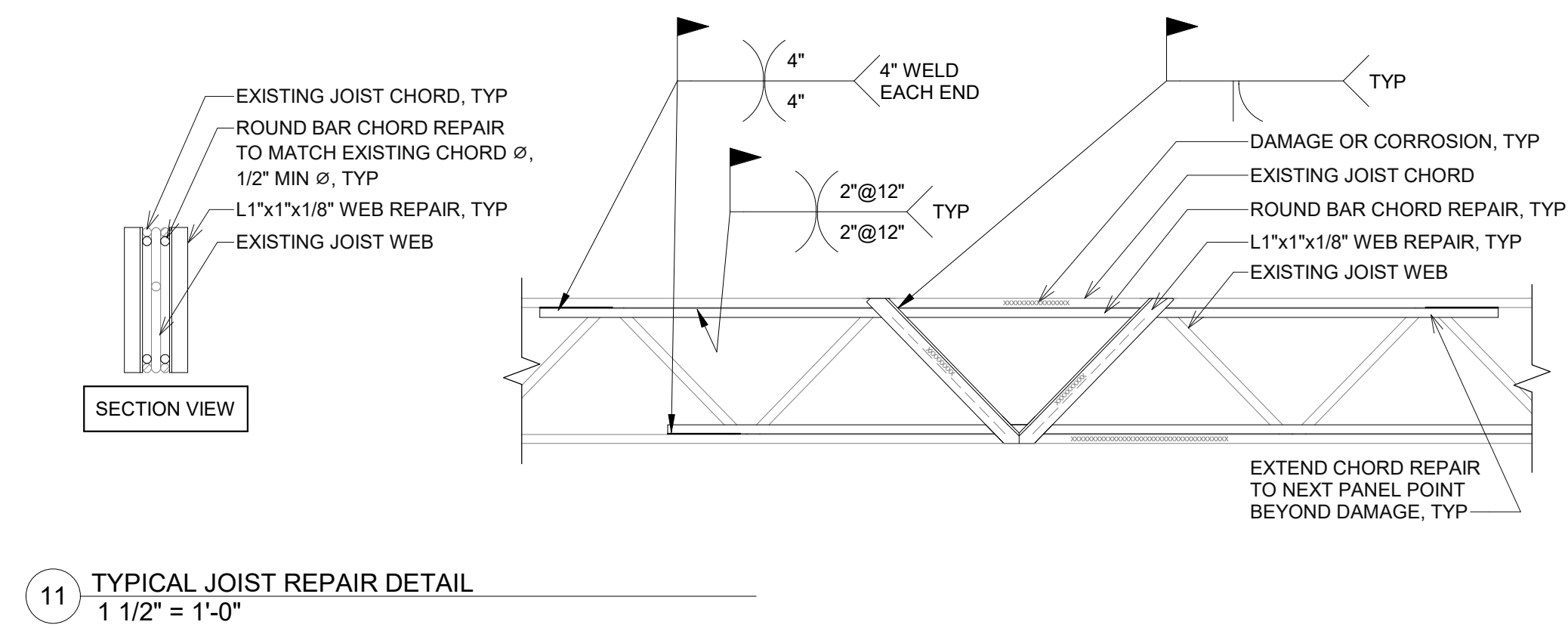
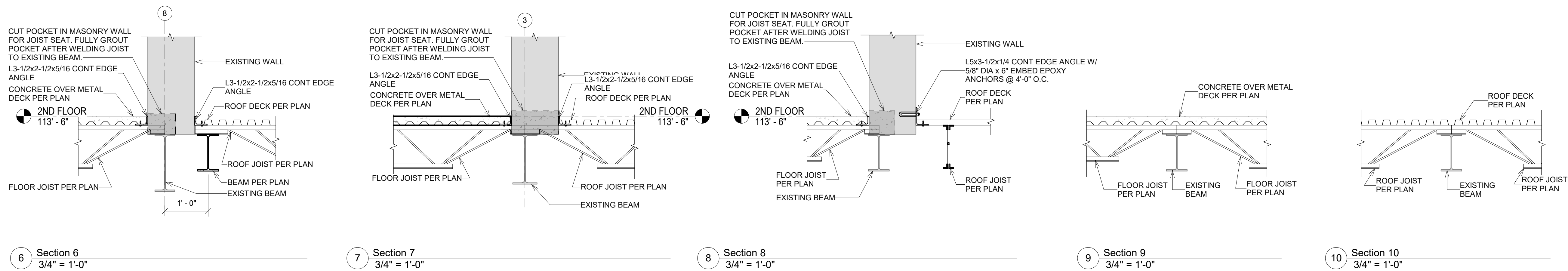
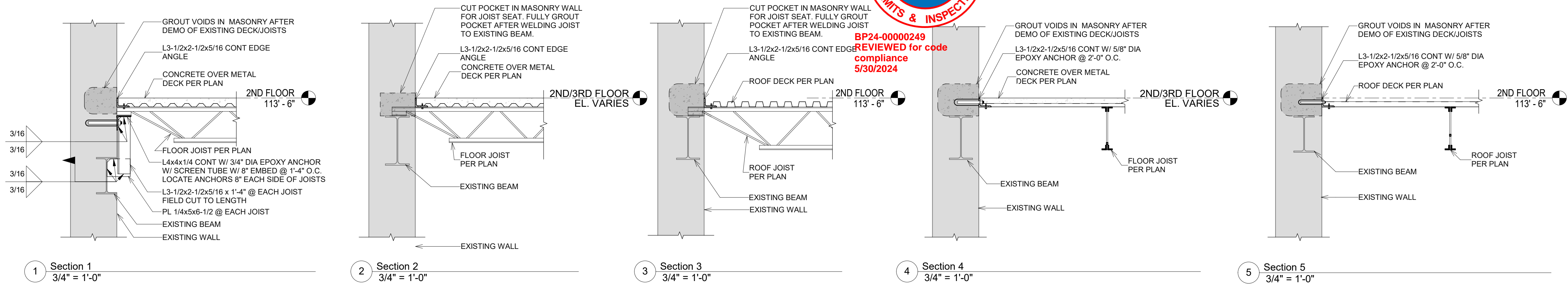
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- NOTES
1. AT COLUMNS WITH MULTIPLE BEAMS, USE LARGER KNIFE PLATE SIZE.
  2. IN LIEU OF KNIFE PLATE IN COLUMN, FOR W8 AND W10 USE SHEAR TAB. FOR W12 AND LARGER USE ST4x11.5 WITH SAME QUANTITY OF BOLTS WITH FLANGE WELDED TO COLUMN TO BE ACCEPTED BY EOR ON SHOP DRAWINGS

BEAM CONNECTION TABLE		
BEAM SIZE	PLATE / L THICKNESS	QUANTITY 3/4" A325 BOLTS *
C8	5/16"	(2)
W8	5/16"	(2)
W10	5/16"	(2)
W12	5/16"	(3)
W14	5/16"	(3)
W16	3/8"	(4)
W18	3/8"	(5)
W21	7/16"	(6)

\* DOUBLE QUANTITY OF BOLTS AT DOUBLE ANGLE CONNECTIONS



- REBAR AND CONCRETE REPAIR:
1. REMOVE LOOSE AND SPALLING CONCRETE. ROUGHEN SURFACE TO REPAIR, AND CLEAN THOROUGHLY.
  2. REMOVE RUST AND CORROSION FROM EXISTING REINFORCING.
  3. COAT REPAIR AREA AND ALL SIDES OF REINFORCING WITH SIKA ARMATEC-110 EPODEM BONDING PRIMER AND CORROSION PROTECTION.
  4. PATCH REPAIR AREA WITH SIKA SIKAREPAIR 223 PATCHING MATERIAL.

13 BASEMENT REPAIR DETAIL  
NTS

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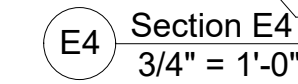
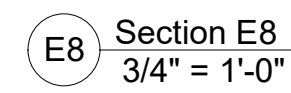
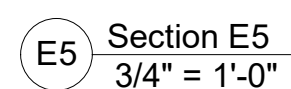
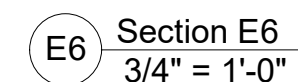
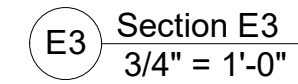
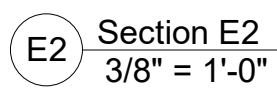
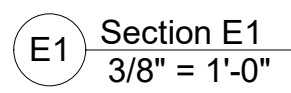


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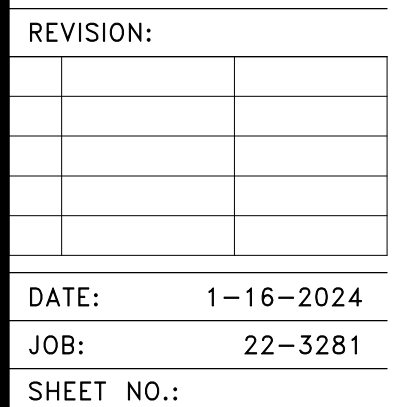
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