

Final Stormwater Management Plan

Proposed Residences At Green Meadow
Senior Living Facility
San Angelo, TX

October 30, 2024

Prepared By:
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INTRODUCTION

This Stormwater Management Plan has been prepared for the proposed development of a 30 unit senior living facility located at 3800 Green Meadow, San Angelo, TX. The total site area is 1.43 acres. The site location is shown in Figure 1 below.



Figure 1: Site Location Map

The purpose of the study is to analyze the effects the project will have on the local drainage patterns and to determine measures necessary to prevent negative impacts downstream.

DRAINAGE ANALYSIS METHODS AND CALCULATIONS

The drainage study was prepared using Hydrology Studio software to analyze both existing and proposed site drainage characteristics. (the Hydrology Studio report pages are included in Appendix B). Hydrology Studio is a hydrology software suite created by Terry Stringer who was the original developer of Hydraflow. When Hydraflow was sold to ACAD Terry created Hydrology Studio to service engineers who worked outside the ACAD interface. The rational

method was used to determine the existing runoff with the modified rational method being used for the developed runoff per the city of San Angelo Stormwater Design Manual.

EXISTING DRAINAGE

The drainage area considered is 1.43 acres in size and is currently vacant ground covered in grass and dirt. On the south end of the site is Green Meadow Drive on the east is an existing commercial development and on the northwest side is an existing arroyo. The site predominantly drains northwesterly to the existing arroyo. The existing drainage map is included in Appendix A.

PROPOSED DRAINAGE PLAN

The development plan calls for construction of a 10,200 SF, 30 unit, senior living facility and associated parking area.

The building will be located in the north portion of the lot with parking located to the south and east. The detention system will be an above ground dry pond located in the northeast portion of the lot. A copy of the proposed drainage plan is included in Appendix A. The outlet of the pond will be an 8” pipe that will outlet into the arroyo at an elevation above the base flood elevation of 1892 ft. The 8” pipe will limit the stormwater flow from the to the predevelopment runoff for the 2 yr and 100 yr storms.

The pre and post development runoff curve numbers were calculated using runoff coefficients listed in the San Angelo Stormwater Design Manual. The existing ground is comprised of spares grass and dirt. The stormwater manual lists a 0.3 coefficient for parks & open areas. To more closely model the existing conditions a runoff coefficient of 0.4 was used. The developed runoff coefficient was calculated using 0.30 for the grass areas and 0.95 for the impervious areas. A developed runoff coefficient of 0.65 was calculated.. The calculation is summarized below.

$$(0.96 \text{ Ac} \times 0.95) / 1.43 + (0.67 \times 0.30) / 1.43 = 0.78$$

Due to the size of the site and the slope of the existing ground a predeveloped and post developed time of concentration of 5.00 minutes was used. Below is a chart comparing the developed runoff values to the predeveloped.

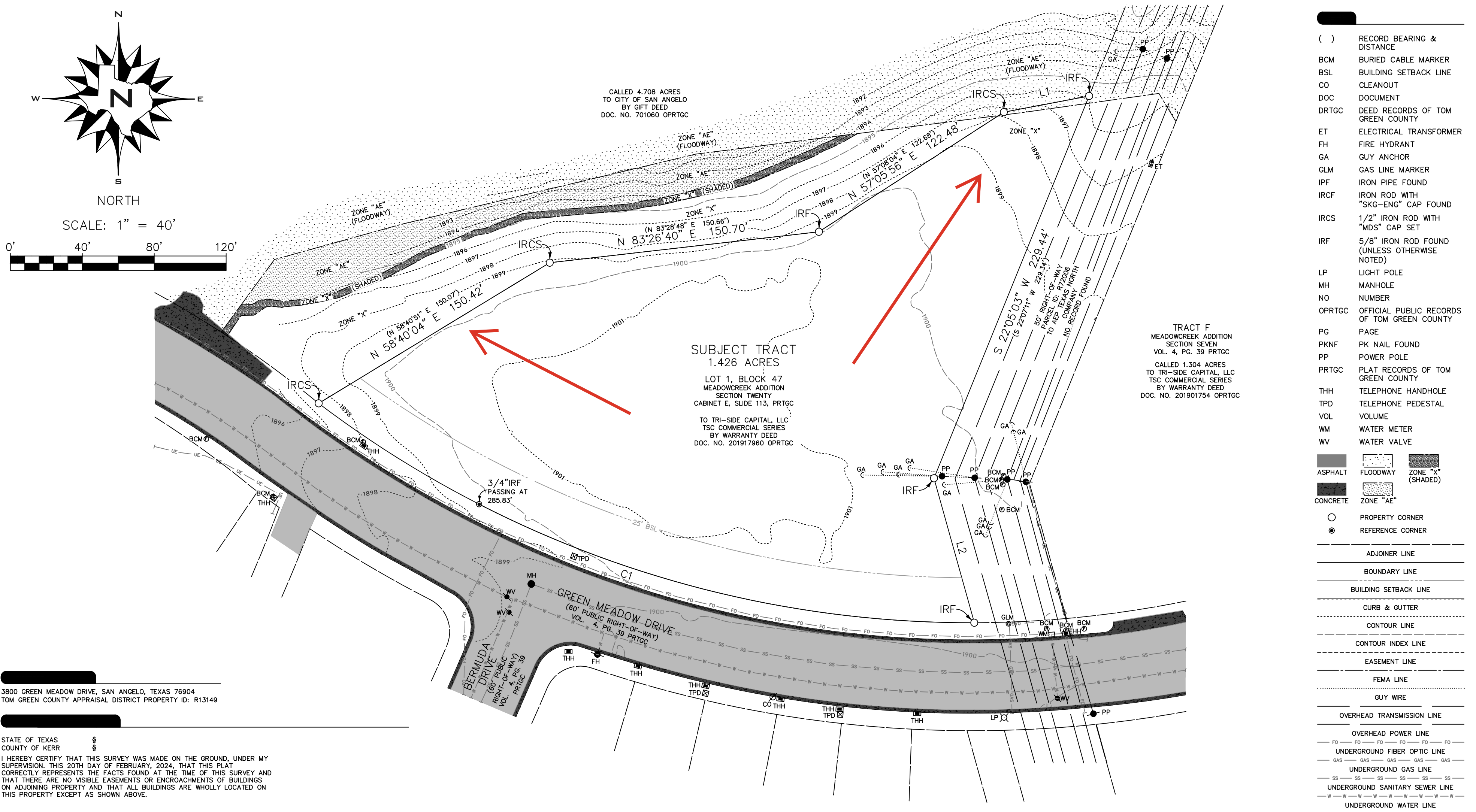
	2 Yr. Runoff (cfs)	100 Yr. Runoff (cfs)
Pre-Developed	2.89	5.65
Developed	3.21	6.00
Outlet from Pond	2.43	3.85
Difference from Predeveloped	-0.46	-1.80

CONCLUSION

The proposed senior living development will increase the stormwater runoff from the site thus requiring stormwater quantity to be addressed per the San Angelo Stormwater Design Manual. Based on the above discussion, if constructed as proposed, the overall stormwater runoff from the site will be decreased.

Appendix A

- Existing Drainage Plan
- Proposed Drainage Plan



3800 GREEN MEADOW DRIVE, SAN ANGELO, TEXAS 76904
TOM GREEN COUNTY APPRAISAL DISTRICT PROPERTY ID: R13149

STATE OF TEXAS §
COUNTY OF KENDALL §
I HEREBY CERTIFY THAT THIS SURVEY WAS MADE ON THE GROUND, UNDER MY SUPERVISION, THIS 20TH DAY OF FEBRUARY, 2024, THAT THIS PLAT CORRECTLY REPRESENTS THE FACTS FOUND AT THE TIME OF THIS SURVEY AND THAT THERE ARE NO VISIBLE EASEMENTS OR ENCROACHMENTS OF BUILDINGS ON ADJOINING PROPERTY AND THAT ALL BUILDINGS ARE WHOLLY LOCATED ON THIS PROPERTY EXCEPT AS SHOWN ABOVE.

JEFF BOERNER RPLS # 4939

OBSERVED				RECORD					
LINE	BEARING	DISTANCE	CHORD	LINE	BEARING	DISTANCE	CHORD		
L1	N 79°15'10" E	45.92	N 79°17'24" E	45.92	L2	S 19°37'57" E	83.45	S 19°35'49" E	83.46

OBSERVED				RECORD						
RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE	RADIUS	ARC LENGTH	CHORD LENGTH	CHORD BEARING	DELTA ANGLE	
C1	600.00'	391.45'	384.54'	N 71°30'00" W	37°22'49"	600.00'	391.28'	384.38'	N 71°27'53" W	37°21'51"

REFERENCE COMMITMENT FOR TITLE INSURANCE ISSUED BY TITLE RESOURCES GUARANTY COMPANY, COUNTERSIGNED BY TITLE RESOURCES GUARANTY COMPANY, OF NO. 2400160-COM, HAVING AN EFFECTIVE DATE OF JANUARY 12, 2024 AND AN ISSUE DATE OF JANUARY 10, 2024. NO FURTHER RESEARCH FOR EASEMENTS OR ENCUMBRANCES WAS PERFORMED BY MDS LAND SURVEYING COMPANY, INC.

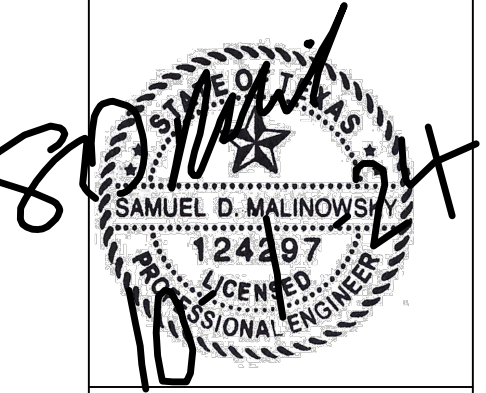
- ALL BEARINGS, DISTANCES AND COORDINATES ARE REFERENCED TO THE TEXAS STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE (4203), NORTH AMERICAN DATUM OF 1983 (2011 ADJUSTMENT), U.S. SURVEY FEET.
- DISTANCES SHOWN HEREON ARE GRID VALUES (US SURVEY FEET).
- FIELD SURVEY COMPLETED 2-9-2024.
- VISIBLE IMPROVEMENTS/UTILITIES WERE LOCATED WITH THIS SURVEY; NO SUBSURFACE PROBING, EXCAVATION OR EXPLORATION WAS PERFORMED FOR THIS SURVEY. THE SURVEYOR HAS NOT BEEN PROVIDED WITH CONSTRUCTION PLANS SHOWING THE LOCATION OF UNDERGROUND UTILITIES. UNDERGROUND UTILITIES MAY EXIST WHICH ARE NOT SHOWN HEREON.
- DETERMINATION OF THE OWNERSHIP, LOCATION, OR DEVELOPMENT OF MINERALS RELATED TO THE SUBJECT TRACT FALL OUTSIDE THE SCOPE OF THIS SURVEY. SUCH MATTERS SHOULD BE DIRECTED BY THE CLIENT OR PROSPECTIVE PURCHASER TO AN EXPERT CONSULTANT.
- ACCORDING TO MAP NO. 48401C0490E OF THE FEDERAL EMERGENCY MANAGEMENT AGENCY'S FLOOD INSURANCE RATE MAPS FOR TOM GREEN COUNTY, DATED JUNE 18, 2012, THE SUBJECT TRACT IS SITUATED WITHIN: UNSHADED ZONE "X", DEFINED AS AREAS DETERMINED TO BE OUTSIDE THE 500-YEAR FLOOD PLAIN; AND ZONE "AE", DEFINED AS SPECIAL FLOOD HAZARD AREAS INUNDATED BY 100-YEAR FLOOD WITH BASE ELEVATIONS DETERMINED. THIS FLOOD STATEMENT DOES NOT IMPLY THAT THE PROPERTY OR STRUCTURES THEREON WILL BE FREE FROM FLOODING OR FLOOD DAMAGE. ON RARE OCCASIONS FLOODS CAN AND WILL OCCUR AND FLOOD HEIGHTS MAY BE INCREASED BY MAN-MADE OR NATURAL CAUSES. THIS FLOOD STATEMENT SHALL NOT CREATE LIABILITY ON THE PART OF THE SURVEYOR.
- THIS SURVEY HAS BEEN PREPARED FOR THE SOLE PURPOSE OF THE TRANSACTION DESCRIBED HEREON REFERENCED TITLE COMMITMENT AND THE PARTIES LISTED THEREON. THIS SURVEY IS NOT TO BE USED FOR ANY SUBSEQUENT TRANSACTIONS AND NO FURTHER RESEARCH FOR EASEMENTS OR ENCUMBRANCES WAS PERFORMED BY MDS LAND SURVEYING COMPANY, INC.
- BUILDING SETBACK LINES SHOWN HEREON ARE IN ACCORDANCE WITH THE SETBACK LINE REQUIREMENTS AS SHOWN ON THE SUBDIVISION PLAT RECORDED IN CABINET "E", SLIDE 113, PLAT RECORDS OF TOM GREEN COUNTY, TEXAS. NO FURTHER RESEARCH OF SAID BUILDING SETBACK LINES WAS PERFORMED BY MDS LAND SURVEYING COMPANY, INC.
- THE MATHEMATICAL CLOSURE FOR THE SUBJECT TRACT BOUNDARY SHOWN HEREON IS IN ACCORDANCE WITH THE MINIMUM STANDARDS SET FORTH BY THE TEXAS BOARD OF PROFESSIONAL ENGINEERS AND SURVEYORS (TBPELS). CLOSURE = 0.007'
- A ONE CALL UTILITY LOCATION SERVICE WAS PERFORMED FOR THIS SURVEY TO LOCATE AND MARK UNDERGROUND UTILITIES. UNDERGROUND UTILITIES MARKED IN CONJUNCTION TO ONE CALL TICKET # 245303870, SUBMITTED ON 2-2-2024, ARE SHOWN HEREON AND WERE LOCATED IN CONJUNCTION TO PIN FLAG AND PAINT STRIPE MARKERS PRESENT IN THE FIELD AT THE TIME OF THE SURVEY. UNDERGROUND UTILITIES AND FACILITIES OWNED AND/OR OPERATED BY FRONTIER COMMUNICATIONS (PHONE), SUDOCOMMUNICATIONS (CABLE / FIBER), HENUS (FIBER), CITY OF SAN ANGELO (WATER AND SANITARY SEWER) TICKET RESPONSE SHOWS SAID UTILITIES WERE MARKED ON 2-12-2024, AND WERE NOT PRESENT PRIOR TO THE COMPLETION OF THIS ON THE GROUND SURVEY AND ARE NOT SHOWN HEREON. DUE TO TIME CONSTRAINTS AND LIMITATIONS THE SURVEYOR WAS NOT ABLE TO LOCATE SAID UTILITIES PRIOR TO THE COMPLETION OF THIS SURVEY. THE UNDERGROUND WATER, SANITARY SEWER AND FIBER OPTIC LINES SHOWN HEREON DO NOT REPRESENT AN ON THE GROUND SURVEY. SAID UTILITIES ARE GRAPHIC LINES AND ARE BASED ON LIMITED INFORMATION MADE AVAILABLE TO THE SURVEYOR DURING THE PROCESS OF CONDUCTING THIS SURVEY. THE SURVEYOR HAS NOT BEEN PROVIDED WITH CONSTRUCTION PLANS SHOWING THE LOCATION OF UNDERGROUND UTILITIES. UNDERGROUND UTILITIES MAY EXIST WHICH ARE NOT SHOWN HEREON. THIS STATEMENT SHALL NOT CREATE LIABILITY ON PART OF THE SURVEYOR AND MDS LAND SURVEYING COMPANY INC ASSUMES NO LIABILITY AS TO THE ACCURACY AND OR COMPLETENESS OF THE LOCATION OF THE UNDERGROUND UTILITIES SHOWN HEREON. SUPPLEMENTAL UNDERGROUND UTILITY INFORMATION REGARDING THE UNDERGROUND WATER AND SANITARY SEWER LINES WAS OBTAINED FROM: [HTTPS://COSA1.MAPS.ARCGIS.COM/APPS/WEBAPPVIEWER/INDEX.HTML?ID=42EB8816BF4427390427ED0C507530](https://cogsa1.maps.arcgis.com/apps/webappviewer/index.html?id=42eb8816bf4427390427ed0c507530), ACCESSED 2-19-2024.
- ELEVATIONS SHOWN HEREON REFERENCE NAVD 88 VERTICAL DATUM AS DETERMINED BY GPS OBSERVATIONS.
- CONTOUR LINES SHOWN HEREON ARE IN CONJUNCTION WITH GPS OBSERVATIONS TAKEN AT THE TIME OF THIS SURVEY. (CONTOUR INTERVALS = 1 FOOT)
- ELEVATIONS SHOWN HEREON REFERENCE NAVD 88 VERTICAL DATUM AS DETERMINED BY GPS OBSERVATIONS.
- THE PROPERTY SHOWN HEREON IS IN THE "PD" ZONING DISTRICT, LOW-RISE, MULTIFAMILY RESIDENTIAL ZONING DISTRICT OF THE CITY OF SAN ANGELO, TEXAS.

LAND TITLE SURVEY
OF
LOT 1, BLOCK 47
MEADOWCREEK ADDITION
SECTION 20
BEING
1.426 ACRES
SITUATED IN THE
A. E. WHITE SURVEY, A-3944
TOM GREEN COUNTY, TEXAS
FEBRUARY 2024

MDS LAND SURVEYING COMPANY, INC.
ALTA | BOUNDARY | CONSTRUCTION | OIL & GAS TOPOGRAPHIC
TEXAS BOARD OF PROFESSIONAL LAND SURVEYING FIRM REGISTRATION NO. 10019600
874 HARPER RD, SUITE 104 - KERRVILLE, TX 78028 - 830-816-1818
JOB No. 24-026-00 SURVEYORS: JB/DB SHEET 1 OF 1

Existing Drainage

Drawings and/or Specifications are original proprietary work and property of the Engineer and intended specifically for this project. Use of items contained herein without consent of the Engineer is prohibited. Drawings illustrate best information available to the Engineer. Field verification of actual elements, conditions, and dimensions is required.



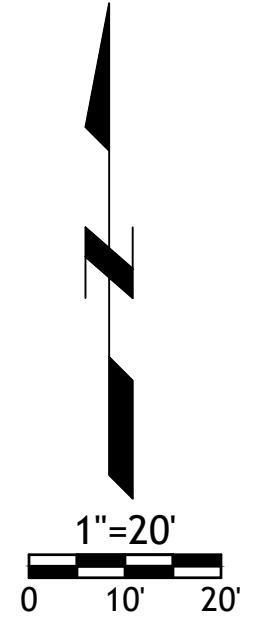
Revisions

GRADING NOTES:

1. EARTHWORK UNDER THE BUILDING SHALL COMPLY WITH THE PROJECT ARCHITECTURAL PLANS. OTHER FILL MATERIAL SHALL BE MADE IN LIFTS NOT TO EXCEED EIGHT INCHES DEPTH COMPACTED TO 95% STANDARD PROCTOR DENSITY. FILL MATERIAL MAY INCLUDE ROCK FROM ON-SITE EXCAVATION IF CAREFULLY PLACED SO THAT LARGE STONES ARE WELL DISTRIBUTED AND VOIDS ARE COMPLETELY FILLED WITH SMALLER STONES. EARTH, SAND OR GRAVEL TO FURNISH A SOLID EMBANKMENT. NO ROCK LARGER THAN THREE INCHES IN ANY DIMENSION NOR ANY SHALE SHALL BE PLACED IN THE TOP 12 INCHES OF EMBANKMENT.
2. AREAS THAT ARE TO BE CUT TO SUBGRADE LEVELS SHALL BE PROOF ROLLED WITH A MODERATELY HEAVY LOADED DUMP TRUCK OR SIMILAR APPROVED CONSTRUCTION EQUIPMENT TO DETECT UNSUITABLE SOIL CONDITIONS.
3. IN ALL AREAS OF EXCAVATION, IF UNSUITABLE SOIL CONDITIONS ARE ENCOUNTERED, A QUALIFIED GEOTECHNICAL ENGINEER SHALL RECOMMEND TO THE OWNER THE METHODS OF UNDERCUTTING AND REPLACEMENT OF PROPERLY COMPACTED, APPROVED FILL MATERIAL. ALL PROOF ROLLING AND UNDERCUTTING SHOULD BE PERFORMED DURING A PERIOD OF DRY WEATHER.
4. CONTRACTOR SHALL USE SILT FENCE OR OTHER MEANS OF CONTROLLING EROSION ALONG THE EDGE OF THE PROPERTY OR OTHER BOTTOM OF SLOPE LOCATIONS.
5. CONTRACTOR IS TO REMOVE AND DISPOSE OF ALL DEBRIS, RUBBISH AND OTHER MATERIALS RESULTING FROM PREVIOUS AND CURRENT DEMOLITION OPERATIONS.
6. THE CONTRACTOR SHALL TAKE ALL PRECAUTIONS NECESSARY TO AVOID PROPERTY DAMAGE TO ADJACENT PROPERTIES DURING THE CONSTRUCTION PHASES OF THIS PROJECT. THE CONTRACTOR WILL BE HELD SOLELY RESPONSIBLE FOR ANY DAMAGES TO THE ADJACENT PROPERTIES OCCURRING DURING THE CONSTRUCTION PHASES OF THIS PROJECT.
7. IT IS NOT THE DUTY OF THE ENGINEER OR THE OWNER TO REVIEW THE ADEQUACY OF THE CONTRACTOR'S SAFETY MEASURES, IN, ON OR NEAR THE CONSTRUCTION SITE AT ANY TIME DURING CONSTRUCTION.
8. THE SITEWORK FOR THIS PROJECT SHALL MEET OR EXCEED ALDI'S STANDARD SITEWORK SPECIFICATIONS.
9. PIPE LENGTHS ARE CENTER TO CENTER OF STRUCTURE OR TO END OF END SECTIONS.
10. HANDICAP STALLS SHALL MEET ADA REQUIREMENTS AND SHALL NOT EXCEED 2% SLOPE IN ANY DIRECTION AT THE BUILDING ENTRY AND ACCESSIBLE PARKING STALLS. SLOPES EXCEEDING 2.0% WILL BE REPLACED AT THE CONTRACTOR'S EXPENSE.
11. CONTRACTOR TO ADJUST DEPTHS OF EXISTING SERVICE LINES AS NECESSARY
12. ALL CONSTRUCTION TRAFFIC, TEMPORARY TRAFFIC CONTROL DEVICES AND PAVEMENT MARKINGS SHALL CONFORM TO REQUIREMENTS OF THE LATEST MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES.

→ Drainage Arrow

Proposed Drainage



GREEN MEADOW
APARTMENTS

3800 GREEN MEADOW DRIVE,
SAN ANGELO, TEXAS 76904

sheet

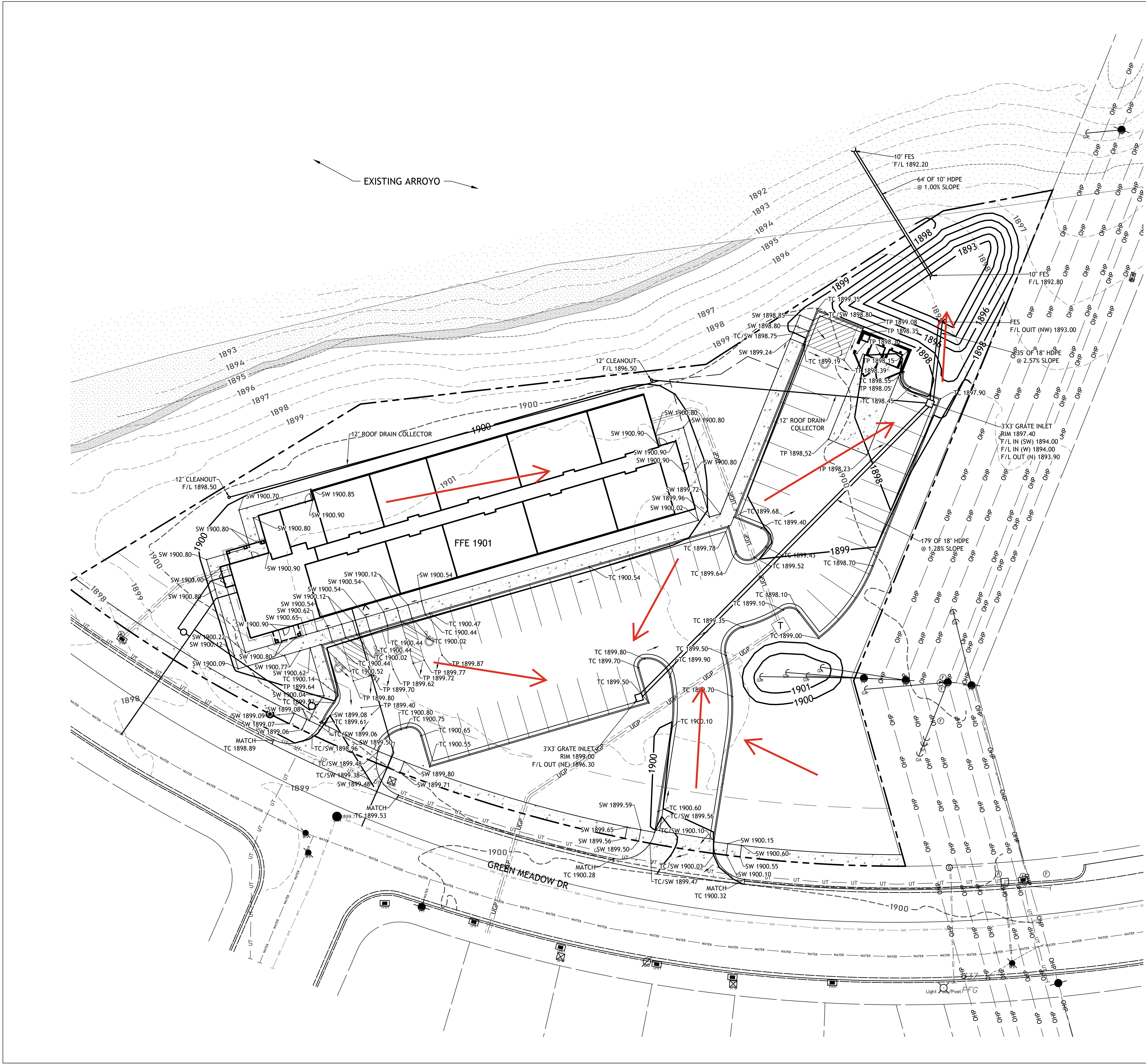
C-5

Civil

GRADING PLAN

permit

1 OCTOBER 2024



Appendix B

- Hydrology Studio Printouts

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Hydrograph by Return Period

Project Name: Green Meadow

Hydrology Studio v 3.0.0.33

10-30-2024

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Outflow (cfs)							
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
1	Rational	Existing		2.886						5.649
2	Mod Rational	Developed		3.219						5.973
3	Pond Route	Green Meadow		2.430						3.854

Hydrograph 2-yr Summary

Project Name: Green Meadow

Hydrology Studio v 3.0.0.33

10-30-2024

Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Existing	2.886	0.08	866	---		
2	Mod Rational	Developed	3.219	0.08	3,767	---		
3	Pond Route	Green Meadow	2.430	0.33	3,632	2	1894.35	1,491

Hydrograph Report

Project Name: Green Meadow

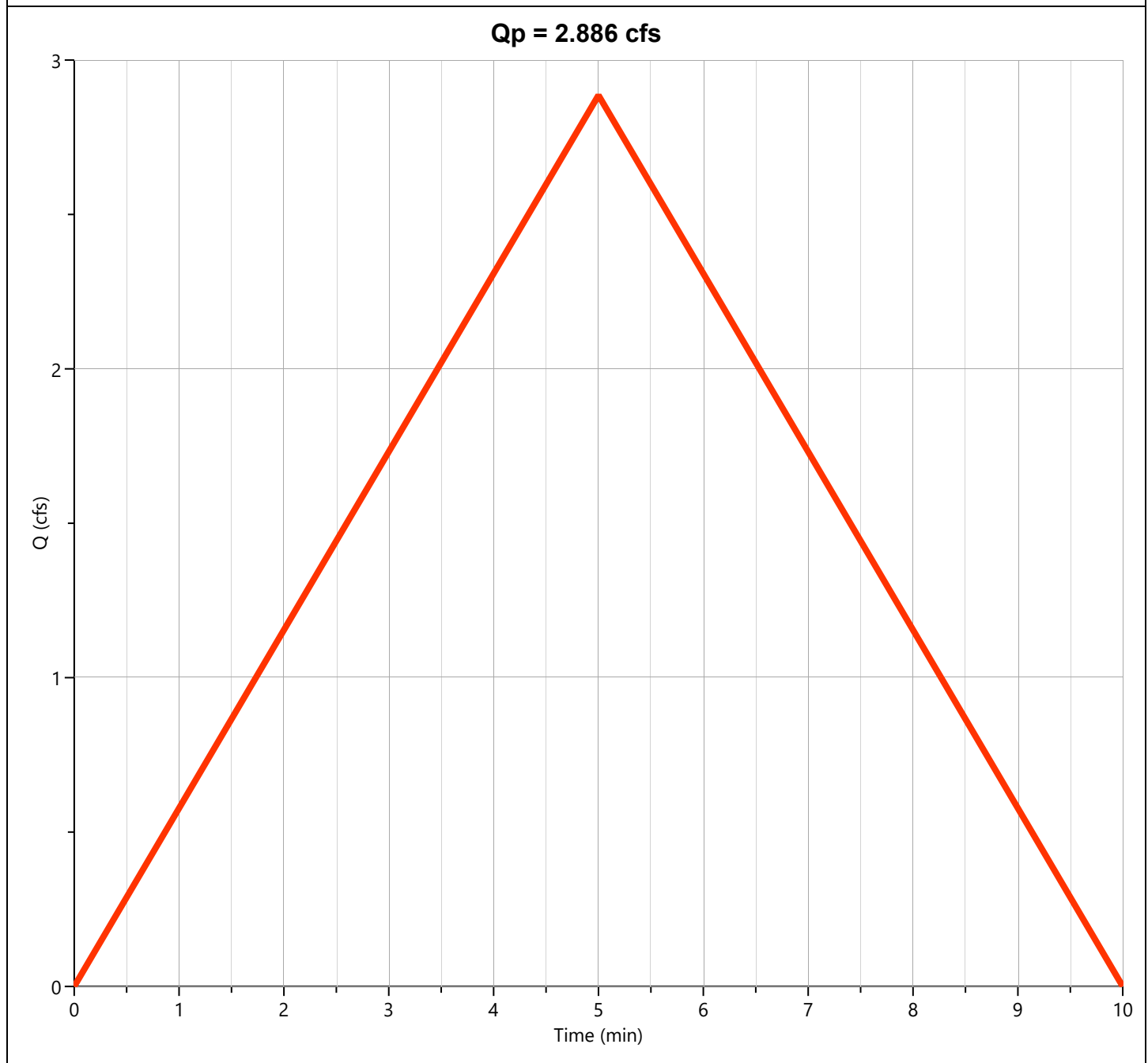
Hydrology Studio v 3.0.0.33

10-30-2024

Existing

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 2.886 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 866 cuft
Drainage Area	= 1.43 ac	Runoff Coeff.	= 0.4
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= San Angelo TX.idf	Intensity	= 5.05 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1

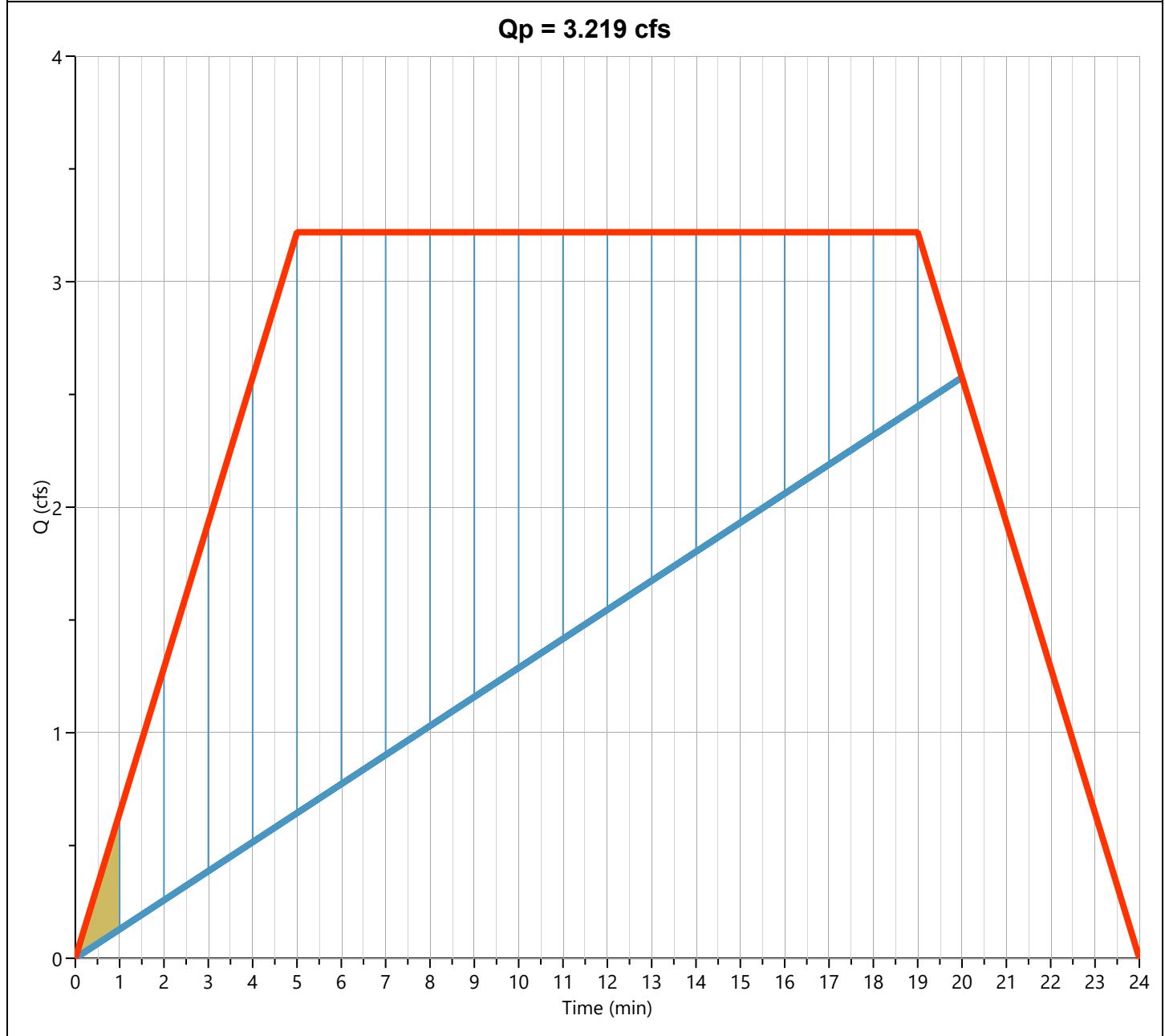


Hydrograph Report

Developed

Hyd. No. 2

Hydrograph Type	= Mod Rational	Peak Flow	= 3.219 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 3,767 cuft
Drainage Area	= 1.43 ac	Runoff Coeff.	= 0.78
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= San Angelo TX.idf	Intensity	= 2.89 in/hr
Freq. Corr. Factor	= 1.00	Storm Duration	= 3.9 x Tc
Target Q	= 2.800 cfs	Required Storage	= 1,751 cuft



Hydrograph Report

Project Name: Green Meadow

Hydrology Studio v 3.0.0.33

10-30-2024

Green Meadow

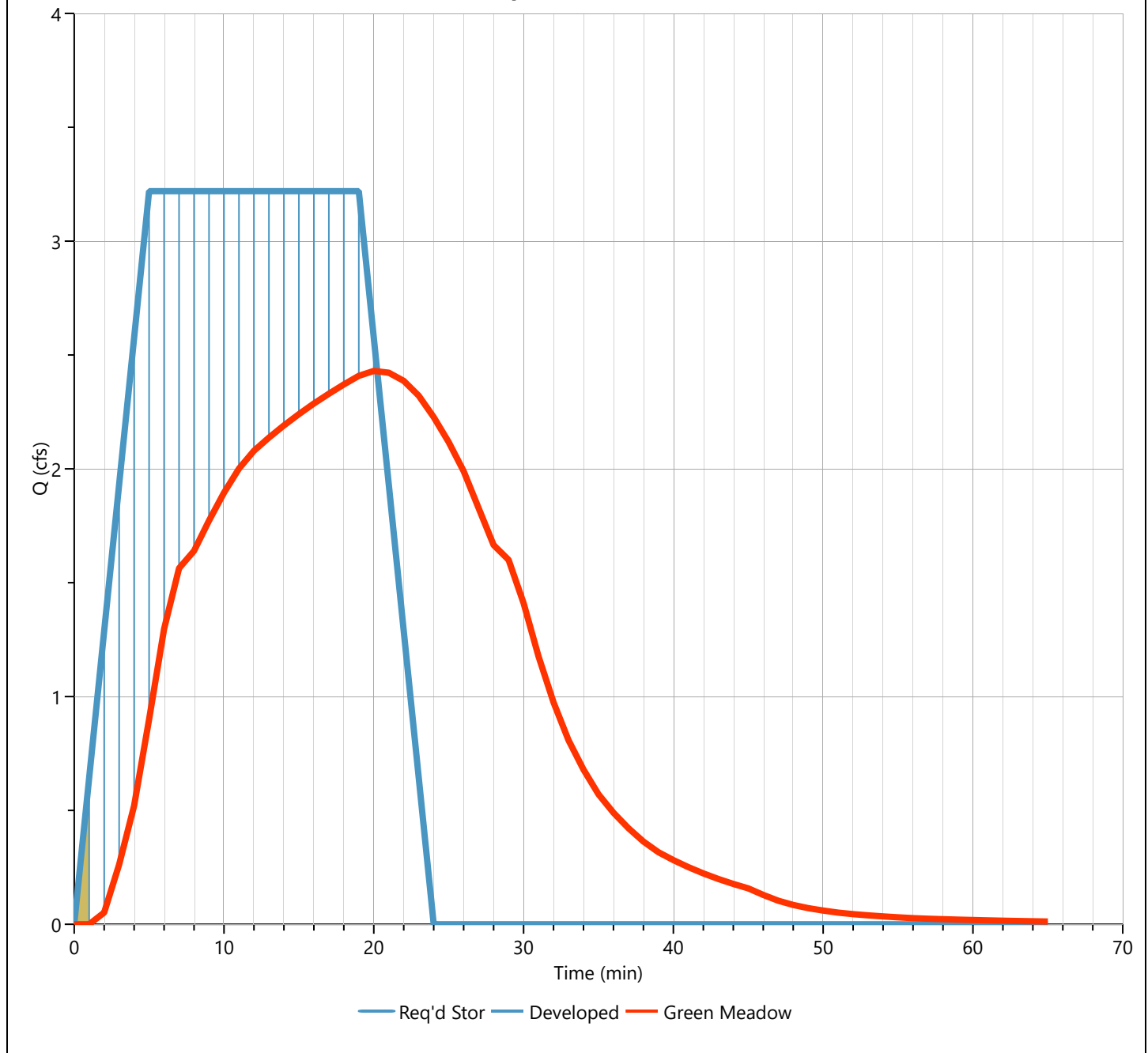
Hyd. No. 3

Hydrograph Type	= Pond Route	Peak Flow	= 2.430 cfs
Storm Frequency	= 2-yr	Time to Peak	= 0.33 hrs
Time Interval	= 1 min	Hydrograph Volume	= 3,632 cuft
Inflow Hydrograph	= 2 - Developed	Max. Elevation	= 1894.35 ft
Pond Name	= Green Meadow	Max. Storage	= 1,491 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 8 min

Qp = 2.430 cfs

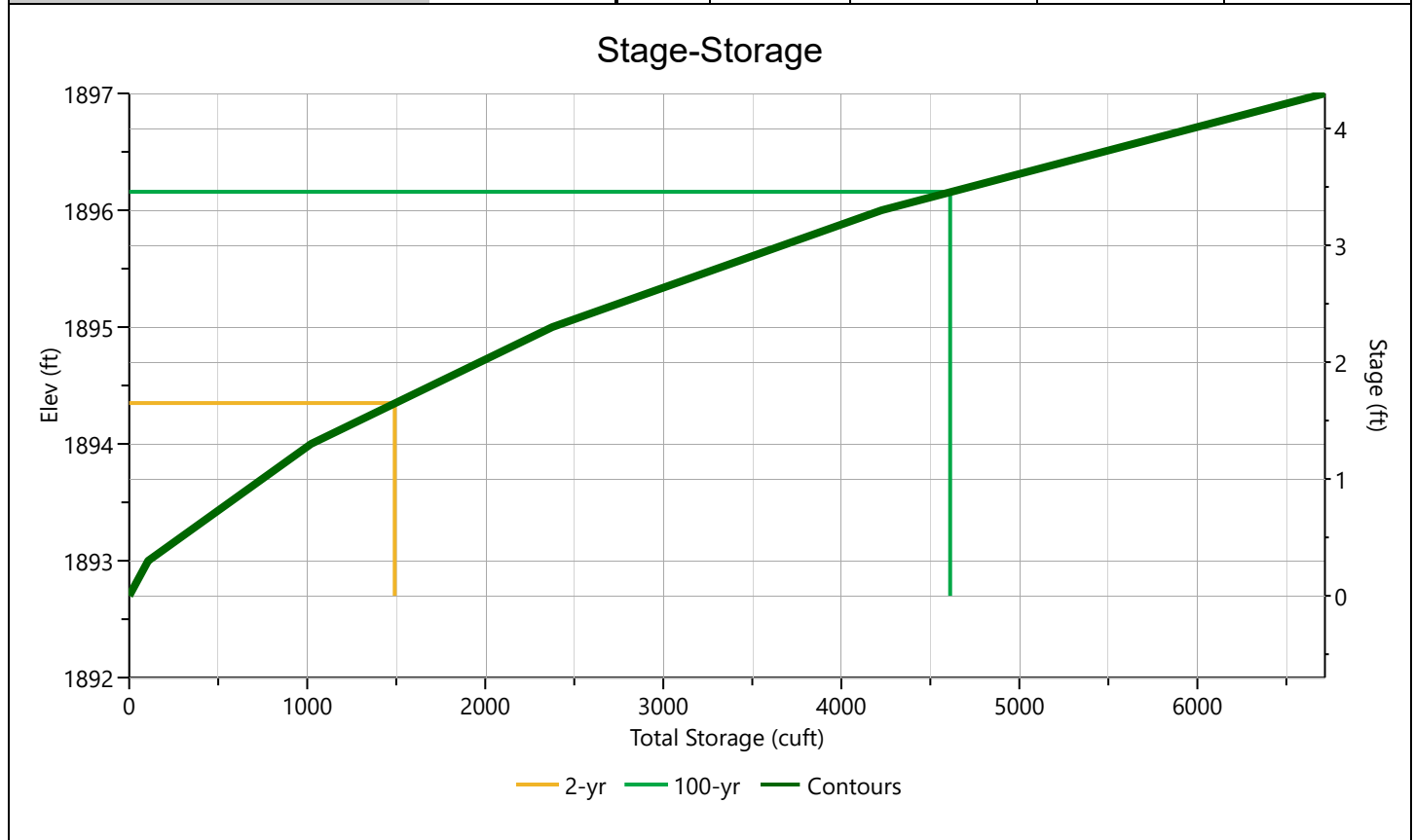


Pond Report

Green Meadow

Stage-Storage

User Defined Contours		Stage / Storage Table				
Description	Input	Stage (ft)	Elevation (ft)	Contour Area (sqft)	Incr. Storage (cuft)	Total Storage (cuft)
Bottom Elevation, ft	1892.70	0.00	1892.70	10	0.000	0.000
Voids (%)	100.00	0.30	1893.00	700	107	107
Volume Calc	Ave End Area	1.30	1894.00	1,130	915	1,022
		2.30	1895.00	1,580	1,355	2,377
		3.30	1896.00	2,120	1,850	4,227
		4.30	1897.00	2,860	2,490	6,717

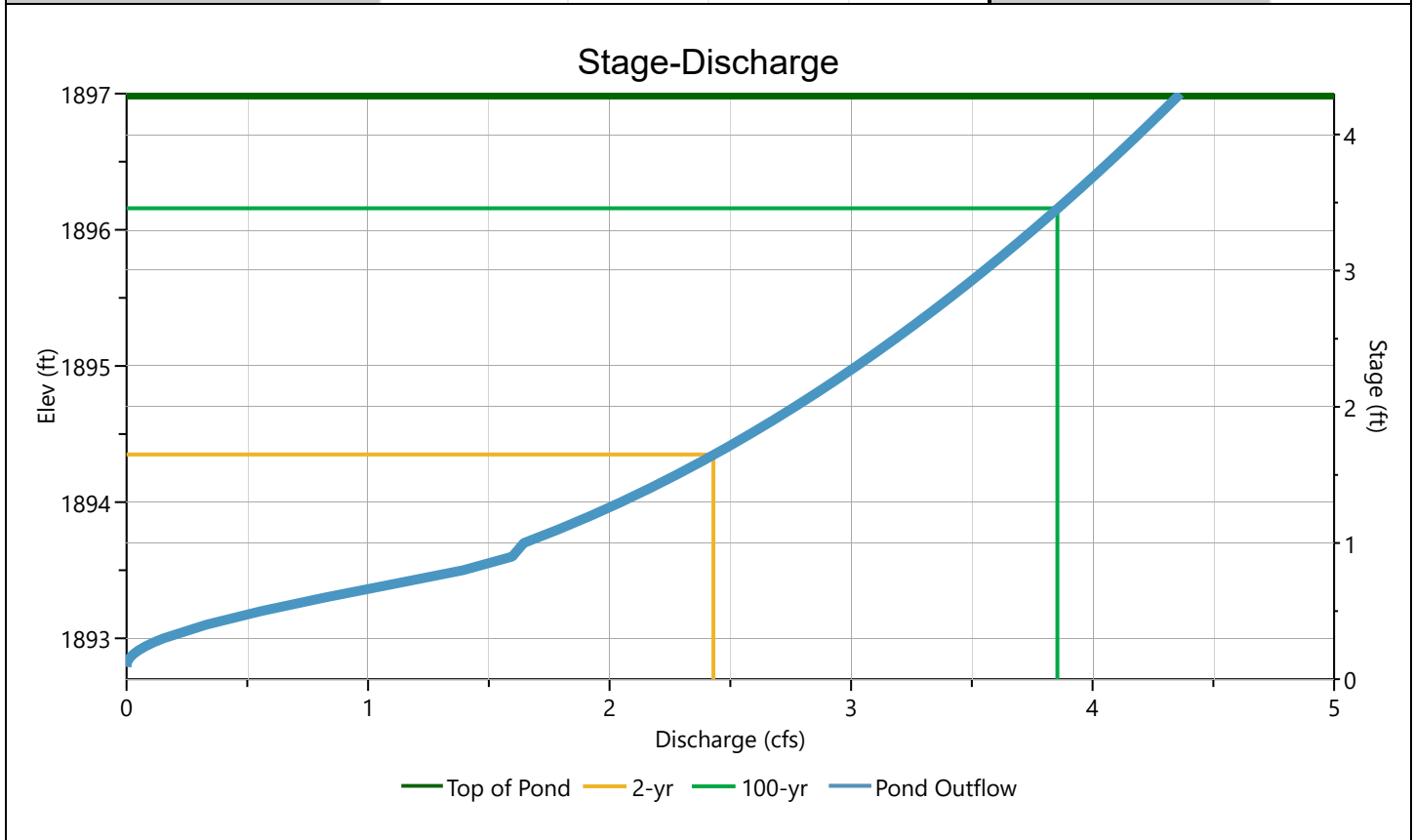


Pond Report

Green Meadow

Stage-Discharge

Culvert / Orifices	Culvert	Orifice			Perforated Riser
		1	2	3	
Rise, in	10				Hole Diameter, in
Span, in	10				No. holes
No. Barrels	1				Invert Elevation, ft
Invert Elevation, ft	1892.80				Height, ft
Orifice Coefficient, Co	0.60				Orifice Coefficient, Co
Length, ft	60				
Barrel Slope, %	.8				
N-Value, n	0.013				
Weirs	Riser	Weir			Ancillary
		1	2	3	
Shape / Type					Exfiltration, in/hr
Crest Elevation, ft					
Crest Length, ft					
Angle, deg					
Weir Coefficient, Cw					



Pond Report

Project Name: Green Meadow

Hydrology Studio v 3.0.0.33

10-30-2024

Green Meadow

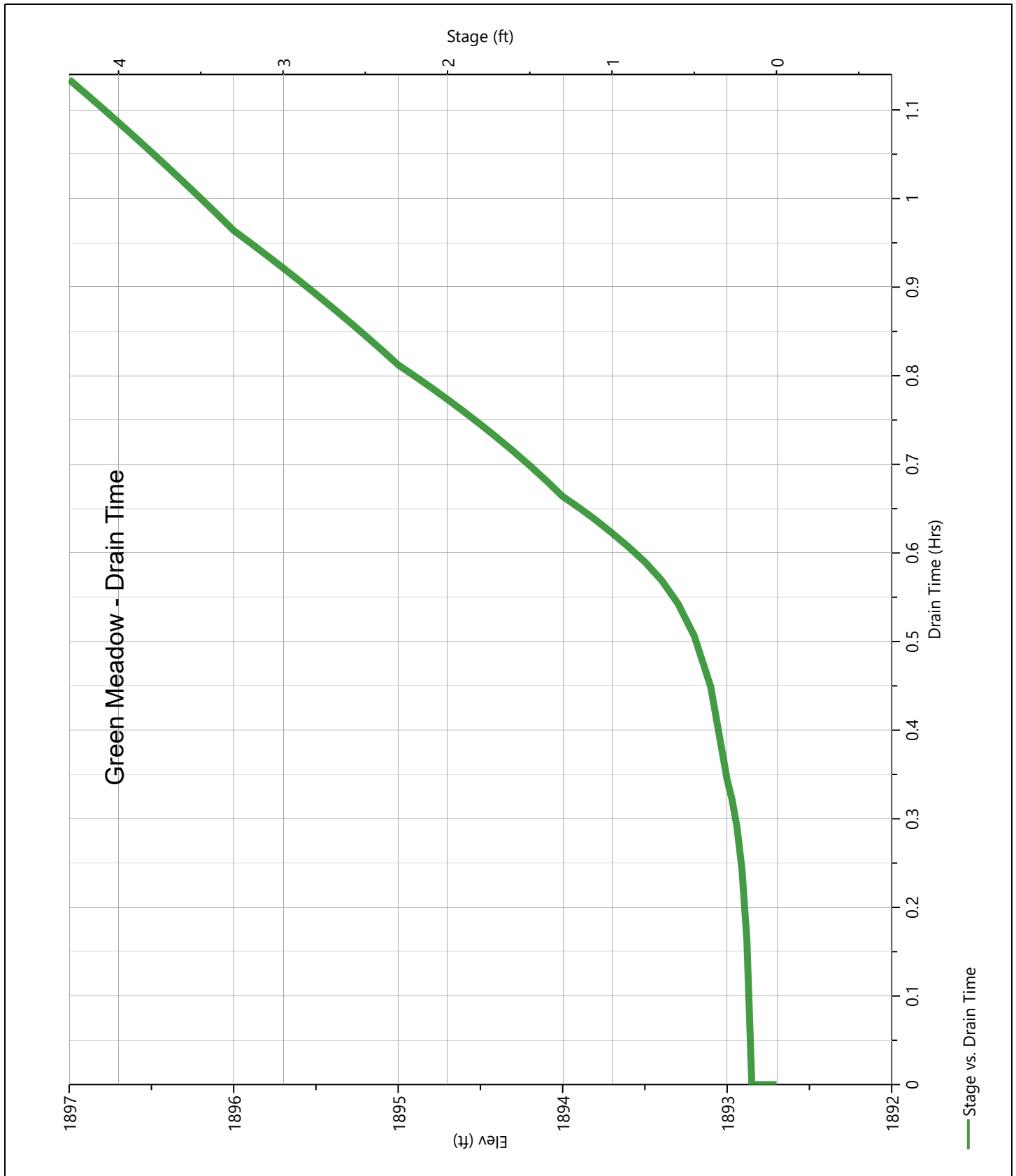
Stage-Storage-Discharge Summary

Stage (ft)	Elev. (ft)	Storage (cuft)	Culvert (cfs)	Orifices, cfs			Riser (cfs)	Weirs, cfs			Pf Riser (cfs)	Exfil (cfs)	User (cfs)	Total (cfs)
				1	2	3		1	2	3				
0.00	1892.70	0.000	0.000											0.000
0.30	1893.00	107	0.153 ic											0.153
1.30	1894.00	1,022	2.047 oc											2.047
2.30	1895.00	2,377	3.023 oc											3.023
3.30	1896.00	4,227	3.754 oc											3.754
4.30	1897.00	6,717	4.364 oc											4.364

Suffix key: ic = inlet control, oc = outlet control, s = submerged weir

Green Meadow

Pond Drawdown

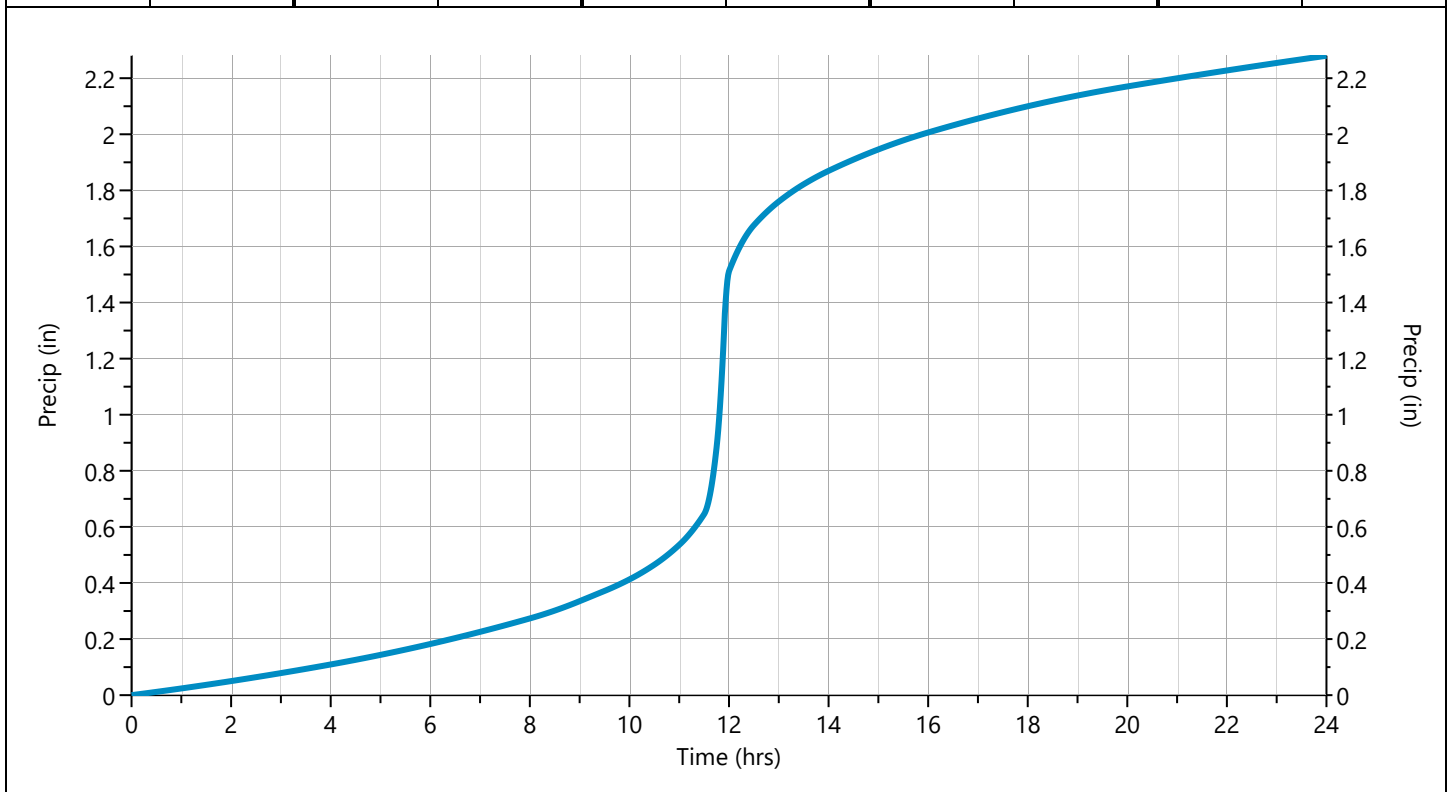


Design Storm Report

Storm Distribution: NRCS/SCS - Type II, 24-hr

Storm Duration	Total Rainfall Volume (in)								
	1-yr	✓ 2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
24 hrs	1.82	2.28	0.00	2.85	3.31	3.94	4.43	4.94	

Incremental Rainfall Distribution, 2-yr									
Time (hrs)	Precip (in)	Time (hrs)	Precip (in)	Time (hrs)	Precip (in)	Time (hrs)	Precip (in)	Time (hrs)	Precip (in)
11.42	0.004226	11.60	0.012809	11.78	0.033312	11.97	0.031372	12.15	0.006411
11.43	0.004286	11.62	0.014308	11.80	0.037483	11.98	0.021878	12.17	0.006266
11.45	0.004347	11.63	0.015808	11.82	0.041655	12.00	0.012385	12.18	0.006122
11.47	0.004408	11.65	0.017308	11.83	0.045826	12.02	0.007821	12.20	0.005977
11.48	0.004469	11.67	0.018808	11.85	0.049998	12.03	0.007421	12.22	0.005833
11.50	0.004530	11.68	0.020307	11.87	0.054170	12.05	0.007277	12.23	0.005689
11.52	0.005327	11.70	0.021807	11.88	0.058341	12.07	0.007133	12.25	0.005544
11.53	0.006810	11.72	0.023307	11.90	0.062513	12.08	0.006988	12.27	0.005400
11.55	0.008309	11.73	0.024806	11.92	0.066684	12.10	0.006844	12.28	0.005255
11.57	0.009809	11.75	0.026306	11.93	0.043606	12.12	0.006699	12.30	0.005111
11.58	0.011309	11.77	0.028920	11.95	0.040866	12.13	0.006555	12.32	0.004967



Hydrograph 100-yr Summary

Project Name: Green Meadow

Hydrology Studio v 3.0.0.33

10-30-2024

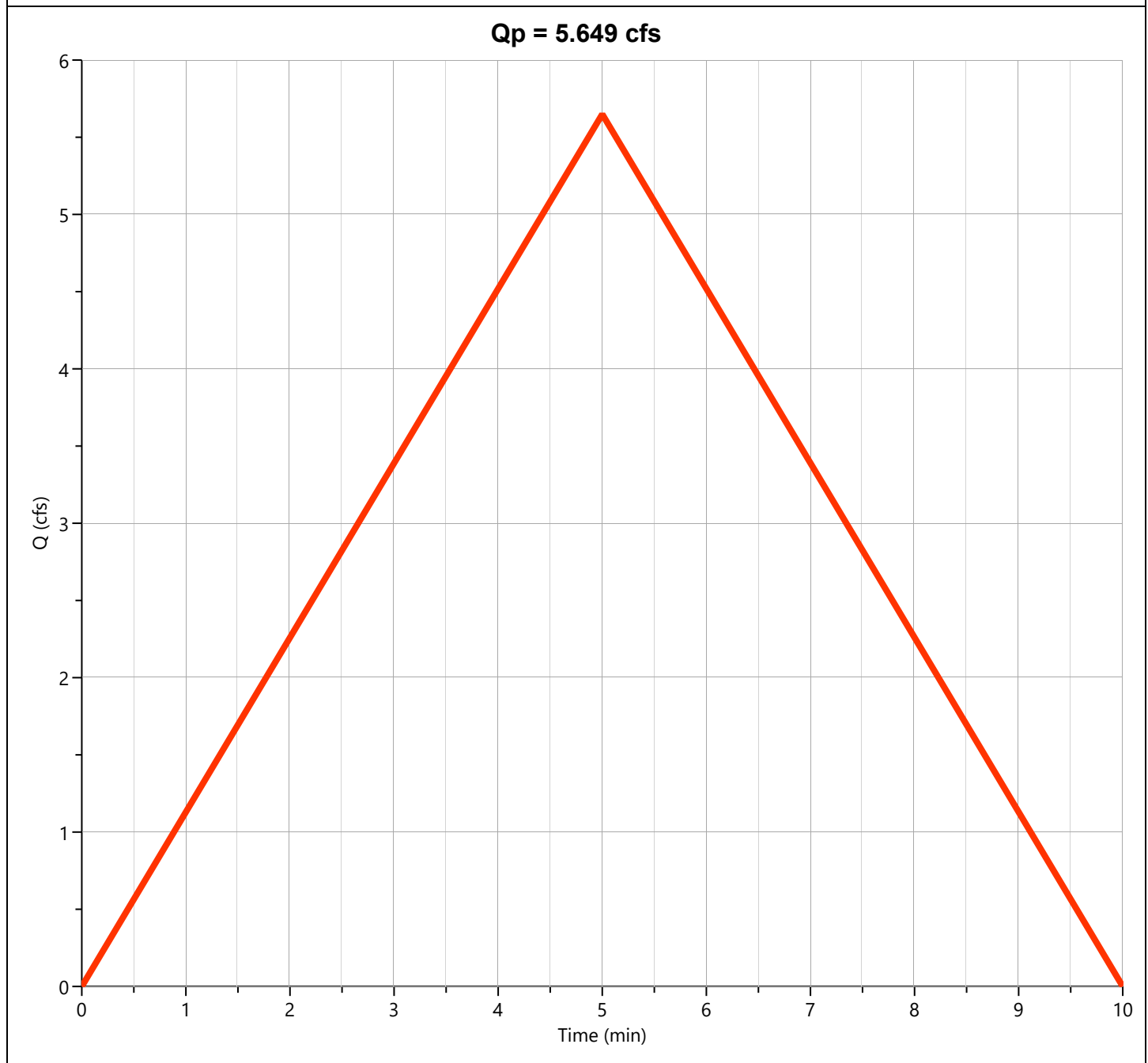
Hyd. No.	Hydrograph Type	Hydrograph Name	Peak Flow (cfs)	Time to Peak (hrs)	Hydrograph Volume (cuft)	Inflow Hyd(s)	Maximum Elevation (ft)	Maximum Storage (cuft)
1	Rational	Existing	5.649	0.08	1,695	---		
2	Mod Rational	Developed	5.973	0.08	9,855	---		
3	Pond Route	Green Meadow	3.854	0.48	9,638	2	1896.16	4,611

Hydrograph Report

Existing

Hyd. No. 1

Hydrograph Type	= Rational	Peak Flow	= 5.649 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 1,695 cuft
Drainage Area	= 1.43 ac	Runoff Coeff.	= 0.4
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= San Angelo TX.idf	Intensity	= 9.88 in/hr
Freq. Corr. Factor	= 1.00	Asc/Rec Limb Factors	= 1/1



Hydrograph Report

Project Name: Green Meadow

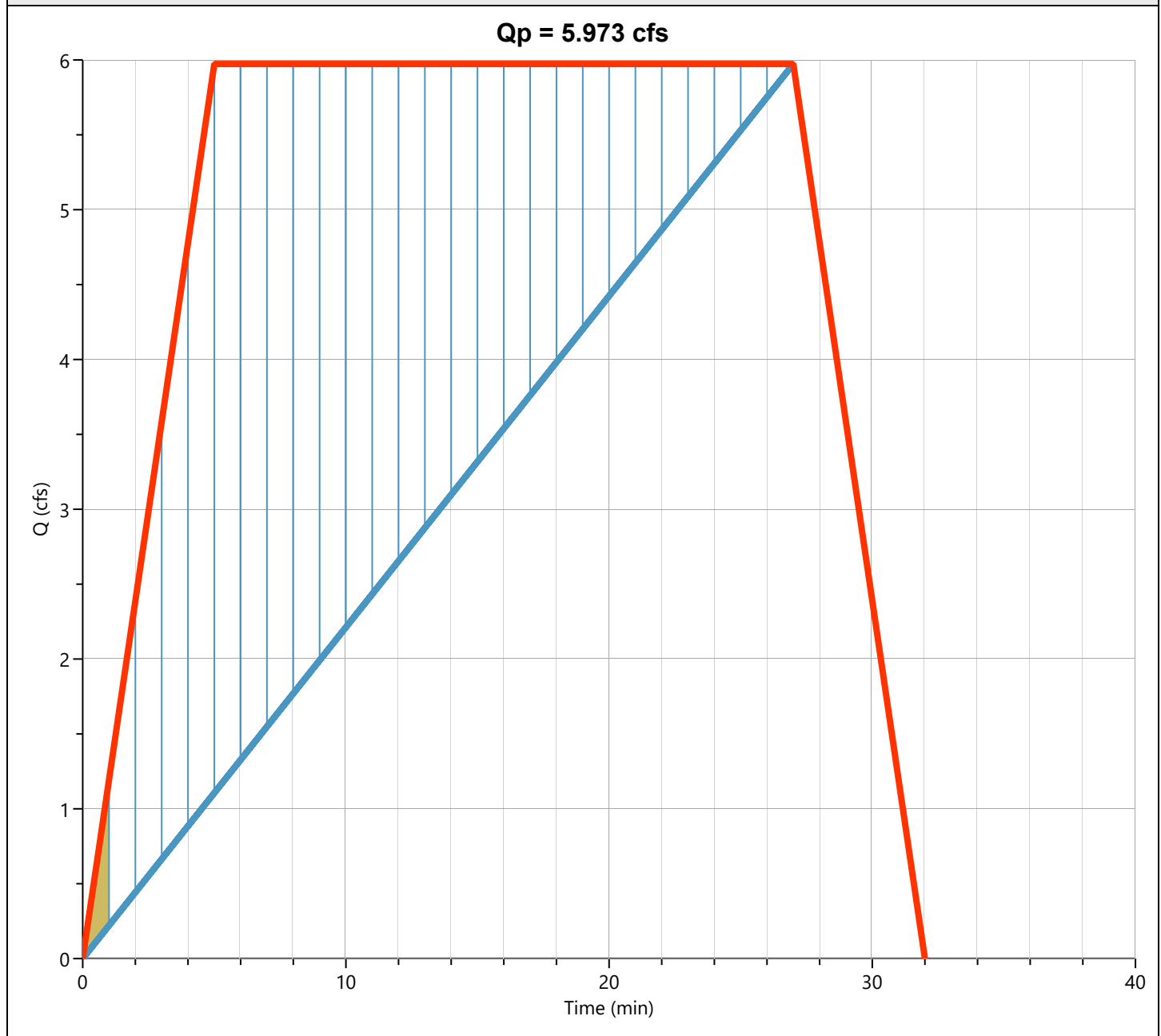
Hydrology Studio v 3.0.0.33

10-30-2024

Developed

Hyd. No. 2

Hydrograph Type	= Mod Rational	Peak Flow	= 5.973 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.08 hrs
Time Interval	= 1 min	Runoff Volume	= 9,855 cuft
Drainage Area	= 1.43 ac	Runoff Coeff.	= 0.78
Tc Method	= User	Time of Conc. (Tc)	= 5.0 min
IDF Curve	= San Angelo TX.idf	Intensity	= 5.35 in/hr
Freq. Corr. Factor	= 1.00	Storm Duration	= 5.5 x Tc
Target Q	= 5.600 cfs	Required Storage	= 4,479 cuft



Hydrograph Report

Green Meadow

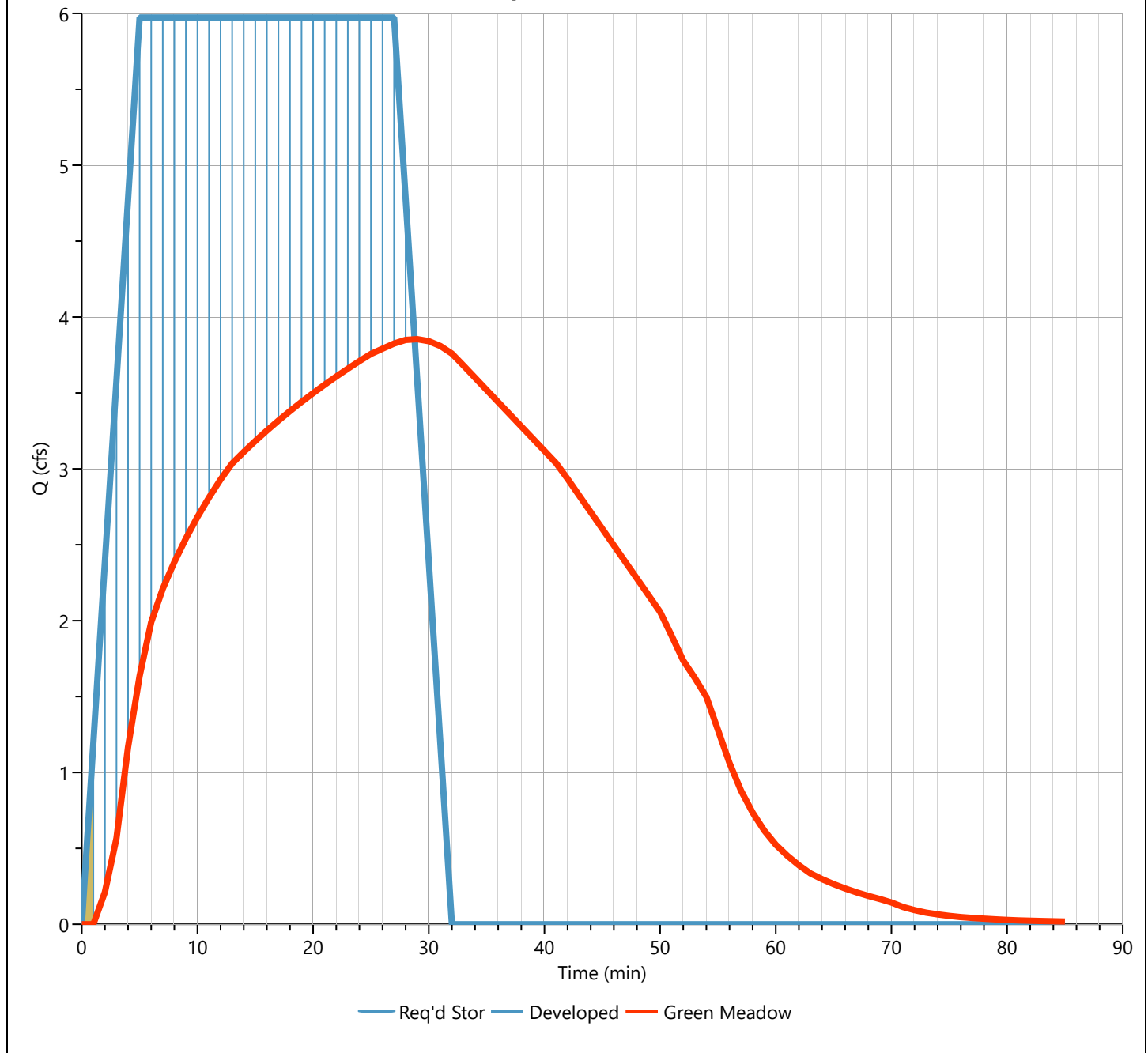
Hyd. No. 3

Hydrograph Type	= Pond Route	Peak Flow	= 3.854 cfs
Storm Frequency	= 100-yr	Time to Peak	= 0.48 hrs
Time Interval	= 1 min	Hydrograph Volume	= 9,638 cuft
Inflow Hydrograph	= 2 - Developed	Max. Elevation	= 1896.16 ft
Pond Name	= Green Meadow	Max. Storage	= 4,611 cuft

Pond Routing by Storage Indication Method

Center of mass detention time = 14 min

Qp = 3.854 cfs

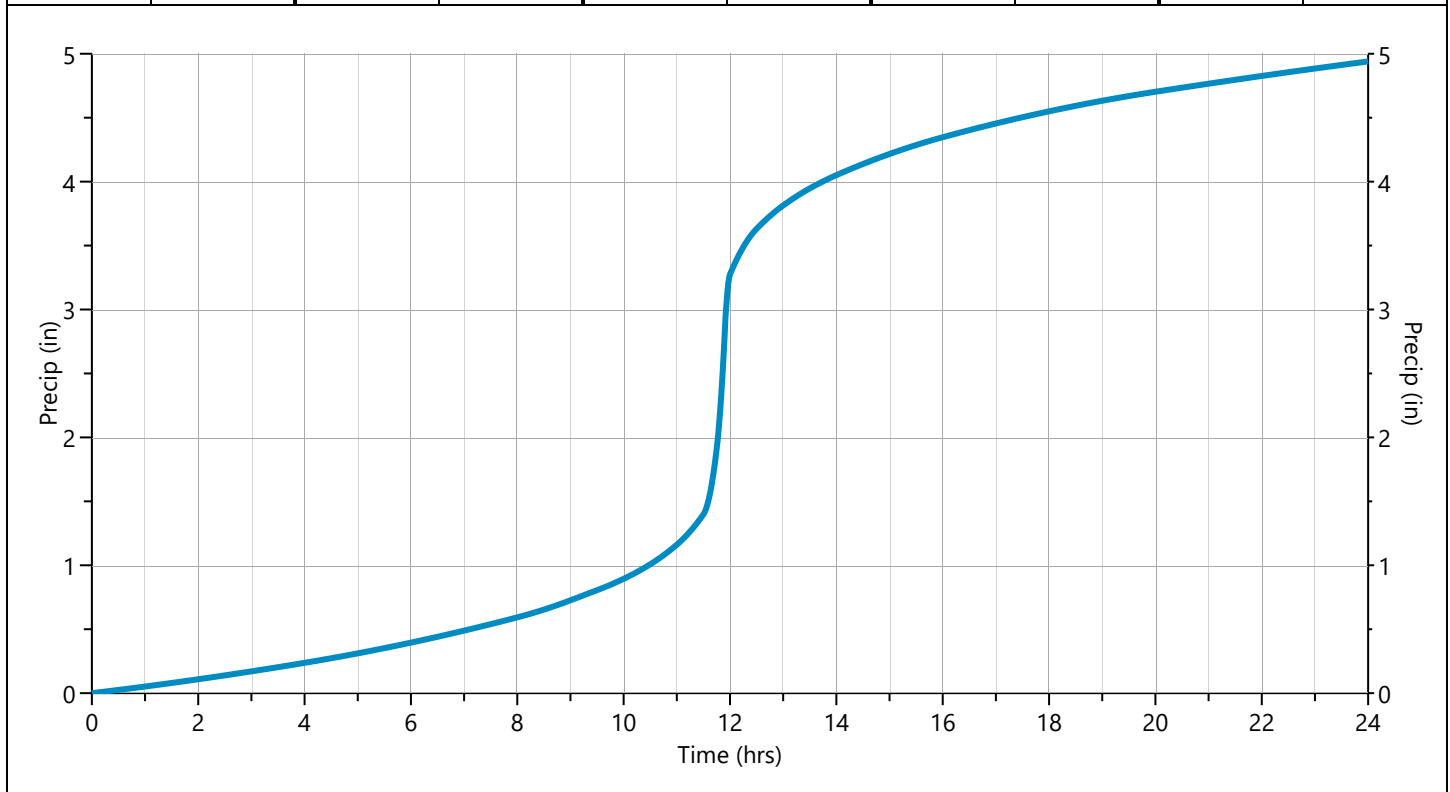


Design Storm Report

Storm Distribution: NRCS/SCS - Type II, 24-hr

Storm Duration	Total Rainfall Volume (in)							
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	✓ 100-yr
24 hrs	1.82	2.28	0.00	2.85	3.31	3.94	4.43	4.94

Incremental Rainfall Distribution, 100-yr									
Time (hrs)	Precip (in)	Time (hrs)	Precip (in)	Time (hrs)	Precip (in)	Time (hrs)	Precip (in)	Time (hrs)	Precip (in)
11.42	0.009155	11.60	0.027752	11.78	0.072175	11.97	0.067973	12.15	0.013890
11.43	0.009287	11.62	0.031001	11.80	0.081214	11.98	0.047403	12.17	0.013577
11.45	0.009419	11.63	0.034251	11.82	0.090252	12.00	0.026834	12.18	0.013264
11.47	0.009551	11.65	0.037500	11.83	0.099290	12.02	0.016946	12.20	0.012951
11.48	0.009682	11.67	0.040750	11.85	0.108329	12.03	0.016080	12.22	0.012638
11.50	0.009814	11.68	0.043999	11.87	0.117367	12.05	0.015767	12.23	0.012325
11.52	0.011542	11.70	0.047248	11.88	0.126406	12.07	0.015454	12.25	0.012012
11.53	0.014754	11.72	0.050498	11.90	0.135444	12.08	0.015141	12.27	0.011699
11.55	0.018004	11.73	0.053747	11.92	0.144482	12.10	0.014828	12.28	0.011387
11.57	0.021253	11.75	0.056997	11.93	0.094479	12.12	0.014515	12.30	0.011074
11.58	0.024503	11.77	0.062659	11.95	0.088542	12.13	0.014203	12.32	0.010761



IDF Report

Equation Coefficients	Intensity = B / (Tc + D)^E (in/hr)								
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
B	17.8240	56.3129	0.0000	70.8619	72.7473	74.7560	104.5665	111.0816	
D	3.2000	10.7000	0.0000	11.7000	11.2000	9.8000	13.6000	14.6000	
E	0.6259	0.8761	0.0000	0.8558	0.8278	0.7975	0.8269	0.8134	

Minimum Tc = 5 minutes

Tc (min)	Intensity Values (in/hr)								
	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
Cf	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
5	4.78	5.05	0	6.37	7.25	8.72	9.32	9.88	
10	3.54	3.96	0	5.09	5.81	6.91	7.66	8.21	
15	2.90	3.28	0	4.26	4.87	5.77	6.53	7.06	
20	2.49	2.80	0	3.68	4.22	4.99	5.72	6.22	
25	2.20	2.46	0	3.25	3.73	4.41	5.10	5.57	
30	1.99	2.19	0	2.91	3.35	3.96	4.61	5.06	
35	1.82	1.98	0	2.64	3.05	3.60	4.21	4.64	
40	1.69	1.81	0	2.42	2.80	3.31	3.89	4.29	
45	1.58	1.66	0	2.24	2.59	3.07	3.61	4.00	
50	1.48	1.54	0	2.08	2.41	2.86	3.37	3.74	
55	1.40	1.44	0	1.95	2.26	2.68	3.17	3.52	
60	1.33	1.35	0	1.83	2.13	2.53	2.99	3.33	

Cf = Correction Factor applied to Rational Method runoff coefficient.

