

GENERAL NOTES - STRUCTURAL

1. The contractor shall verify dimensions and conditions before construction and notify the engineer of any discrepancies, inconsistencies, or difficulties affecting the work before proceeding. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. All conflicts, inconsistencies, or other difficulties affecting structural work shall be called to the architect or engineer's attention for direction before proceeding.
2. All design and construction work for this project shall conform to the requirements of the 2018 International Building Code, as amended by the City of Andover, Kansas.
3. These drawings are for this specific project and no other use is authorized.

5. Structural Design Load Criteria:
- A. Dead Load:
- Roofs = 20 psf
- Rooftop Patio = 100 psf
- B. Live Load:
- Roofs = 25 psf
- Floors = 40 psf
- Maintenance Platform = 40 psf
- Public = 100 psf

- C. Snow:
- $P_g = 20 \text{ psf}$ ,  $C_e = 1.0$
- $P_f = 14 \text{ psf}$ ,  $P_s = 14 \text{ psf}$ ,  $P_m = 20 \text{ sf}$
- $I_s = 1.0$ ,  $C_d = 1.0$ ,  $G = 1.0$
- Drift: 4 unbalanced snow loads per ASCE/SEI 7-10

- D. Lateral Loads:
- 1) Wind  $V = 115 \text{ mph}$ , exposure B,  $G_{CPI} = +/- 1.08$
- Design wind pressures to be used for the design of exterior components and cladding materials on the designated zones of walls and roof structures shall be per Section 30.7.1 and Table 30.7.1-2 of ASCE/SEI 7-10. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable.
- 2) Seismic  $C_s = 5s + 0.105 S_1 + 0.034$ ,  $I_E = 1.0$
- Site Classification D (Assumed).
- Seismic Design Category B
- Basic Seismic Force-Resisting System:
- A1-T Light-Framed Walls with Shear Panels of All Other Materials

- E. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the 2018 International Building Code.

6. Concrete:

- A. All concrete for foundations (walls, grade beams, and footings) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 8 gallons of water per 100 pounds of cement and not over 4 inches of slump.
- B. All concrete for interior flat work shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 560 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 8 gallons of water per 100 pounds of cement and not over 4 inches of slump.
- C. Concrete for exterior flatwork shall have a minimum design compressive strength of 4500 psi in 28 days, with not less than 560 pounds of cement per cubic yard of concrete, not over 8 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrainment, and a maximum of 4 inches of slump.
- D. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.
- E. The preceding minimum mix requirements may have up to 15% maximum of the cement content replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced.
- F. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1745 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be tapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior corners, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over free-draining granular material as prescribed by the project soils report.
- G. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 315 and meet requirements of ACI 318, current editions.
- H. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement.
- I. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer.
- J. No aluminum items shall be embedded in any concrete.

7. Reinforcing Steel:

- A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform to the requirements of ASTM A185.
- B. Clear minimum coverage of concrete over reinforcing steel shall be as follows:
- Concrete placed against earth 3"
- Formed concrete against earth 2"
- Slabs 1"
- Other 1"
- All coverage shall be nominal bar diameter minimum.
- C. All dowels shall be the same size and spacing as adjoining main bars (splice lap 48 bar diameters or 30" minimum unless noted otherwise).
- D. At corners of all walls, beams, and grade beams supply corner bars (minimum 2"-6" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars.
- E. Bars marked continuous shall be lapped 48 bar diameters (3'-0" minimum) at splices and embedments, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise.
- F. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.
- G. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way.

8. Structural Steel:

- A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel. Hollow Structural Sections (HSS) shall be ASTM A500 grade B. Fabrication and erection shall be in accordance with AISC 303-05 Code of Standard Practice for Steel Buildings and Bridges in the 13th Edition of the AISC Steel Construction Manual.
- B. All welding shall conform to the recommendations of the AWS.
- C. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All beam connections shall be designed per the AISC Manual of Steel Construction Framed Beam Connections for 40 kip reactions, and shall account for eccentricity when the bolt line is more than 2" from the center of the support. All connections must be two bolt minimum.
- D. All anchor bolts shall be 3/4" diameter, ASTM F1554, Grade 36 unless noted otherwise.

9. Foundations:

- A. The soil investigation was prepared by Terracon, the report number is 0124560 and their latest report number is 418-402-1771.
- B. Spread footings and continuous wall footings are designed to bear on soil capable of safely sustaining 2500 psf.
- C. Contractor shall provide for denaturing at excavations from either surface water or seepage.
- D. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense.
- E. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.

10. Concrete Block Masonry

- A. Concrete block used in exterior walls or load bearing walls shall meet the requirements of ASTM C90 and have a minimum net compressive strength of 2500 psi and laid up using type N mortar such that  $f_m$  equals 1500 psi. Mortar shall be volume proportion based cement lime mortar. Proportioning shall be completed by box measure. Any block in contact with earth shall be normal weight units, laid using type "S" mortar and grouted solid.
- B. The contractor shall provide adequate temporary bracing for all masonry walls during construction.
- C. All concrete block shall have 9 gage (or larger) horizontal joint reinforcing (ladder or truss) per architectural drawings and specifications (16" maximum vertical spacing).
- D. Concrete block shall be reinforced as follows in 8" walls unless noted otherwise:
- 1) Vertical reinforcing shall be a minimum of 1 - #4 bar in 8" walls at 4'-0" on center, at each corner, at each door and window jamb, each side of control joints and in the end wall of each length of wall. Lap splices for masonry vertical reinforcing shall be 48 bar diameters or 24" minimum.
- 2) Horizontal reinforcing:
- A) Horizontal joint reinforcing as noted above.
- B) Continuous horizontal bars shall be included per section or detail in bond beam or optional running bond beam where noted. Where bond beams and continuous at corners of walls, supply corner bars matching size of horizontal bars (minimum 2'-0" or 40 bar diameters in each direction).
- E. Grout, where noted above shall have a minimum design ultimate compressive strength of 2500 psi at 28 day test and 3/8" maximum aggregate size.
- F. Lintels over all openings in walls not otherwise covered shall be on 8" x 8" bond beam with 2 - #6 bars in the bottom of the bond beam.

11. Post-Installed Anchors:

- A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluation Service A Special inspection is required for all post-installed anchors.
- B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 308.2 and ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.
- C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.

12. Timber and Wood Framing:

- A. Quality and construction of wood framing members and their fasteners for load supporting purposes not otherwise indicated on the drawings shall be in accordance with the 2018 International Building Code.
- B. All studs and top and bottom plates shall be Douglas Fir No. 2 grade visually graded lumber, with an allowable fiber stress in bending of 400 psi minimum and an elastic modulus of 1600,000 psi unless noted otherwise. All joist, truss members and headers to be No. 2 grade (min) (unless noted otherwise).
- C. Bridging of stud bearing walls and shear walls shall be solid matching sheathing joints.
- D. Joist blocking and bridging shall be solid wood or cross bridging of either wood or metal strips. Spacing in any case, shall not exceed 8'-0".
- E. Wood members and sheathing shall be fastened with number and size of fasteners not less than that set forth in Table 2304.4.1 of the 2018 International Building Code. Floor sheathing shall be APA rated tongue and groove Stud-I-Floor, exposure I, glued and nailed with 10d nails or #10 screws at 6" on center to supports at edges and 12" on center field. Sheathing of shear walls or roof diaphragms shall be edge nailed with 8d common nails at 6" on center and nailed to intermediate framing and/or blocking members with 8d common nails at 12" on center unless otherwise noted on the drawings.
- F. Sill plates shall be bolted to concrete slabs with 1/2" diameter bolts at 32" on center (N.O. Re. shearwall sched). Provide plate washers at sill plate anchors for shearnails per shearnail sched. Plates in direct contact with concrete or masonry shall be treated lumber.
- G. All hangers, ties and connections shown are based on Simpson Strong Tie as the basis of design, provide Simpson Strong Tie or an approved equal. Joist hangers shall be equal to "LUS" for wood application and "LB" for steel weld-on application. Roof truss ties shall be equal to "H2.5A" and tie the roof truss to the top plate (provide 2) "H2.5A" Diagonally across from each other when uplift load shown in truss shop submittal exceeds 600lbs). Roof girder ties shall be equal to a "L6T2", "L6T3" or "L6T4" tie (dependent on number of piles) and tie the truss girder to the top plate. Provide "H4" at the top of each stud to top track when the top track has roof truss attached.
- H. Service condition - dry with moisture content at or below 18% in service.
- I. Laminated strand lumber (LSL) shall have an allowable flexural stress ( $F_b$ ) of 1,700 psi (reduced by size factor) and an elastic modulus ( $E$ ) of 1,300,000 psi.
- J. Laminated veneer lumber (LVL) shall have an allowable flexural stress ( $F_b$ ) of 2,600 psi (reduced by size factor) and an elastic modulus ( $E$ ) of 1,400,000 psi.
- K. Parallel Strand Lumber (PSL) shall have an allowable flexural stress ( $F_b$ ) of 2,400 psi (reduced by size factor) and an elastic modulus ( $E$ ) of 2,000,000 psi. ( $E$ ) = 2,200,000 psi for members > 18").

- L. Pre-engineered wood trusses shall be designed in accordance with the Truss Plate Institute's national design standard for metal-plate connected wood truss construction (ANSI/TPI-1 latest edition). Trusses shall be designed and manufactured by an authorized member of the Wood Truss Council of America (NTCA). Truss design shall conform to specified codes, allowable stress increases, deflection limitations and other applicable criteria of the governing code.
- M. Truss shop drawings showing complete erection and fabrication details and calculations (including connections) shall be submitted to the project architect / engineer for review prior to fabrication and/or erection. Calculations shall bear the seal of a professional engineer, registered in the state of the project location. Shop drawings shall also be submitted to the local government controlling agency when requested by that agency.
- N. All trusses shall be securely braced both during erection and permanently as indicated on the approved truss design drawings and in accordance with TPI's commentary and recommendations for handling, installing and bracing metal-plate connected wood trusses (HIB-41, booklet) and the latest edition of ANSI/TPI-1.

- O. The truss manufacturer shall supply all hardware and fasteners for joining truss members together and fastening truss members to their supports. Metal connector plates shall be manufactured by a member of the Wood Truss Council of America (NTCA) and shall be 20 gauge minimum. Connector plates shall meet or exceed ASTM A653, grade 33, with ASTM A424 galvanized coating designation 660.
- P. Provide truss space directly above and centered over HVAC closets. Refer to Architectural and MEP drawings for exact locations.
- Q. Shipment, handling, and erection of trusses shall be by experienced, qualified persons and shall be performed in a manner so as not to endanger life or property. Apparent truss damage shall be reported to the truss manufacturer for evaluation prior to erection. Cutting or alteration of trusses is not permitted.
- R. Pre-Engineered Floor Trusses Design Criteria:
- Top Chord Dead Load = 30 psf
- Top Chord Live Load = Per General Note 5B
- Bottom Chord Dead Load = 10 psf
- Live Load Deflection = L/480 ( $1/8"$  max)
- Total Load Deflection = L/360
- Refer to Sheet 52.4 for 4th Floor roof & exterior deck load criteria.
- S. Roof Truss Design criteria:
- Top Chord Dead Load = 10 psf
- Top Chord Live Load = 25 psf (Plus Rooftop Equipment)
- Top Chord Snow Load = 20 psf or 14 psf plus Drift
- Bottom Chord Dead Load = 10 psf
- Bottom Chord Live Load = 5 psf
- Live Load Deflection = L/360
- Total Load Deflection = L/300
- Refer to Sheet 52.4 for 4th Floor roof & exterior deck load criteria.
- T. Roof trusses shall be designed per IBC 2018 for net uplift resulting from wind loading as calculated using components and cladding loading.
- U. Construction bracing shall be provided by the contractor as required to keep the building and studs plumb.
- V. Structural members shall not be cut for pipes, etc., unless specifically detailed. Notching and boring of studs and top of plates shall conform to the provisions of section 2308.9.1.0 and 2308.9.1.1 of the IBC. Where top plates or sole plates are cut for pipes, a metal tension tie with minimum 0.008 inches thick and 1/2" inches wide shall be fastened to each plate across and to each side of the opening with not less than (6) 16d nails, in accordance section 2308.9.1.6 of the IBC.
- W. All fasteners for wood to wood connections and wood connections shall be as indicated in structural drawings or manufacturer literature to achieve full capacity of connector. Alternate fasteners may be submitted as a substitution request. Submittal must show that alternative fasteners will not reduce the capacity of the connection.

13. Shop Drawing Review:

- A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc.
- B. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall:
- 1) Review each submission for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.
- 2) Review and approve each submission.
- 3) Stamp each submission as approved.
- C. Bob D. Campbell and Company, Inc. shall assume that no submission comprises a variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation.
- D. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC.
- E. 1) Concrete mix designs and material certificates including admixtures and compounds applied to the concrete after placement.
- 2) Reinforcing steel shop drawings including erection drawings, nail elevations (include all mesh openings) and bending details. Bar list will not be reviewed for correct quantities.
- 3) Structural steel shop drawings including erection drawings and piece details, include connection submittals and miscellaneous framing.
- 4) Miscellaneous anchors shown on the structural drawings.
- 5) Wood truss design calculations and detailed erection and fabrication drawings. Standard stick framing shop drawings need not be submitted.
- 6) Construction and control joint plans and/or elevations.
- F. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrevised material or submissions without GC approval stamp.

14. Structural Special Inspection:

- A. The structural design for this project is based on completion of special inspections during construction in accordance with chapter 17 of the 2018 International Building Code. The owner shall employ one or more qualified special inspectors to provide the required special inspections.
- B. Special inspections shall be required for the items indicated below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those inspections.
- 1) Placement of Concrete
- 2) Testing of Concrete
- 3) Bolts in Concrete
- 4) Placement of Reinforcing Steel
- 5) Verification of Soil Bearing Capacities
- 6) High Strength Bolting
- 7) Drill & Epoxy Bolts
- 8) Structural Welding
- 9) Shear wall installation
- 10) Post-Installed Anchors
- 11) Wood shear walls and holdowns
- 12) Wood gravity framing and placement.
- C. The special inspector shall furnish inspection reports to the building official, owner, architect and structural engineer, and any other designated person.
- D. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building official and structural engineer.
- E. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code.

15. Copyright and Disclaimer:

- A. All drawings in the structural set (S-series drawings) are the copyrighted work of Bob D. Campbell and company, inc. These drawings may not be photographed, traced, or copied in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding and construction. Subcontractors may not reproduce these drawings for any purpose or in any manner.
- B. I, Michael J. Falbe, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc., do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of S-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signed statements may appear elsewhere in the construction document package.

NAILING SCHEDULE (REFER TO NOTES #1 and #2)

CONNECTION	ATTACHMENTS (REF NOTE #3 and #4)	
1 JOIST TO SILL OR GIRDER	3- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
2 BRIDGING TO JOIST	2- 3" x 0.131" NAILS-TOENAIL EACH END	2-8d NAILS-TOENAIL EACH END
3 SOLE PLATE TO JOIST OR BLOCKING & TRUSS TO TOP IT	3" x 0.131" NAILS AT 8"o.c.-TYPICAL FACE NAIL 4-3" x 0.131" NAILS AT 16"o.c.-BRACED WALL PANELS	16d BOX NAILS AT 16"o.c. MAX. FACE NAILING 3-16d BOX NAILS AT 16"o.c. BRACED WALL PANEL
4 TOP PLATE TO STUD	3- 3" x 0.131" NAILS-END NAIL	2-16d NAILS-END NAIL
5 STUD TO SOLE PLATE	4- 3" x 0.131" NAILS-TOENAIL OR 3- 3" x 0.131" NAILS-END NAIL	4-8d NAILS-TOENAIL OR 2-16d NAILS-END NAIL
6 DOUBLE STUDS	3" x 0.131" NAILS AT 8"o.c.-FACE NAIL	16d BOX NAILS AT 24"o.c. MAX. FACE NAIL
7 DOUBLED TOP PLATES	3" x 0.131" NAILS AT 12"o.c.-FACE NAIL	16d BOX NAILS AT 16"o.c. MAX. FACE NAIL
8 DOUBLE TOP PLATE LAPS AND INTERSECTIONS	12-3" x 0.131" NAILS	8-16d NAILS
9 BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	3-3" x 0.131" NAILS -TOENAIL	3-8d NAILS-TOENAIL
10 RIM JOIST TO TOP PLATE	3" x 0.131" NAILS AT 8"o.c.-TOENAIL	10d NAILS AT 6"o.c. MAX.-TOENAIL
11 TOP PLATE LAPS AND INTERSECTIONS	3- 3" x 0.131" NAILS-FACE NAIL	2-16d NAILS-FACE NAIL
12 CONTINUOUS HEADER, TWO PIECES	3" x 0.131" NAILS AT 10"o.c. ALONG EACH EDGE	16d NAILS AT 16"o.c. MAX. ALONG EACH EDGE-TOENAIL
13 CEILING JOISTS TO PLATE	5- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
14 CONTINUOUS HEADER TO STUD	4- 3" x 0.131" NAILS-TOENAIL	4-8d NAILS-TOENAIL
15 CEILING JOISTS, LAPS OVER PARTITIONS	4- 3" x 0.131" NAILS-FACE NAIL	3-16d NAILS-FACE NAIL
16 CEILING JOISTS TO PARALLEL RAFTERS	4- 3" x 0.131" NAILS-FACE NAIL	3-16d NAILS-FACE NAIL
17 RAFTER TO PLATE	3- 3" x 0.131" NAILS-TOENAIL	3-8d NAILS-TOENAIL
18 1" BRACE TO EACH STUD AND PLATE	2- 3" x 0.131" NAILS-FACE NAIL	2-8d NAILS-FACE NAIL
19 BUILT-UP CORNER AND MULTIPLE STUDS	3" x 0.131" NAILS AT 16"o.c.	16d NAILS AT 24"o.c. MAX.
20 BUILT-UP GIRDER AND BEAMS	3" x 0.131" NAILS AT 24"o.c. FACE NAILED TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES 3- 3" x 0.131" NAILS AT ENDS AND EACH SPLICE	20d NAILS AT 32"o.c. MAX-TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES. 2-20d NAILS AT ENDS AND EACH SPLICE
21 BUILT-UP LAMINATED VENEER LUMBER BEAMS	3" x 0.131" NAILS AT 6"o.c. TOP AND BOTTOM ALONG EDGE	16d NAILS AT 12"o.c. TOP AND BOTTOM ALONG EDGE
22 2" PLANKING	4- 3" x 0.131" NAILS AT EACH SUPPORT	16d NAILS AT EACH SUPPORT
23 RIM BOARD TO TRUSS	2 - 3" x 0.131" FACE NAILS (17/18 @ EA TRUSS)	2-10d NAILS - FACE NAILS (17/18 @ EA TRUSS)
24 BUILT-UP STUD PACK COLUMNS	REFER TO DETAIL 6/S/1	REFER TO DETAIL 6/S/1

NOTES:

- 1) ALL NAILS SHALL BE AS NOTED UNLESS OTHERWISE SPECIFIED ON STRUCTURAL DRAWINGS OR ALTERNATE PROVIDED BY ENGINEER IN WRITING.
- 2) CONDITIONS NOT SPECIFIED SHALL BE IN ACCORDANCE WITH CURRENT INTERNATIONAL BUILDING CODE.
- 3) NAILING DESIGNATION:
- 4- 3" x 0.131" NAILS
- DIAMETER IN INCHES
- NAIL LENGTH
- QUANTITY
- 4) ALL NAILS NOTED AS 8d, 10d, 16d, ETC. SHALL BE COMMON NAILS UNLESS NOTED BOX.
- 5) REFER TO SHEARNAIL SCHEDULE FOR ADDTL NAILING REQUIREMENTS

TYPICAL SYMBOL LEGEND:

- A - BEAM OR HEADER PER SCHED ON S/1
- A-U - UPSET BEAM OR HEADER PER SCHED ON S/1
- # - FOOTING TYPE PER SCHED ON S/1
- \* - SHEARNAIL HOLDDOWN TYPE PER SCHED ON S/2
- 5W - SHEARNAIL PER SCHED ON S/2
- C-J - CONSTRUCTION JOINT PER 2/S/3
- S-J - SAW JOINT PER 1/S/3
- - SPAN DIRECTION

JonesGillamRenz  
730 N. Ninth Street, Suite 301  
Solino, KS 67401  
785.827.0386  
jgr@jgarchitects.com

ANDOVER SENIOR LIVING  
NEW APARTMENTS  
ANDOVER, KANSAS



REVISION:

DATE: 1-03-2025

JOB: 24-3385

SHEET NO.:

S1.0


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- NOTES:
1. JAMB STUDS SHALL MATCH SIZE & GRADE OF WALL STUDS UNO.
  2. WHERE BEAM IS NOTED "URSET", ALL JAMB STUDS NOTED WILL EXTEND TO DOUBLE TOP PLATE.
  3. ALL EXTERIOR LUMBER TO BE TREATED.
  4. PROVIDE SQUASH BLOCKS AT TRUSSES & BLOCKING FRAMING WHERE JAMBS OR STUD PACKS ARE DISCONT. QUANTITY TO MATCH JAMB OR STUD PACK ABOVE.
  5. PROVIDE 2x6 SPACER PLs AT HEADERS CONSTRUCTED WITH 2x LUMBER.
  6. AT CONTRACTOR'S OPTION-PROVIDE GULLAM IN LIEU OF PSLs.
  7. REFER TO DETL 5/51/1 FOR MULTI-PLY MEMBER CONNECTION REQUIREMENTS.
  8. REFER TO DETL 5/51/1 FOR MULTI-PLY MEMBER CONNECTION TYPE 24 IN NAILING SCHEDULE ON SHEET 51.0.
  9. REFER TO DETAILS 5/51/1 FOR TYPICAL HEADER CONDITIONS.

STUD BEARING WALL & SHEATHING SCHEDULE	
LOCATION	STUD SIZE AND SPACING
(TYP.) EXTERIOR WALL	2x6 @ 16"oc STUDS
INTERIOR WALL (EXCEPT AT NON TRUSS BRG CORRIDORS)	REFER TO SECTIONS ON SHEET 53.2
INTERIOR WALL (AT NON TRUSS BRG CORRIDORS)	REFER TO SECTIONS ON SHEET 53.2

- NOTES:
1. PROVIDE 2x BLOCKING @ MID HEIGHT (5'-0" MAX. @ ALL LOAD BEARING WALLS NOT SHEATHED ON BOTH SIDES AND ALL 2x8 WALLS.
  2. ALL STUDS TO BE NO. 2 GRADE U.O.
  3. RE: 6/5/11 FOR NAILING OF MULTIPLE STUDS.
  4. REFER TO ARCH/MEP DRAWING FOR LOCATIONS OF FURRED OUT WALLS TO ACCOMMODATE PLUMBING OR MEP ITEMS.

STRUCTURAL DECK & SLAB SCHEDULE	
MARK	DESCRIPTION
	4" CONC. SLAB ATOP 15 MIL VAPOR BARRIER ATOP 4" GRAVEL ATOP 20" LOW VOLUME CHANGE MATERIALS REQ'D BY PROJECT GEOTECH. REPORT. REINF. SLAB w/ 6x6-6/6 WWF. EL. 17C = 100'-0".
FD-1	3/4" PLYWOOD SHEATHING ATTACH w/ 8d NAILS @ 6"oc AT EDGES AND 12"oc AT FIELD.
RD-1	3/4" ZIP STRIP ROOF SHEATHING ATTACH w/ 8d NAILS @ 6"oc AT EDGES AND 12"oc AT FIELD.

- NOTES:
1. SOG = SLAB-ON-GRADE TYPE
  2. FD = FLOOR DECK TYPE
  3. RD = ROOF DECK TYPE

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- NOTES:
1. SPREAD FOOTINGS LOCATED AT INTERIOR SHALL BE POURED MONOLITHIC WITH THE SLAB AS A THICKENED PORTION OF SLAB UNLESS THEY HAVE A STEEL COLUMN BEARING ATOP.
  2. SPREAD FOOTINGS LOCATED AT INTERIOR WITH STEEL COLUMNS BEARING ATOP SHALL BE LOCATED AT 9'-2".
  3. SPREAD FOOTINGS LOCATED AT PERIMETER (EXTERIOR) OF BUILDING SHALL BE POURED MONOLITHIC WITH GRADE BEAMS.

## COLUMN SCHEDULE

TYPE	SIZE	BASE E	SHAPE	ANCHOR BOLTS
(C1)	H563x3x3/8x3/8	1/2"x10"x10"	A	(4) 3/8"ø x 2'-0" Lg
(C2)	H565x5x1/4	1/2"x5 1/2"x13 1/8"	B	(4) 3/8"ø x 2'-0" Lg

### NOTES:

1. SEE PLAN FOR ORIENTATION OF COLUMNS.
2. ALL COLUMNS SHALL BE CONTINUOUS WITH NO SPLICES.
3. AB LENGTH INCLUDES 4" HK ± 4" PROJECTION UNO.
4. UNO. SET COLUMN BASE PLATES ON 1" GROUT TYPICAL.
5. EACH AB SHALL HAVE A 3"x3"x3/8" PLATE WASHER BOT. (IN LIEU OF HK) @ 4" PROJ. ANCHOR WHERE NOTED IN THE SCHED. (4"x4"x3/4" PLATE WASHER @ BOLTS 1"ø AND LARGER).
6. 3/8"ø ANCHOR BOLTS SHALL HAVE A 2"x2"x1/4" PLATE WASHER @ TOP & 1"ø ANCHOR BOLTS SHALL HAVE A 3"x3"x3/8" PLATE WASHER, WELD MASHER TO COLUMN BASE PLATE WITH 1/8" FILLET WELD @ 4- SIDES.
7. ALL ANCHOR BOLTS SHALL BE ASTM F1954, GRADE 36 UNO.

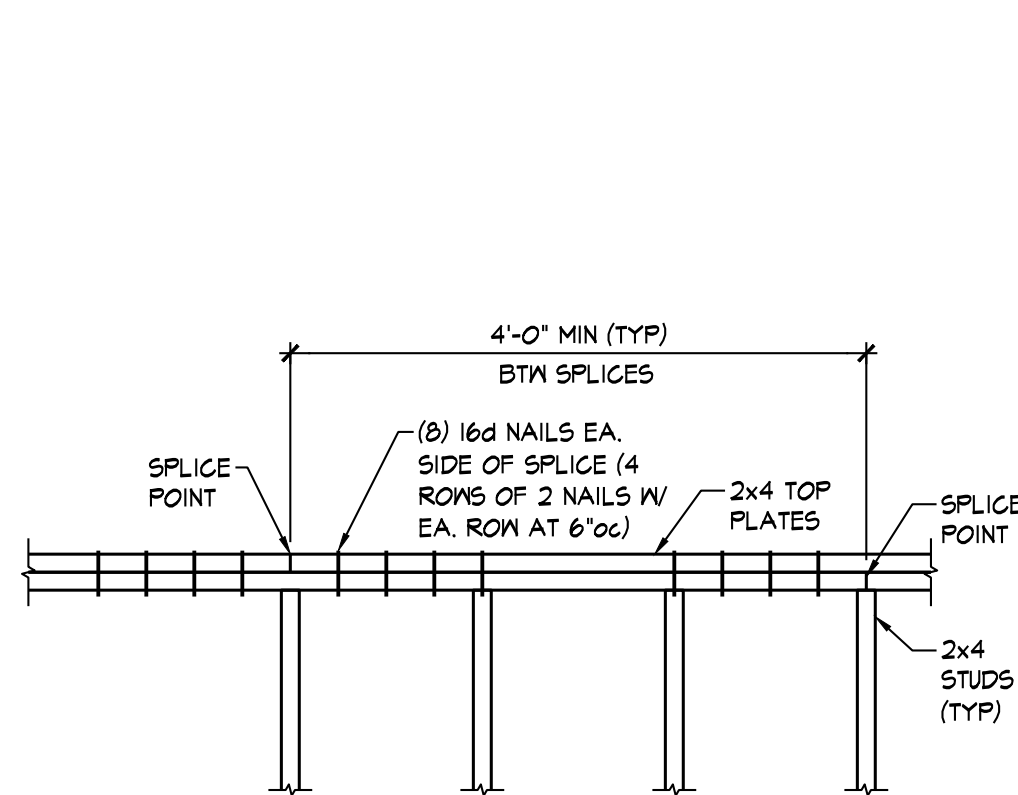
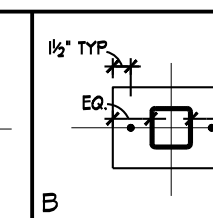
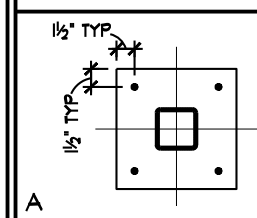
### SHAPE (NOT TO SCALE)

A

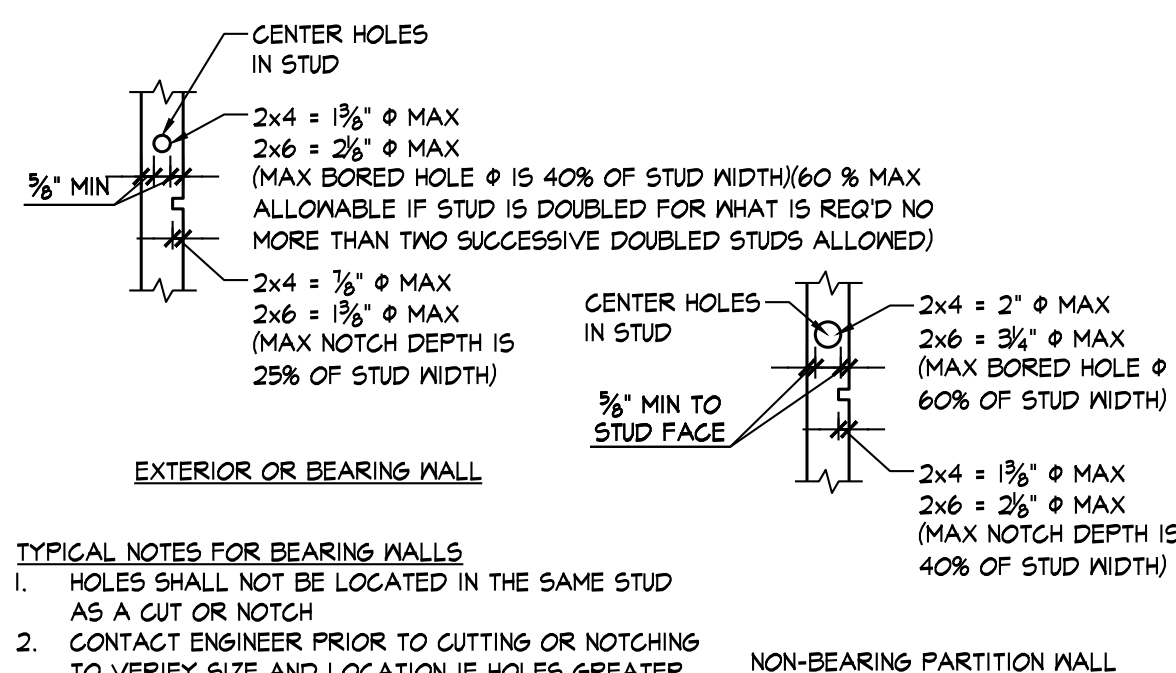
B

- NOTES:
1. SEE PLAN FOR ORIENTATION OF COLUMNS.
  2. ALL COLUMNS SHALL BE CONTINUOUS WITH NO SPLICES.
  3. SET BASE INCLUDES 4" HK & 4" PROJECTION UNO.
  4. UNO. SET LENGTH COLUMNS PLATE ON 1" GROUT TYPICAL.
  5. EACH AB JAIL SHALL HAVE A 3"x3"x $\frac{1}{2}$ " PLATE WASHER BOT. (IN LIEU OF HK) @ 4" FROM TOP WARE NOTED IN THE SCHED. 4"x4"x $\frac{1}{2}$ " PLATE WASHER @ BOL. 1" UNO. LARGER).
  6.  $\frac{3}{4}$ " ANCHOR BOLTS SHALL HAVE A 2"x2"x $\frac{1}{2}$ " PLATE WASHER @ TOP & 1" ANCHOR BOLTS SHALL HAVE A 3"x3"x $\frac{1}{2}$ " PLATE WASHER. NELD WASHER TO COLUMN BASE PLATE WITH  $\frac{1}{4}$ " FILLET MELD @ 4" SIDES.
  7. ALL ANCHOR BOLTS SHALL BE ASTM F1554, GRADE 36 UNO.

SHAPE (NOT TO SCALE)

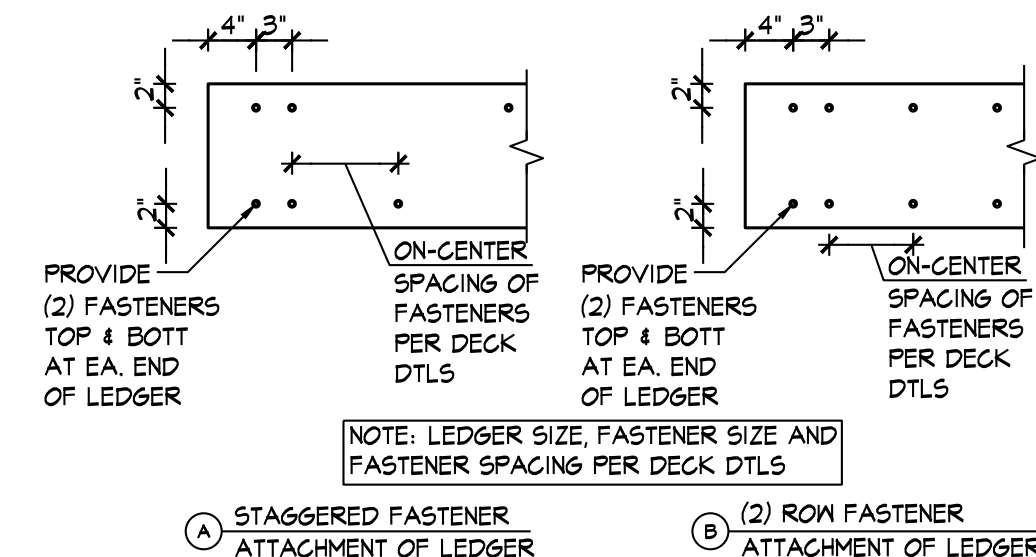


(TYP) TOP PLATE SPLICE

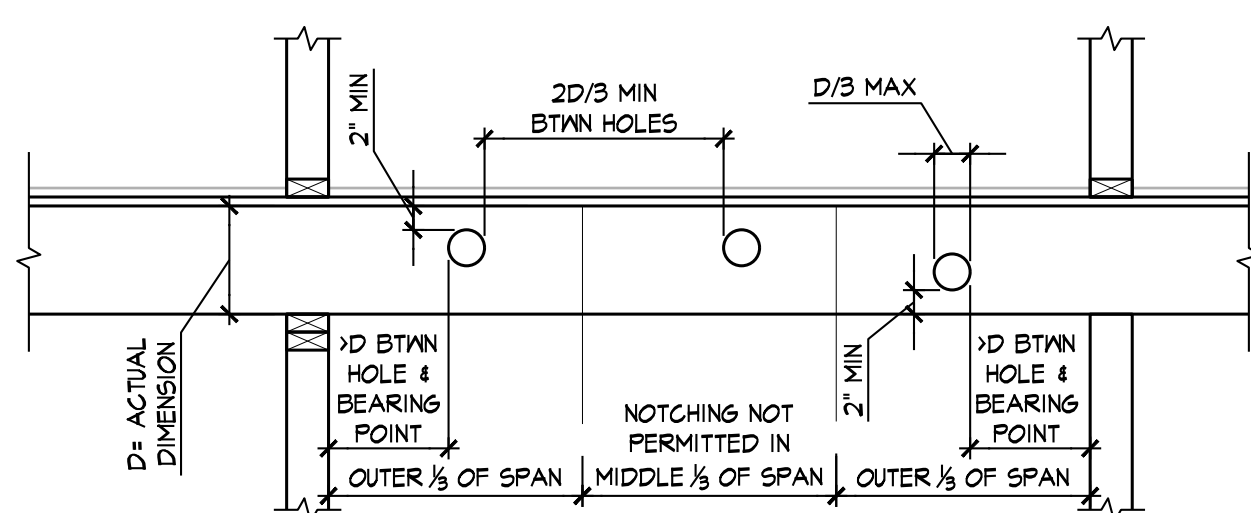


- TYPICAL NOTES FOR BEARING WALLS
1. HOLES SHALL NOT BE LOCATED IN THE SAME STUD AS A CUT OR NOTCH
  2. CONTACT ENGINEER PRIOR TO CUTTING OR NOTCHING TO VERIFY SIZE AND LOCATION IF HOLES GREATER THAN 20% STUD WIDTH OR NOTCHES GREATER THAN 10% STUD WIDTH ARE REQUIRED IN TWO OR MORE CONSECUTIVE STUDS
  3. NOTCHES OR HOLES NOT PERMITTED IN JAMBS, STUD PACKS AND AT ENDS OF SHEARWALLS

SECTION 2  
 $\frac{3}{4}" = 1'-0"$  S.I.

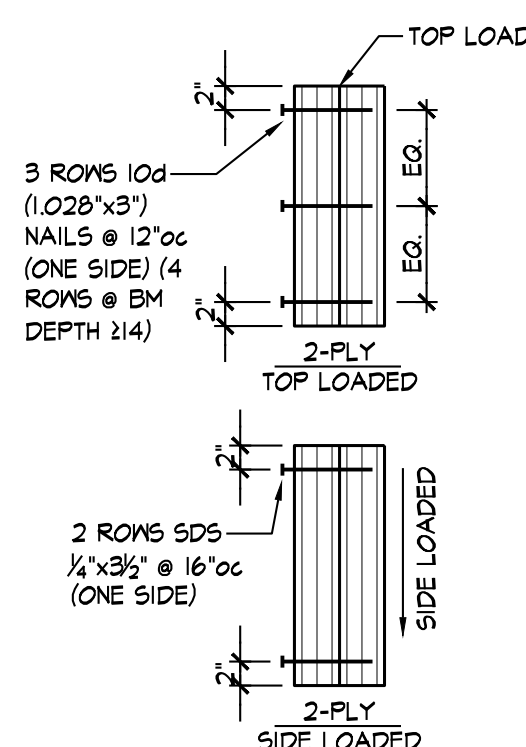


TYPICAL LEDGER  
CONNECTION 3  
 $\frac{3}{8}" = 1'-0"$  S.I.

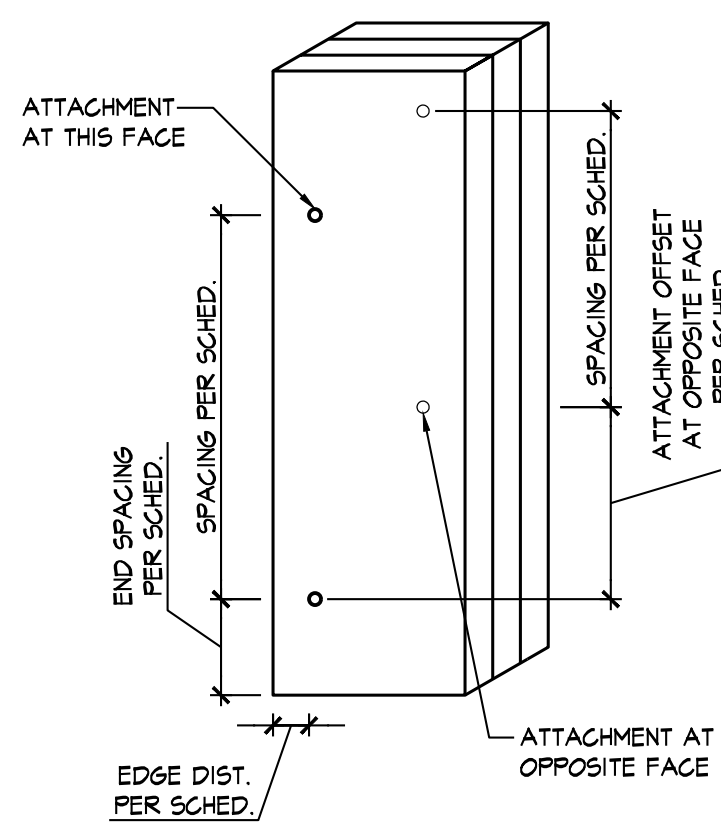


- NOTE:**
1. CONTACT ARCHITECT PRIOR TO CUTTING JOISTS TO VERIFY SIZE AND LOCATION
  2. DETAIL APPLIES TO 2x FRAMING ONLY. REFER TO ENGINEER OR COMPOSITE JOIST MANUFACTURER'S RECOMMENDATIONS AT PS 19 | VI 9 | SI 9 & GI 11 AM

SECTION 4  
 $\frac{3}{4}" = 1'-0"$  S.I.



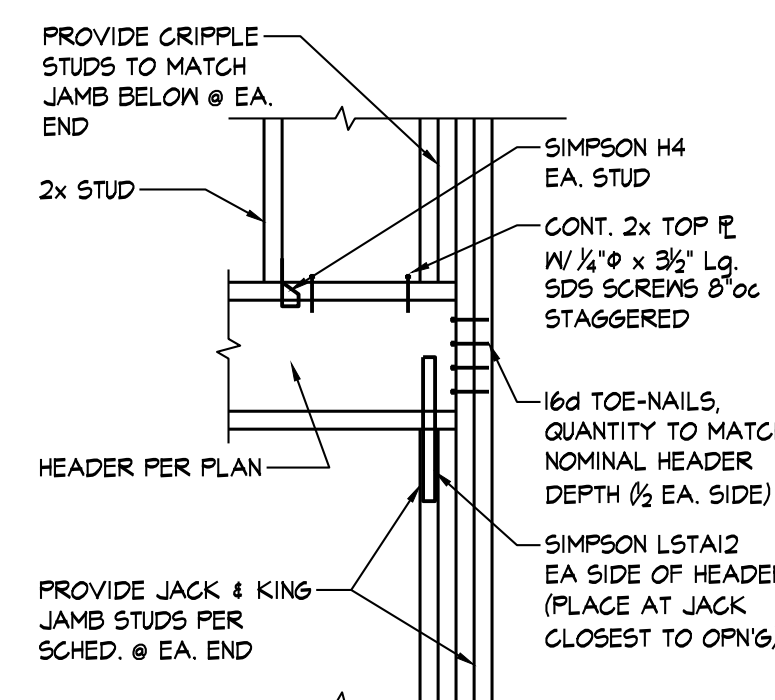
## TYPICAL MULTI-PLY BEAM CONNECTION



## TYPICAL MULTI-PLY STUD CONNECTION

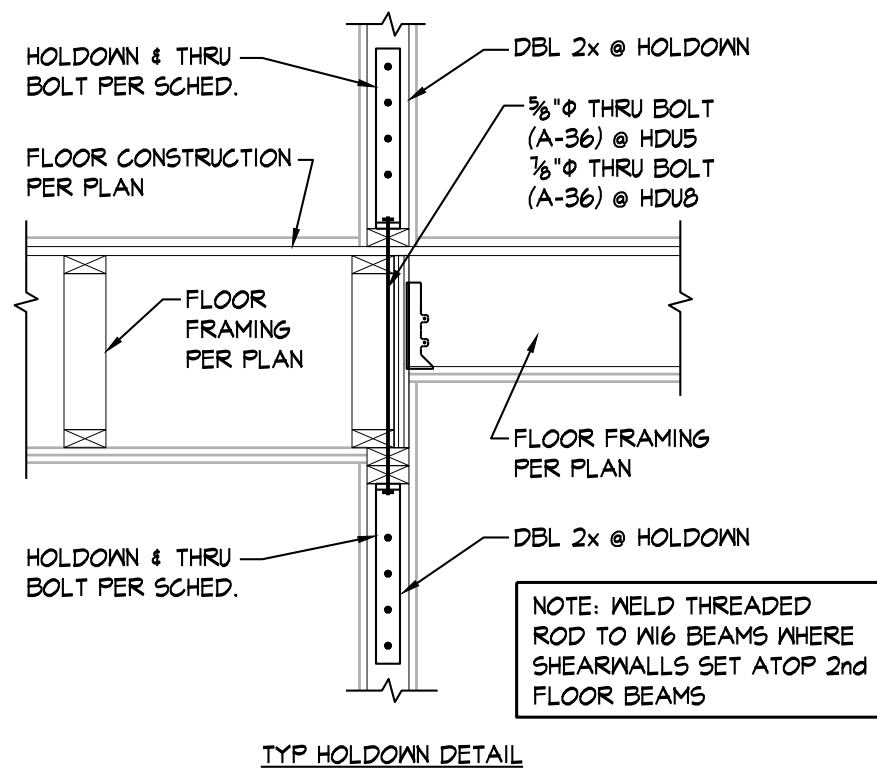
BUILT-UP STUD PACK COLUMN ATTACHMENT SCHEDULE		
NUMBER OF FLIES	ATTACHMENT AT COLUMN STUD PACKS SUPPORTING BEAMS	ATTACHMENT AT WALL STUD PACKS SUPPORTING TRUSSES
2-PLY MEMBERS	8d NAILS AT 12"oc, 1" FROM EDGE, W/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc W/ FIRST NAIL 2" FROM EA. END	8d NAILS AT 12"oc, 1" FROM EDGE, W/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc W/ FIRST NAIL 2" FROM EA. END
3-PLY MEMBERS	20d NAILS AT 16"oc, 1 1/2" FROM EDGE W/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 8", @ 16"oc W/ FIRST NAIL 3" FROM EA. END	8d NAILS AT 12"oc, 1" FROM EDGE, W/ OPPOSITE EDGE NAILED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc W/ FIRST NAIL 2" FROM EA. END
4-PLY MEMBERS	1/4"x8"3' SIMPSON SDS SCREWS AT 16"oc, 1 1/2" FROM EDGE W/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 8", @ 16"oc W/ FIRST SCREW 4" FROM EA. END	3 PLIES ATTACHED PER 3-PLY ATTACHMENT WITH 4TH PLY ATTACHED WITH 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROWS 6"
5-PLY MEMBERS	1/4"x6"3' SIMPSON SDS SCREWS AT 12"oc, 1 1/2" FROM EDGE W/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc W/ FIRST SCREW 4" FROM EA. END	3 PLIES ATTACHED PER 3-PLY ATTACHMENT WITH 4TH & 5TH PLY ATTACHED AT OPPOSITE SIDES WITH 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROWS 6"
6-PLY MEMBERS	1/4"x8"3' SIMPSON SDS SCREWS AT 12"oc, 1 1/2" FROM EDGE W/ OPPOSITE EDGE SCREWED FROM OPPOSITE SIDE OFFSET 6", @ 12"oc W/ FIRST SCREW 4" FROM EA. END	3-PLIES ATTACHED PER 3-PLY ATTACHMENT WITH 4TH & 5TH PLY ATTACHED WITH 8d NAILS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROWS 6" AND 5TH AND 6TH PLY ATTACHED WITH 1/4"x8"3' SIMPSON SDS SCREWS AT 12"oc IN 2 ROWS, 1 1/2" FROM EDGE, OFFSET ROWS 6"oc W/ FIRST SCREW 4" FROM EA. END

- NOTES:
1. ALL BUILT-UP STUD PACKS MUST ALIGN FLOOR-TO-FLOOR WITH SOLID BLOCKING (SQUASH BLOCKS) AT FLOOR CAVITIES
  2. EXTEND ALL STUD PACKS TO COLUMNS UNLESS NOTED OTHERWISE.
  3. ALL NAILS ARE COMMON NAILS UNLESS NOTED OTHERWISE.

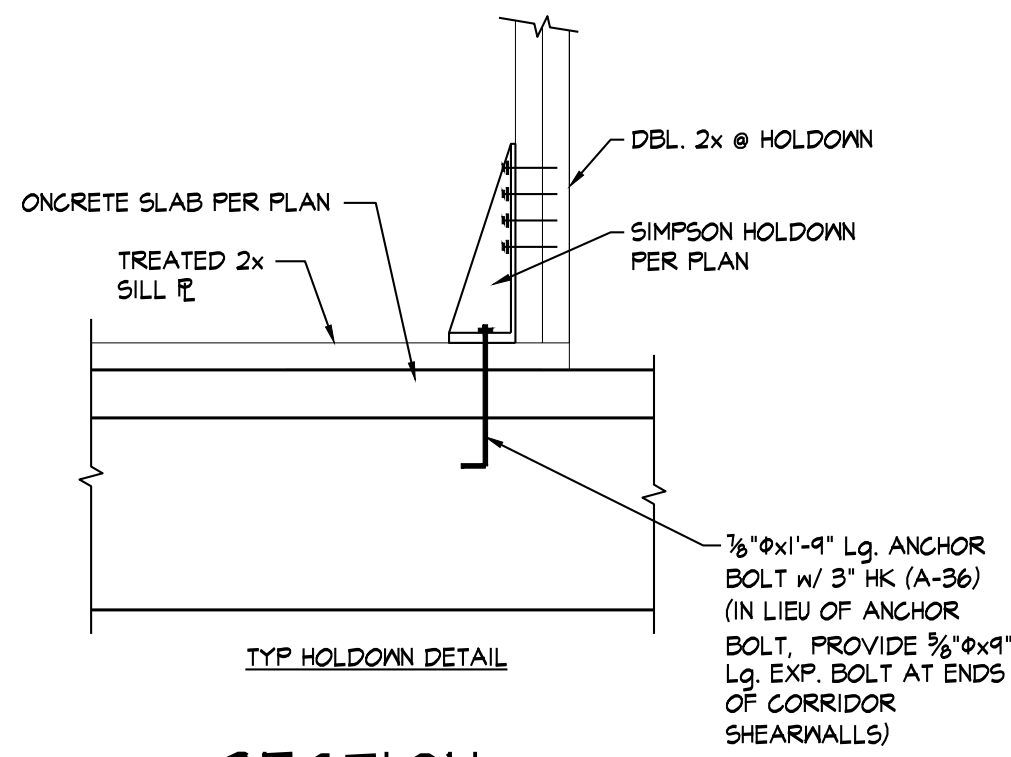


TYP. HEADER DETAIL @ ROOF TRUSS BEARING LOCATIONS

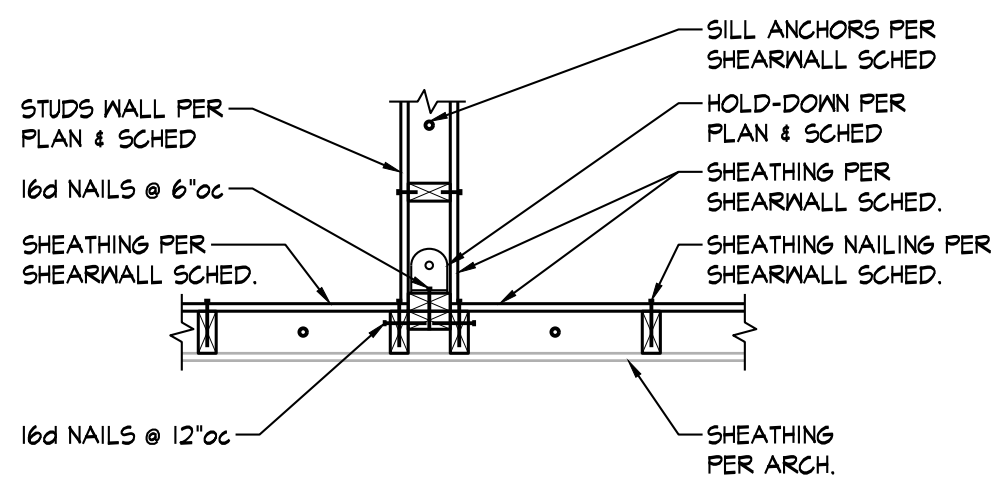
SECTION 7  
 $\frac{3}{4}" = 1'-0"$  S.I.



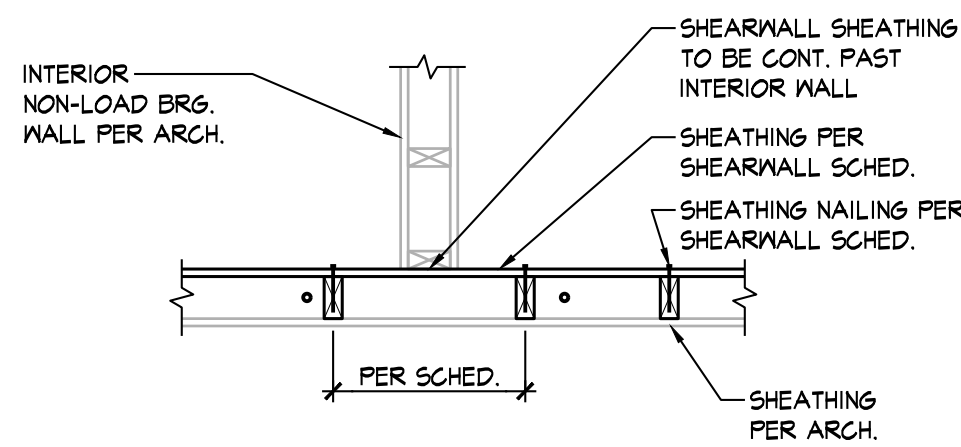
SECTION 2  
3/4" = 1'-0" S1.2



SECTION 3  
3/4" = 1'-0" S1.2



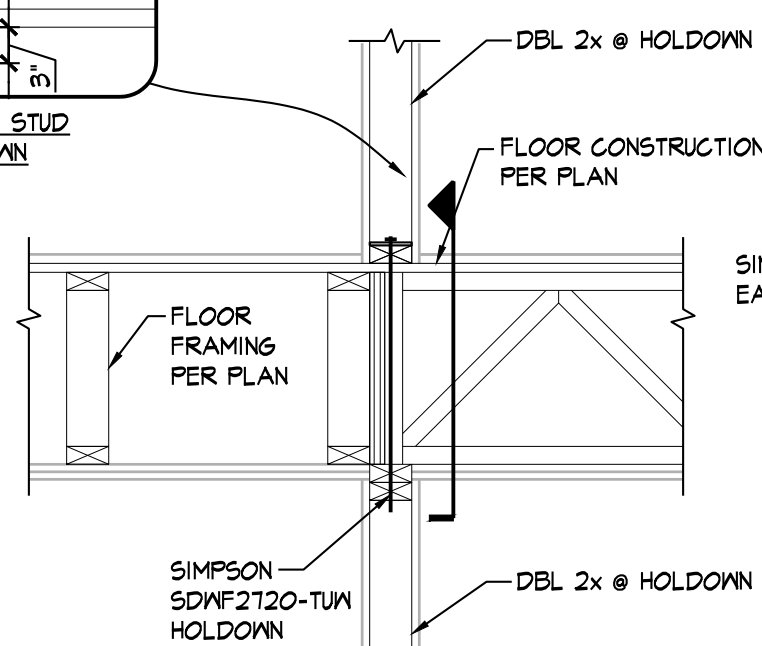
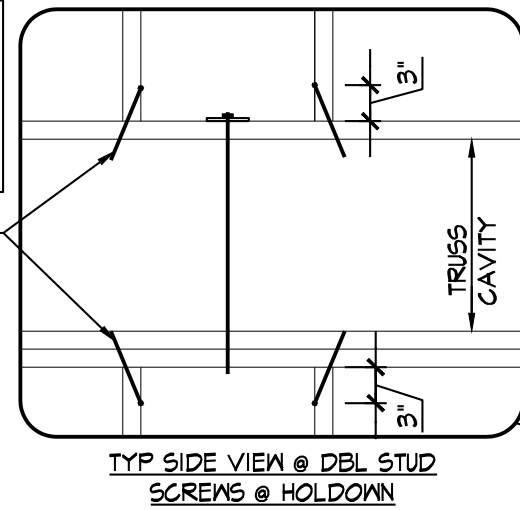
SECTION 4A  
3/4" = 1'-0" S1.2



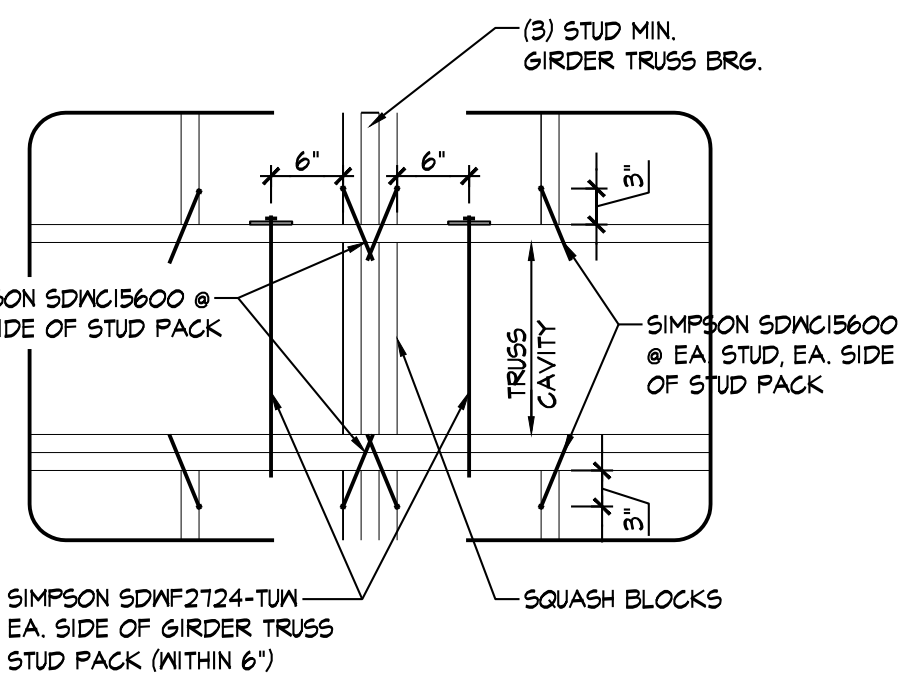
SECTION 4B  
3/4" = 1'-0" S1.2

PROVIDE UNIT UPLIFT HOLDOWNS @ 48"oc MAX @ LOAD BEARING INTERIOR WALLS SUPPORTING ROOF TRUSSES. HOLDOWNS SHALL BE PROVIDED WITHIN 6" OF JAMBS OF ALL INTERIOR LOAD BEARING ROOF HEADERS & GIRDER TRUSS BEARING AND WITHIN 48" OF SHEARWALL HOLDOWNS

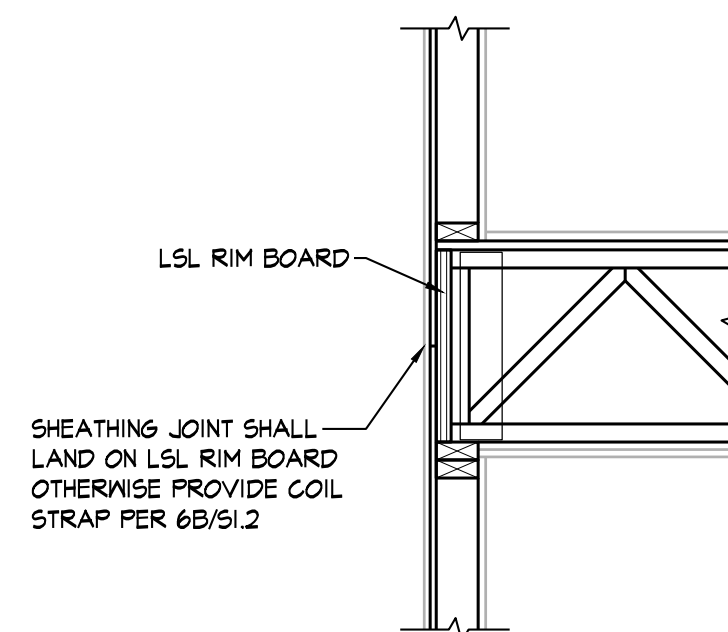
SIMPSON SDWC15600 @ EA. STUD ADJACENT TO SDWF SCREEN USE TEMPLATE FOR 22 DEG INSTALLATION



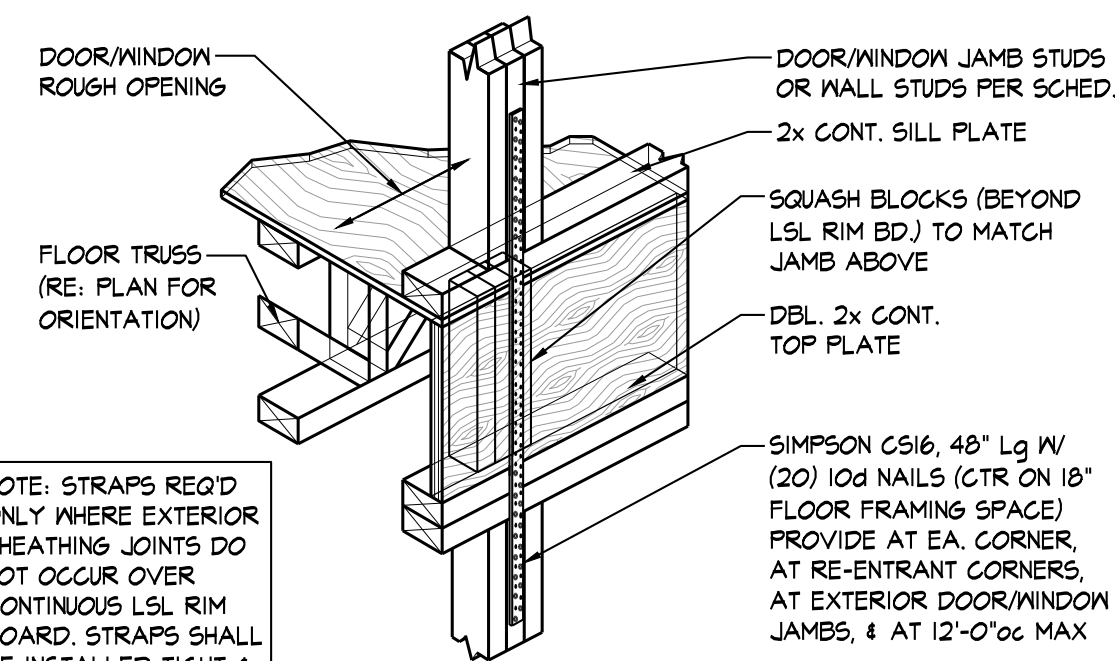
SECTION 5A  
3/4" = 1'-0" S1.2



SECTION 5B  
3/4" = 1'-0" S1.2



TYPICAL EXTERIOR SHEATHING JOINT SECTION 6A  
3/4" = 1'-0" S1.2



NOTE: STRAPS REQ'D ONLY WHERE EXTERIOR SHEATHING JOINTS DO NOT OCCUR OVER CONTINUOUS LSL RIM BOARD. STRAPS SHALL BE INSTALLED TIGHT & W/OUT SLACK.

TYPICAL COIL STRAP @ EXTERIOR JAMBS SUPPORTING ROOF FRAMING AT FLOOR DIRECTLY BELOW ROOF

DETAIL 6B  
3/4" = 1'-0" S1.2

HOLDOWN SCHEDULE			
MARK	FLOOR LEVEL (W/ APPLICABLE HOLDOWN TYPE PER FLOOR)		
	1st FLOOR	2nd FLOOR	3rd FLOOR
*	HDUB-SDS2.5	HDUB-SDS2.5	HDUS-SDS2.5
**	HDUS-SDS2.5	HDUS-SDS2.5	HDUS-SDS2.5

- NOTES:
- HOLDOWN TYPES ARE BASED UPON MANUFACTURER SIMPSON STRONG-TIE.
  - REFER TO SECTION DETAILS ON S1.2 FOR TYPICAL HOLDOWN DETAILS.
  - WHERE THE ENDS OF PERPENDICULAR SHEAR WALLS INTERSECT AND ONLY ONE HOLDOWN SHOWN ON PLAN, FASTEN ALL STUDS TOGETHER PER SCHEDULE AND USE LARGER OF THE TWO HOLDOWNS SHOWN ON THE SHEAR WALL SCHEDULE.
  - ALL HOLDOWN POSTS TO BE (2) 2x's (MIN) (UNO.) TO MATCH STUD SIZE & GRADE NOTED IN WALL SCHEDULE. PROVIDE ADDITIONAL STUDS AS REQ'D TO MEET QUANTITY NOTED IN SCHED.
  - REFER TO SECTIONS 2/S1.2, 3/S1.2, 4A/S1.2 & 4B/S1.2 FOR HOLDOWN ANCHOR REQUIREMENTS.

SHEARWALL SCHEDULE					
SHEARWALL LOCATION	SHEARWALL TYPE		FLOOR		NUMBER OF WALL STUDS AT HOLD-DOWN (RE: NOTE 4)
			1st FLOOR WALLS	2nd & 3rd FLOOR WALLS	
AT DEMISING WALLS	SM	MATERIAL & THICKNESS	7/8" OSB SHEATHING ONE SIDE, W/ EDGES BLOCKED	7/8" OSB SHEATHING ONE SIDE, W/ EDGES BLOCKED	
		NAIL SIZE & SPACING	8d NAILS 4/12	8d NAILS 6/12	
AT CORRIDOR WALLS	SM	MATERIAL & THICKNESS	7/8" OSB SHEATHING ONE SIDE, W/ EDGES BLOCKED	7/8" OSB SHEATHING ONE SIDE, W/ EDGES BLOCKED	
		NAIL SIZE & SPACING	8d NAILS 4/12	8d NAILS 6/12	

- NOTES:
- NAILING SHALL BE TO ALL STUDS, TOP & BOTTOM PLATES, AND BLOCKING WHERE INDICATED.
  - HOLDOWNS PER PLAN & SCHEDULE.
  - WHERE THE ENDS OF PERPENDICULAR SHEAR WALLS INTERSECT AND ONLY ONE HOLDDOWN SHOWN ON PLAN, FASTEN ALL STUDS TOGETHER PER SCHEDULE AND USE LARGE OF THE TWO HOLDOWNS SHOWN IN THE SHEARWALL SCHEDULE. REFERENCE DETAILS 4A, 4B, 4C, AND 4D ON SHEET S1.2 FOR SHEATHING AND HOLDOWN ATTACHMENT AT PERPENDICULAR WALLS AND STUD WALL SIZE TRANSITIONS.
  - PROVIDE 2 WALL STUDS AT EACH HOLDDOWN UNLESS NOTED OTHERWISE IN SCHEDULE. AT LOCATIONS WHERE A SHEARWALL TERMINATES AT A OPENING JAMB, PROVIDE NUMBER OF STUDS PER JAMB SCHEDULE PLUS AN ADDITIONAL STUD FOR THE SHEARWALL. ATTACH ALL STUDS TOGETHER PER 6/S1.
  - NAIL SPACING SHOWN AS (N/I) INDICATES FASTENERS SPACING IN INCHES AT THE EDGES/FIELD WHERE FIELD IS THE INTERMEDIATE MEMBERS.
  - TYPICAL SILL PLATE TO WOOD SHALL BE 20d COMMON NAILS (1.0x2x4") AT 12"oc UNLESS NOTED OTHERWISE IN SCHEDULE.
  - TYPICAL SILL PLATE TO CONCRETE SHALL BE 1/2"Ø ANCHORS:  
AT 2x4 WALLS SPACE AT 24"oc MAX WITH 1/2"x2 1/2"x2 1/2" PLATE WASHER OR SIMPSON BPS 1/2" - 3 @ CONTRACTORS OPTION  
PLATE WASHERS TO MAINTAIN MAX OF 1/2" BETWEEN EDGE OF SILL PLATE AND EDGE OF PLATE WASHER
  - SHEARWALL SHEATHING CALLED OUT AT CORRIDOR WALLS SHALL BE LOCATED AT UNIT SIDE OF WALL.
  - AT GYPSUM SHEARWALLS NO. 6 x 1 1/2" TYPES S OR W SCREENS CAN BE UTILIZED AS THE SAME SPACING AS SPECIFIED 6d NAILS.
  - NAILS @ WOOD STRUCTURE PANEL SHEAR WALLS SHALL BE GALVANIZED COMMON OF TYPE INDICATED IN SCHED.



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JOB:	24-3385
SHEET NO.:	



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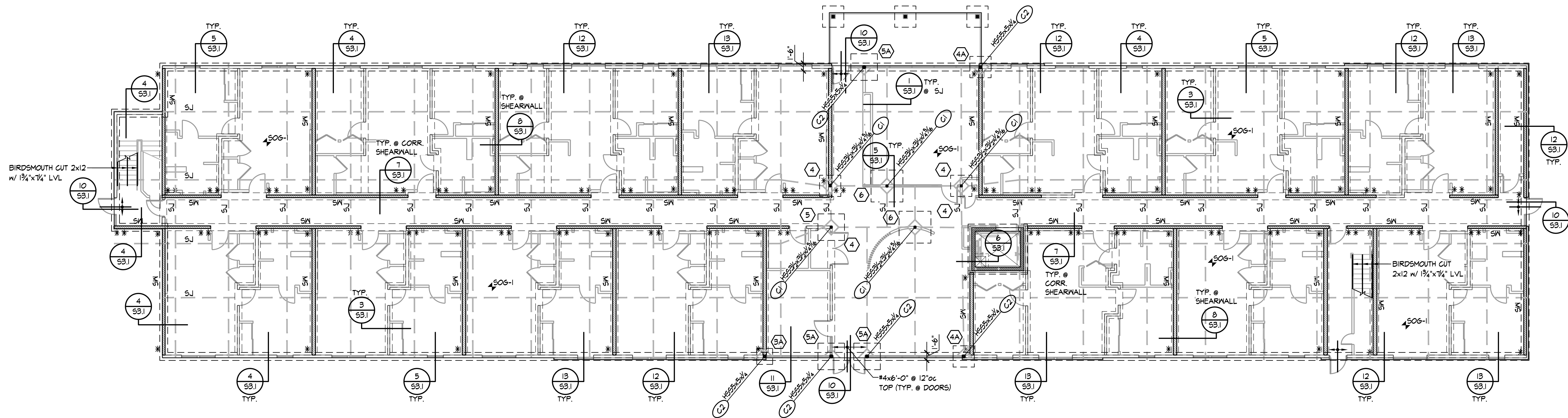
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S2.1

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FOUNDATION PLAN

3/32" = 1'-0"

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S1.0
2. REFER TO COLUMN & FOOTING SCHEDULE ON SHEET S1.1
3. T/G EL. 100'-0"
4. T/FTG. EL. 99'-2" U.N.O.
5. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN
6. REFER TO SHEET S2.6 FOR SHEARWALL AND HOLDOWN INFORMATION
7. REFER TO SHEETS S1.1 AND S1.2 FOR TYPICAL NAILING WOOD FRAMING DETAILS



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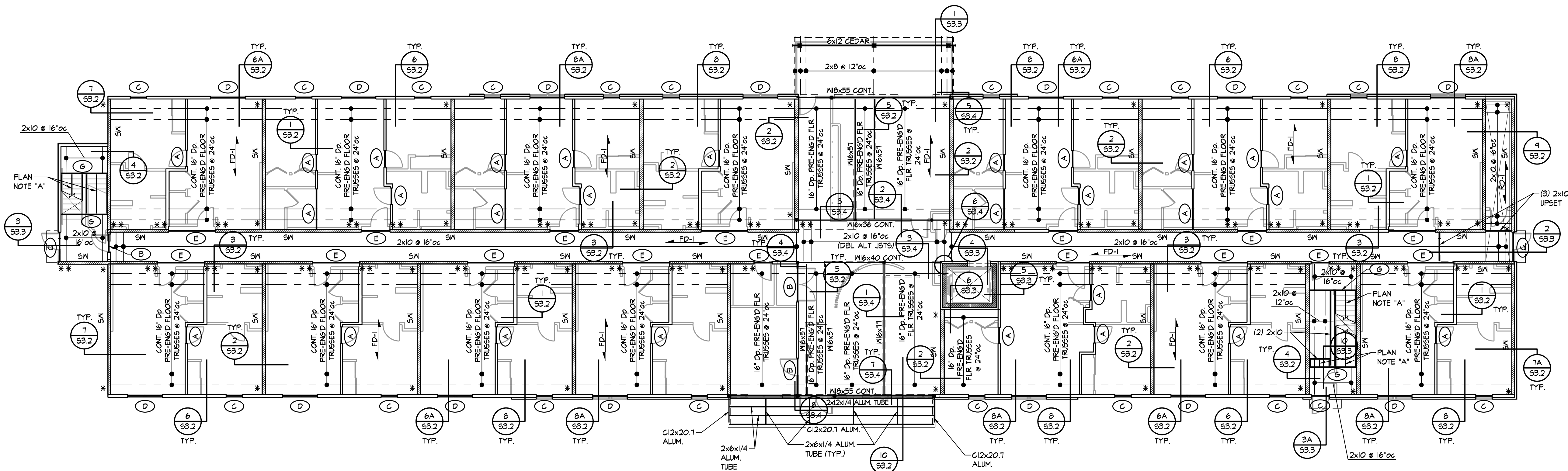
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S2.2

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## SECOND FLOOR FRAMING PLAN

3/32" = 1'-0"

### NOTES:

1. REFER TO GENERAL NOTES ON SHEET S1.0
2. REFER TO HEADER SCHEDULE ON SHEET S1.1
3. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN
4. REFER TO SHEET S2.6 FOR SHEARWALL AND HOLDOWN INFORMATION
5. REFER TO SECTIONS SA, SB, 6A AND 6B ON SHEET S1.2 FOR HOLDOWN DETAILS AT THE SECOND FLOOR
6. REFER TO SHEETS S1.1 AND S1.2 FOR TYPICAL NAILING WOOD FRAMING DETAILS
7. PLAN NOTE "A" DENOTES BIRDSMOUTH CUT 2x12 W/ 1/4"x1/4" LVL @ EACH STAIR STRINGER



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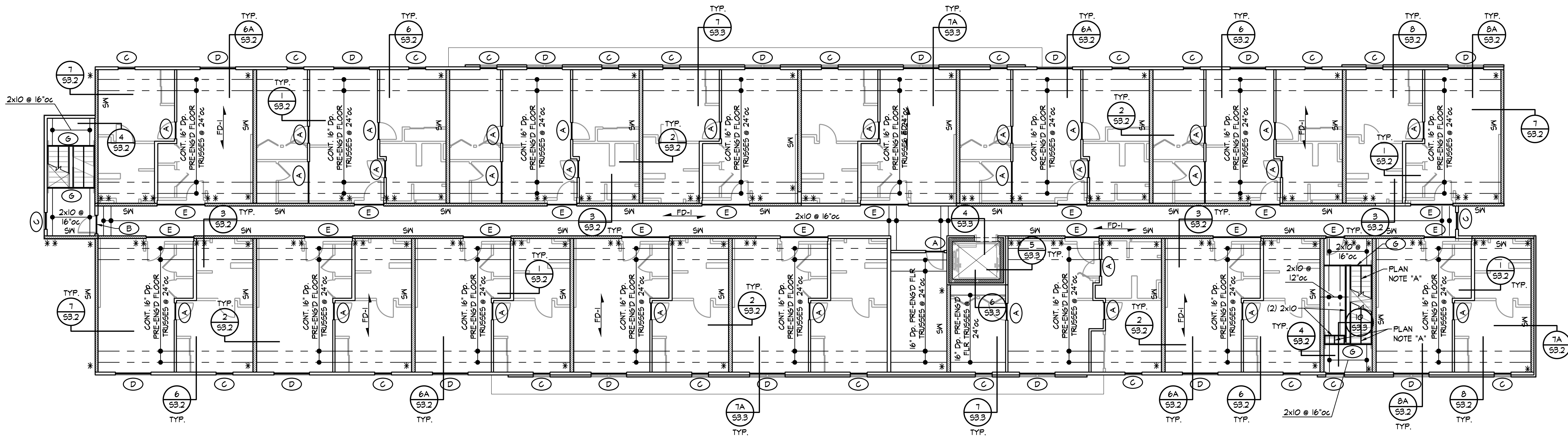
DATE: 1-03-2025

JOB: 24-3385

SHEET NO.:

S2.3

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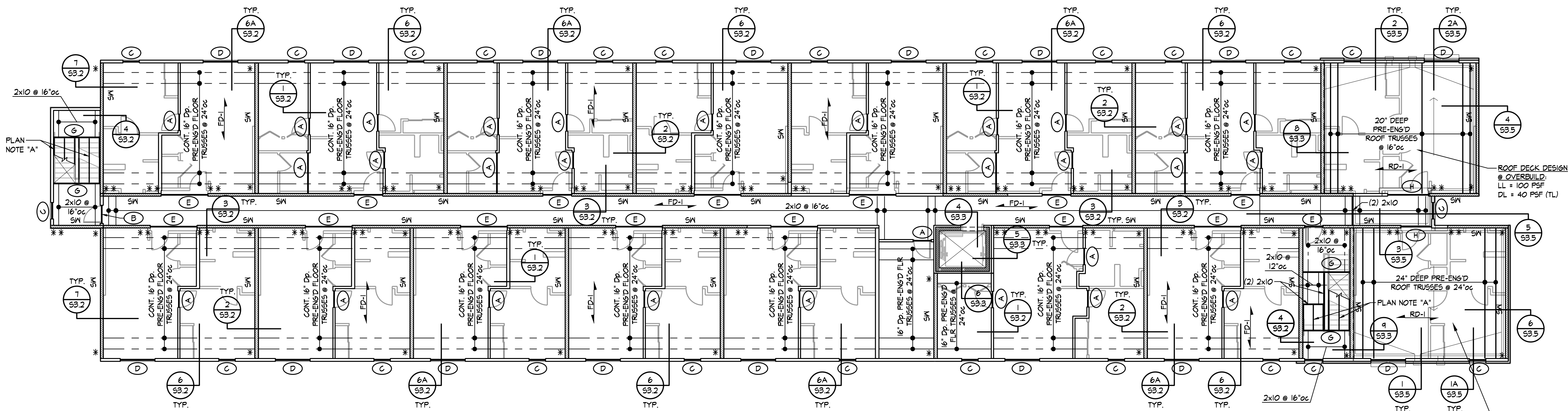


THIRD FLOOR FRAMING PLAN

3/32" = 1'-0"

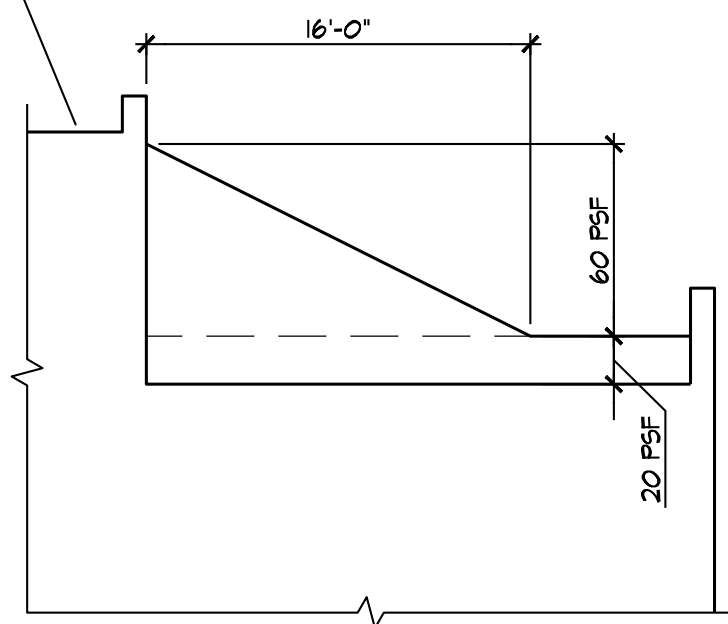
NOTES:

1. REFER TO GENERAL NOTES ON SHEET S1.0
2. REFER TO HEADER SCHEDULE ON SHEET S1.1
3. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN
4. REFER TO SHEET S2.6 FOR SHEARWALL AND HOLDOWN INFORMATION
5. REFER TO SECTIONS 5A, 5B, 6A AND 6B ON SHEET S1.2 FOR HOLDOWN DETAILS AT THE SECOND FLOOR
6. REFER TO SHEETS S1.1 AND S1.2 FOR TYPICAL NAILING WOOD FRAMING DETAILS
7. PLAN NOTE "A" DENOTES BIRDSMOUTH CUT 2X12 W/ 1 3/4" X 1/4" LVL @ EACH STAIR STRINGER



FOURTH FLOOR & LOW ROOF FRAMING PLAN  
3/32" = 1'-0"

- NOTES:
1. REFER TO GENERAL NOTES ON SHEET S1.0
  2. REFER TO HEADER SCHEDULE ON SHEET S1.1
  3. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN
  4. REFER TO SHEET S2.6 FOR SHEARWALL AND HOLDOWN INFORMATION
  5. REFER TO SECTIONS 5A, 5B, 6A AND 6B ON SHEET S1.2 FOR HOLDOWN DETAILS AT THE SECOND FLOOR
  6. REFER TO SHEETS S1.1 AND S1.2 FOR TYPICAL NAILING WOOD FRAMING DETAILS
  7. PLAN NOTE "A" DENOTES BIRDSMOUTH CUT 2x12 w/ 1 3/4"x1 1/2" LVL @ EACH STAIR STRINGER
  8. ALL EXTERIOR DECK FRAMING TO BE TREATED LUMBER AT EXTERIOR ROOF DECK



DESIGN SNOW DRIFT AT LOW ROOF



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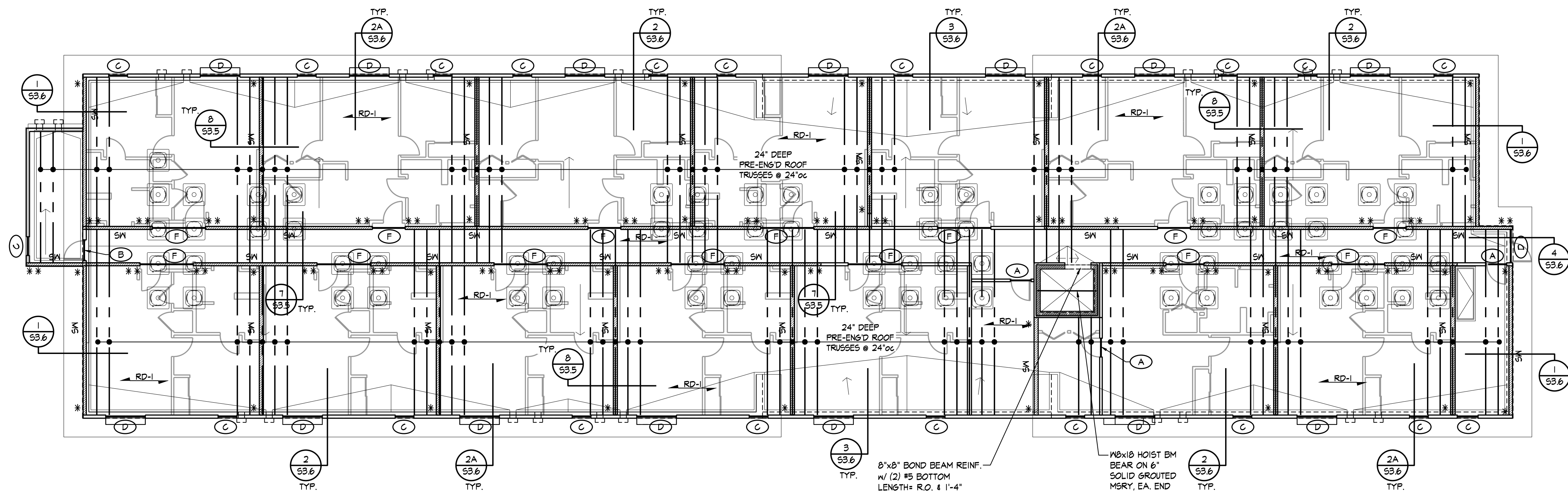
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SHEET NO.:

S2.5

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ROOF FRAMING PLAN

3/32" = 1'-0"

NOTES:

1. REFER TO GENERAL NOTES ON SHEET S1.0
2. REFER TO HEADER SCHEDULE ON SHEET S1.1
3. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN
4. PROVIDE TRIPLE 2x6 OR QUADRUPLE 2x4 STUDS AT ALL PRE-ENG. TRUSS GIRDERS (EXTEND TO FOUNDATION)
5. REFER TO SHEETS S1.1 AND S1.2 FOR TYPICAL NAILING WOOD FRAMING DETAILS

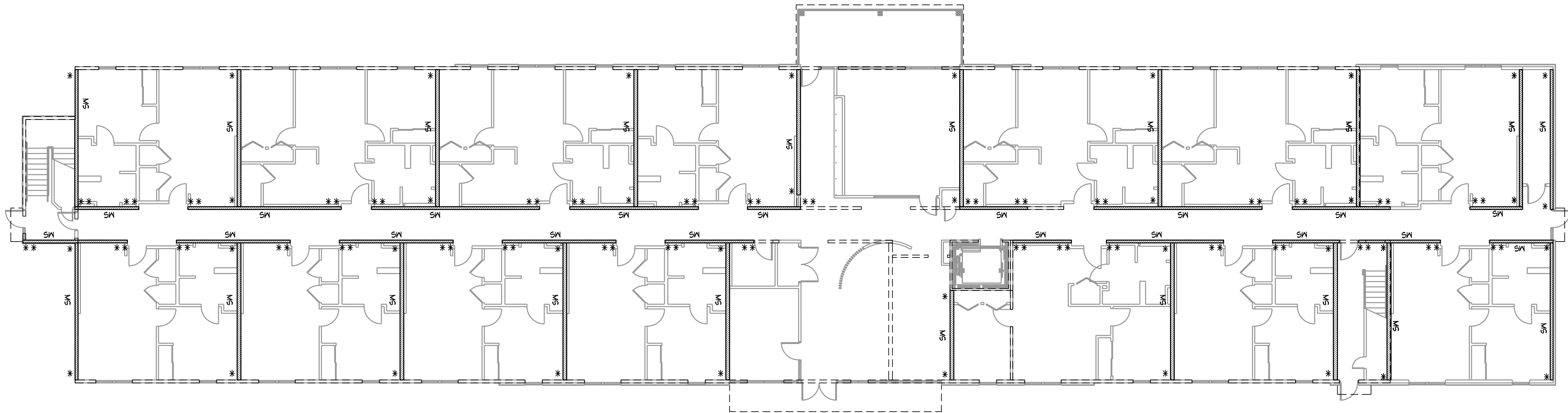


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S2.6

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SHEARWALL PLAN

3/32" = 1'-0"

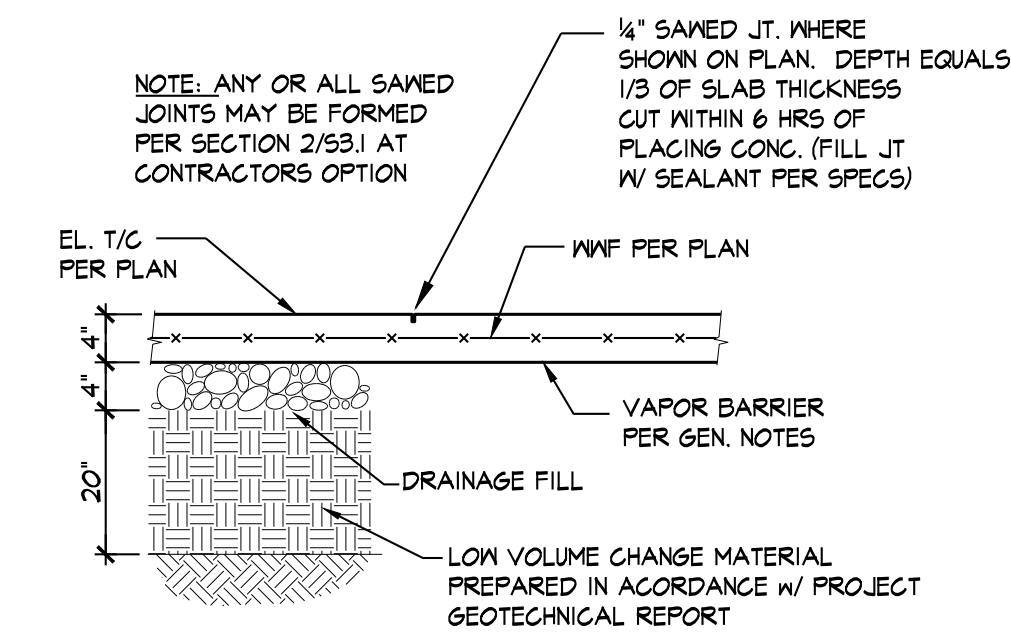
NOTES:

1. REFER TO GENERAL NOTES ON SHEET S1.0
2. REFER TO SHEARWALL & HOLDOWN SCHEDULES ON SHEET S1.2
3. SHEARWALLS/HOLDOWNS DESIGNATED AS FOLLOWS:

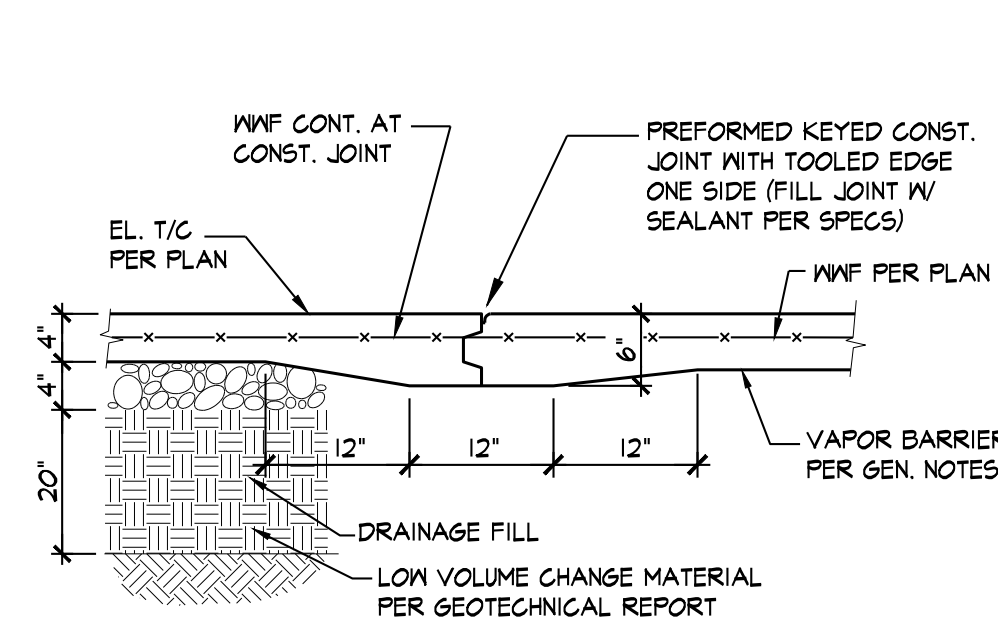


SHEAR WALL EXTENTS INDICATED W/ HATCHED AREA  
HOLDOWN TYPE MARK: (1) HOLDOWN TYPICAL EACH  
END OF SHEARWALL PER HOLDOWN ANCHOR SCHED.

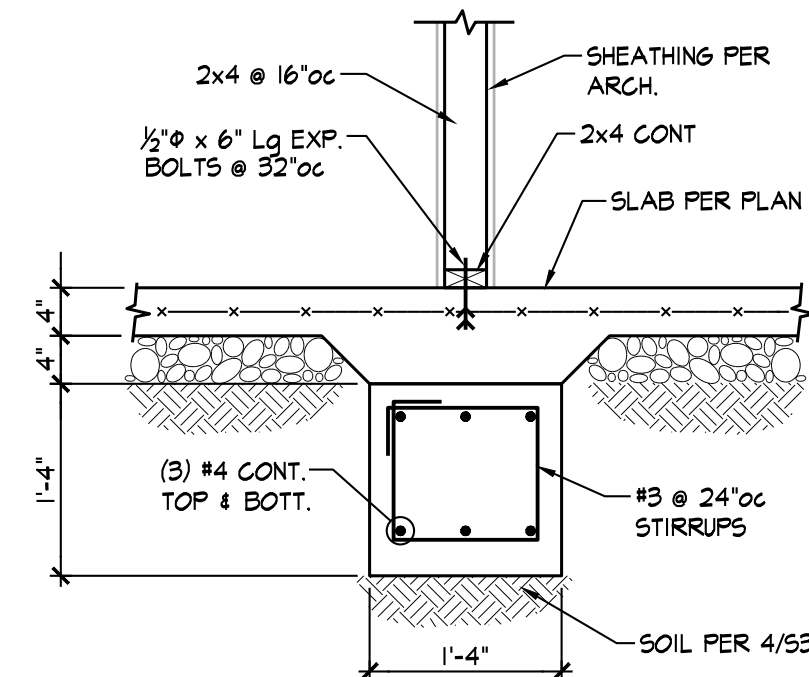
4. REFER TO DETAILS 2/S1.2 & 3/S1.2 FOR HOLDOWNS AT END OF WALL



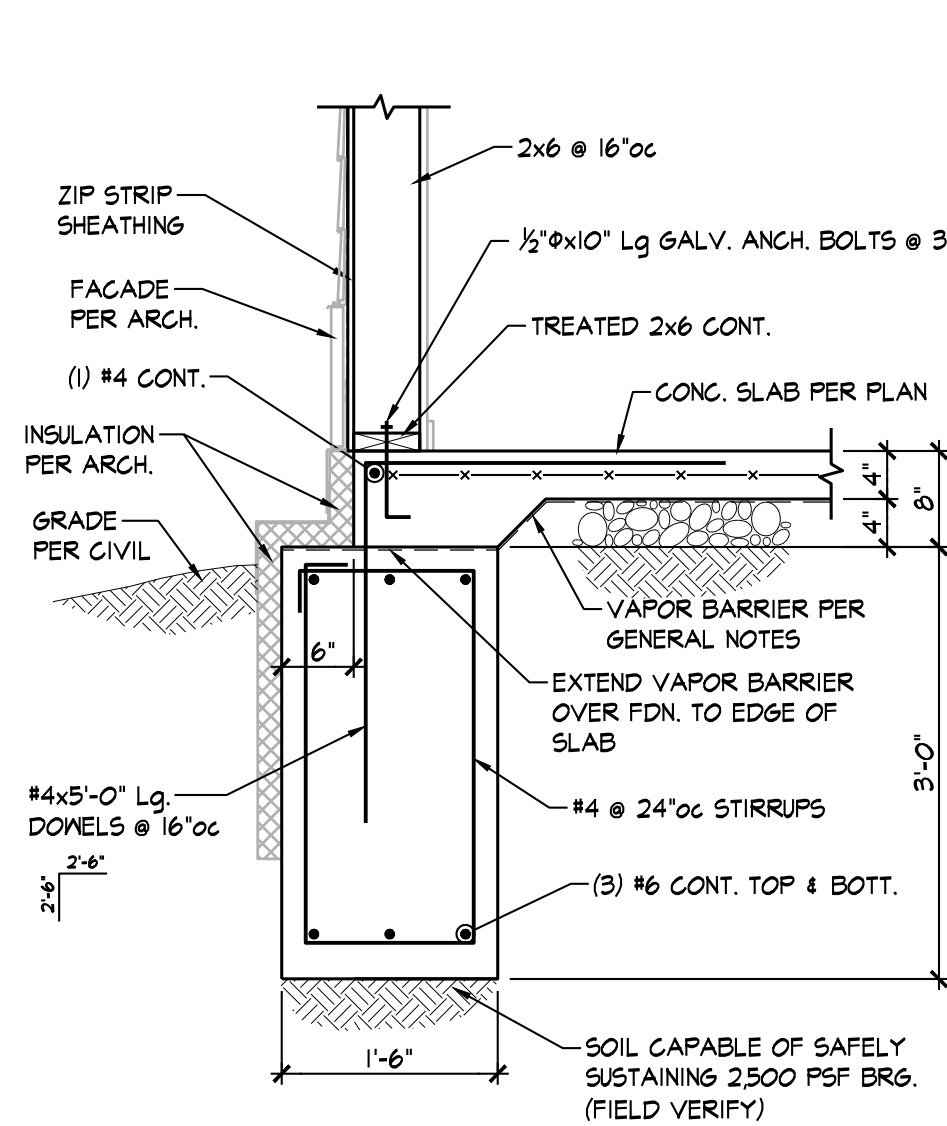
SECTION 1  
3/4\" = 1'-0\" S3.1



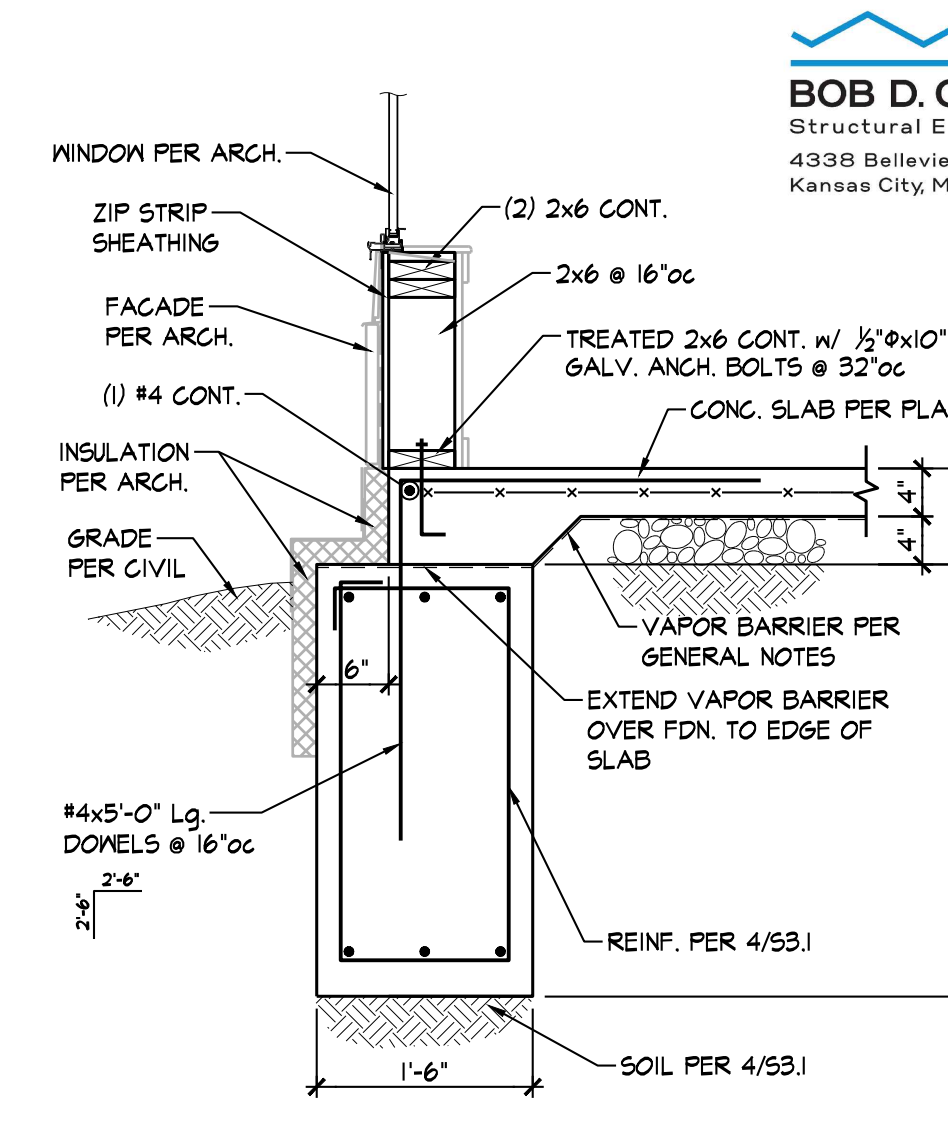
SECTION 2  
3/4\" = 1'-0\" S3.1



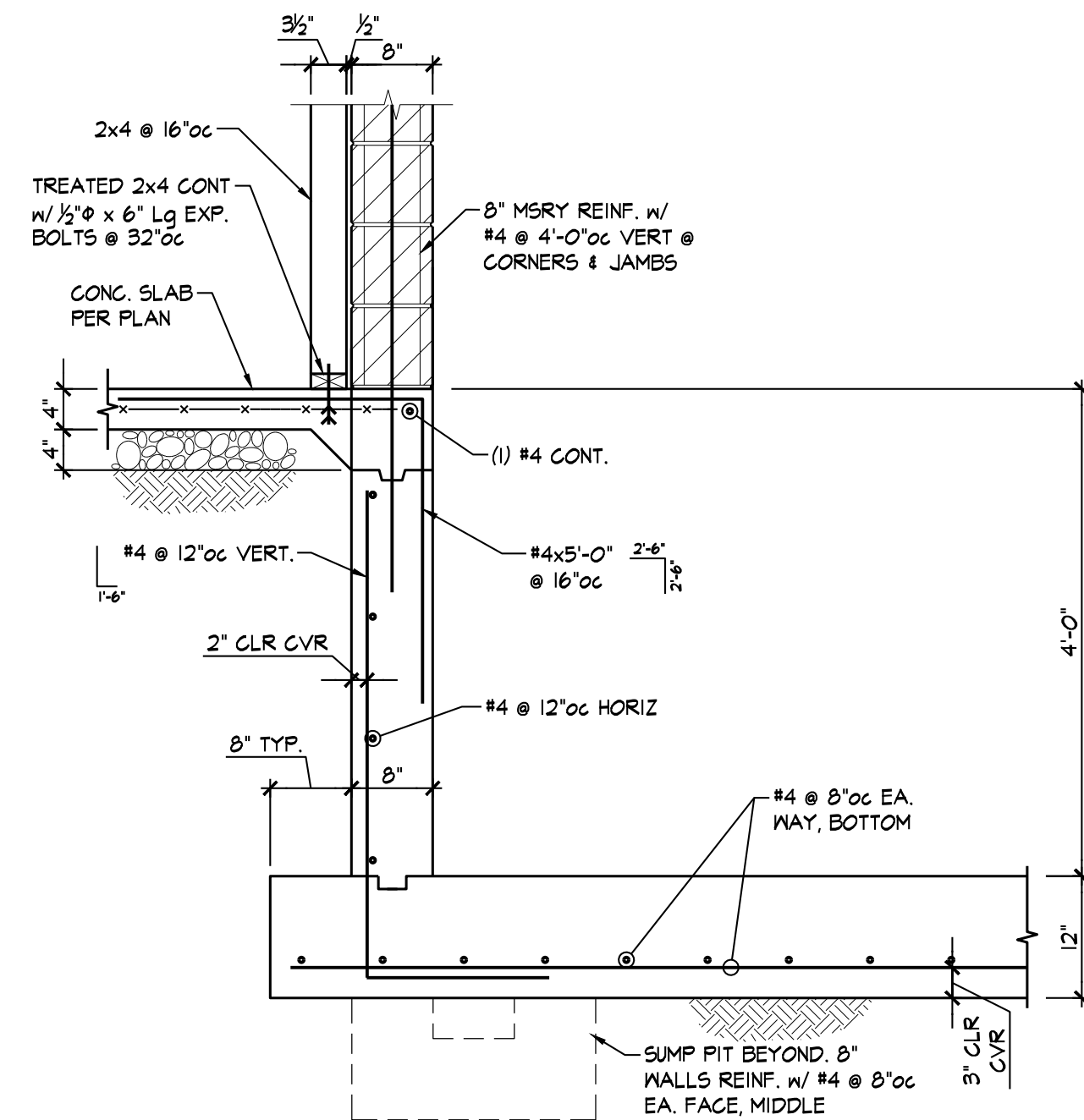
SECTION 3  
3/4\" = 1'-0\" S3.1



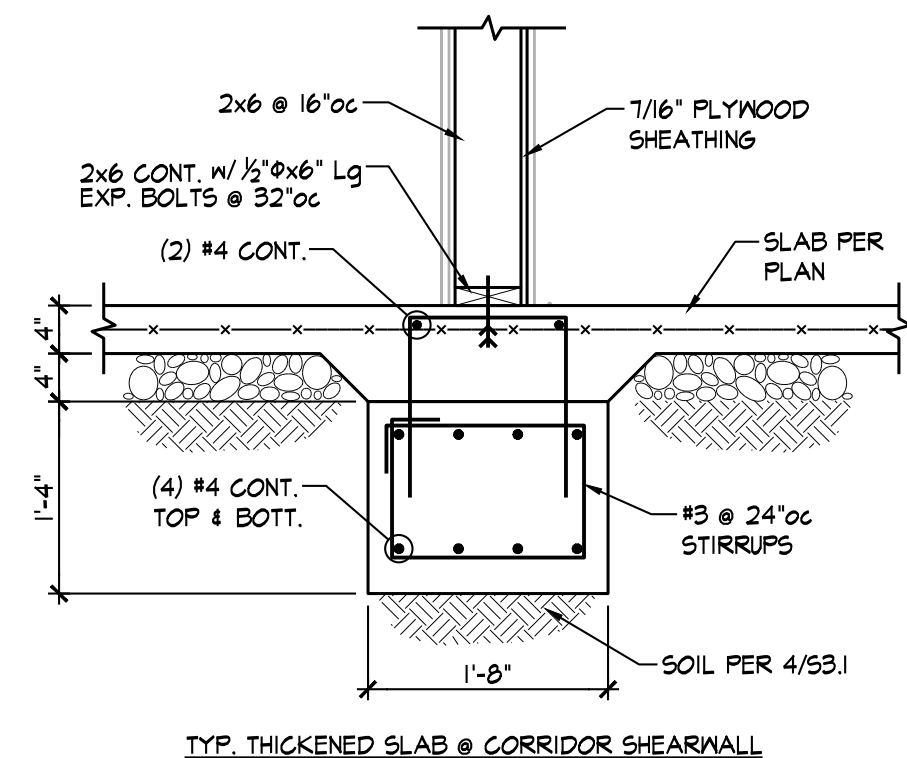
SECTION 4  
3/4\" = 1'-0\" S3.1



SECTION 5  
3/4\" = 1'-0\" S3.1

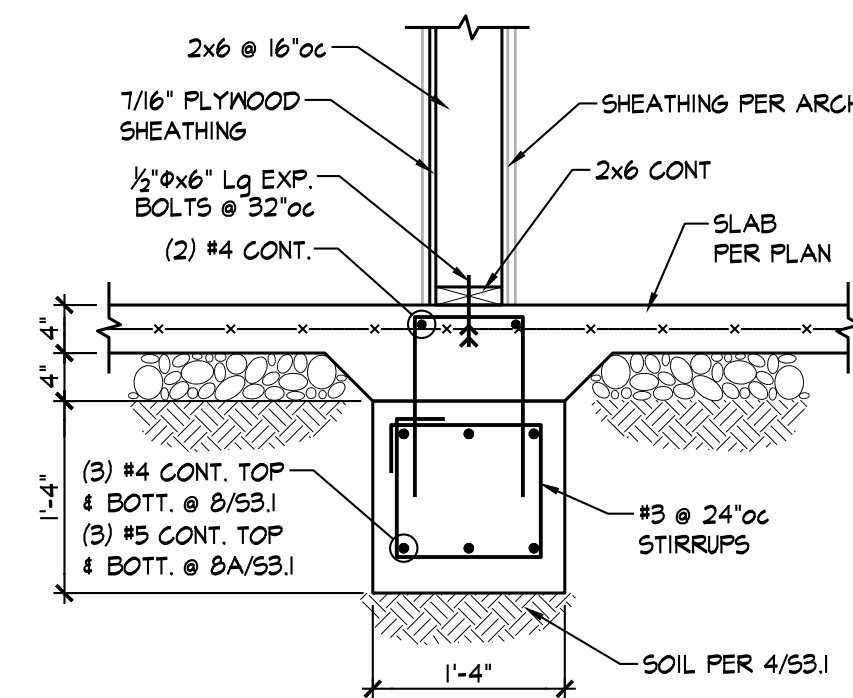


SECTION 6  
3/4\" = 1'-0\" S3.1

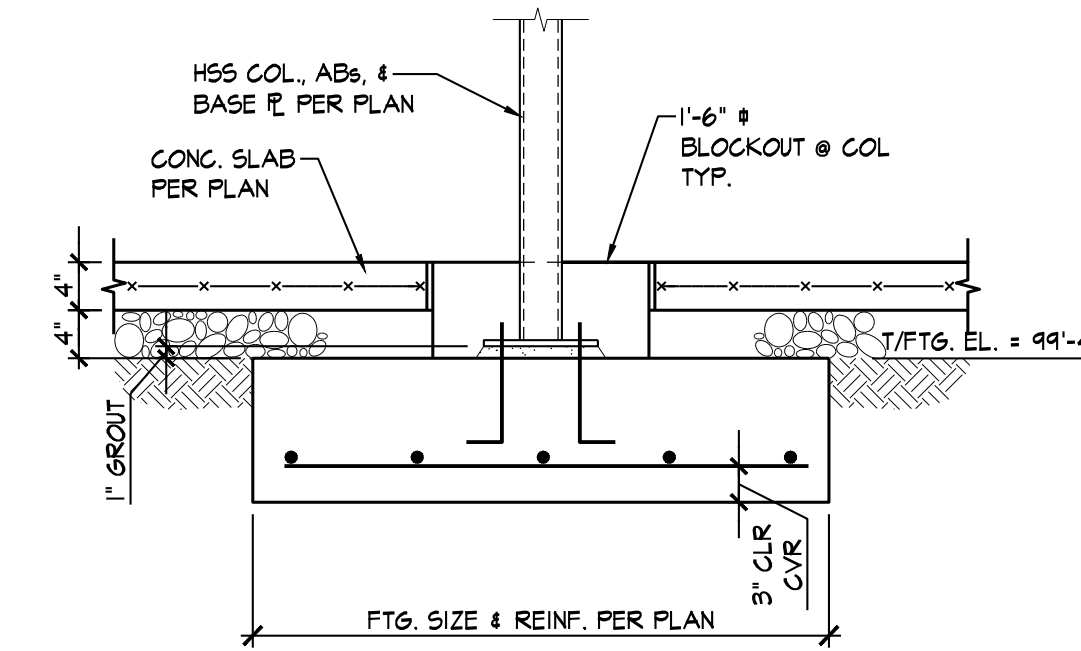


SECTION 7  
3/4\" = 1'-0\" S3.1

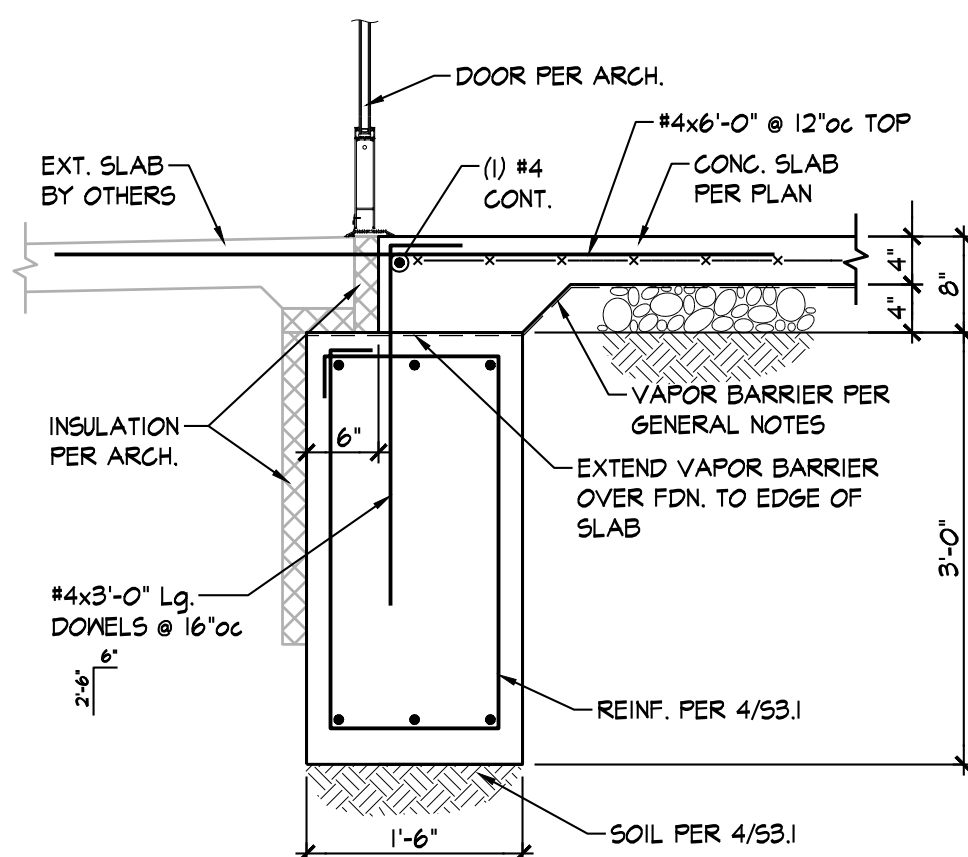
NOTE: @ ENDS OF SHEARWALLS, PROVIDE SIMPSON HD5-SD52.5 HOLDDOWN ANCHOR W/ DBL STUD & 3/8\"/>



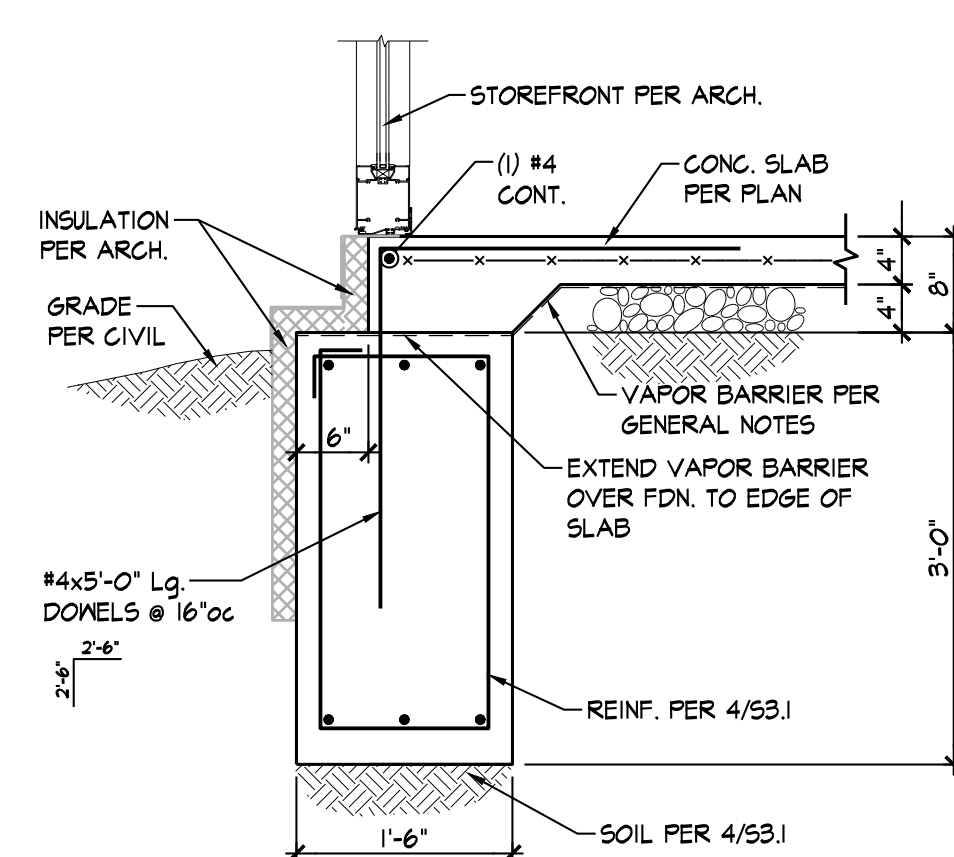
SECTION 8  
3/4\" = 1'-0\" S3.1



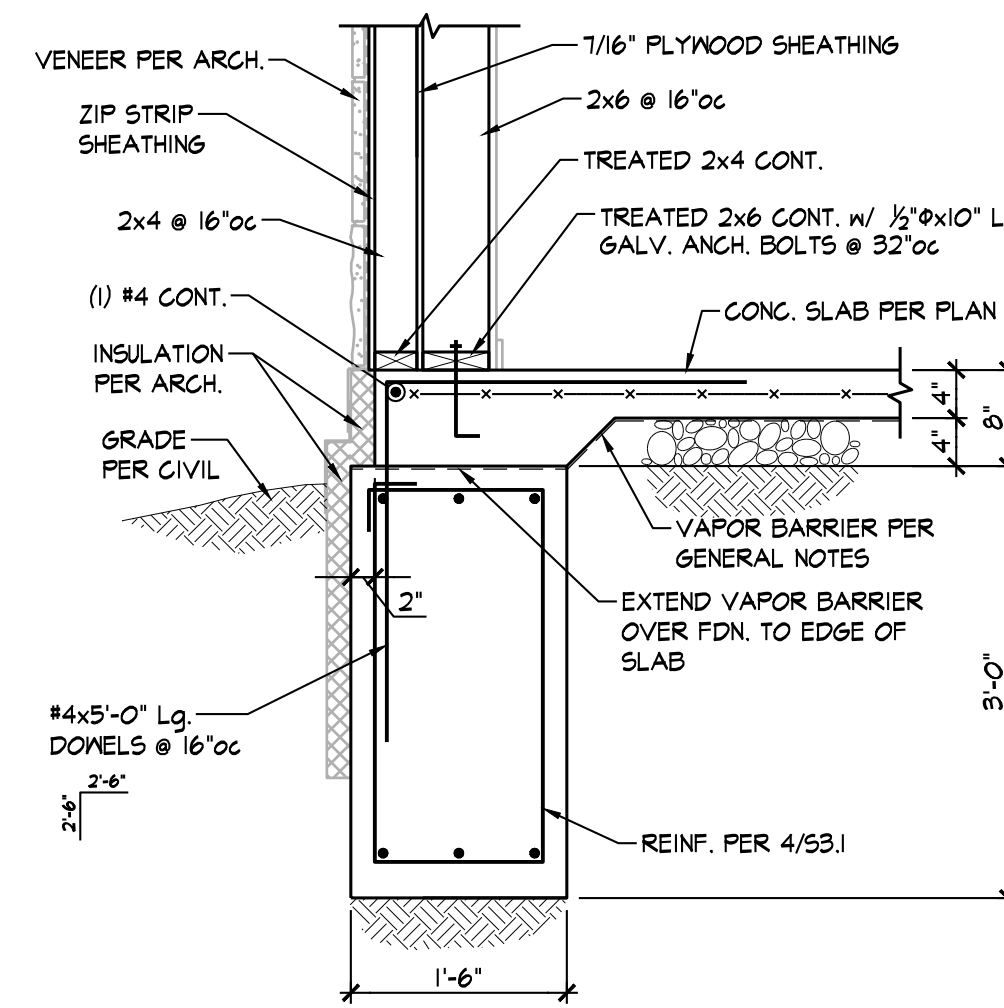
SECTION 9  
3/4\" = 1'-0\" S3.1



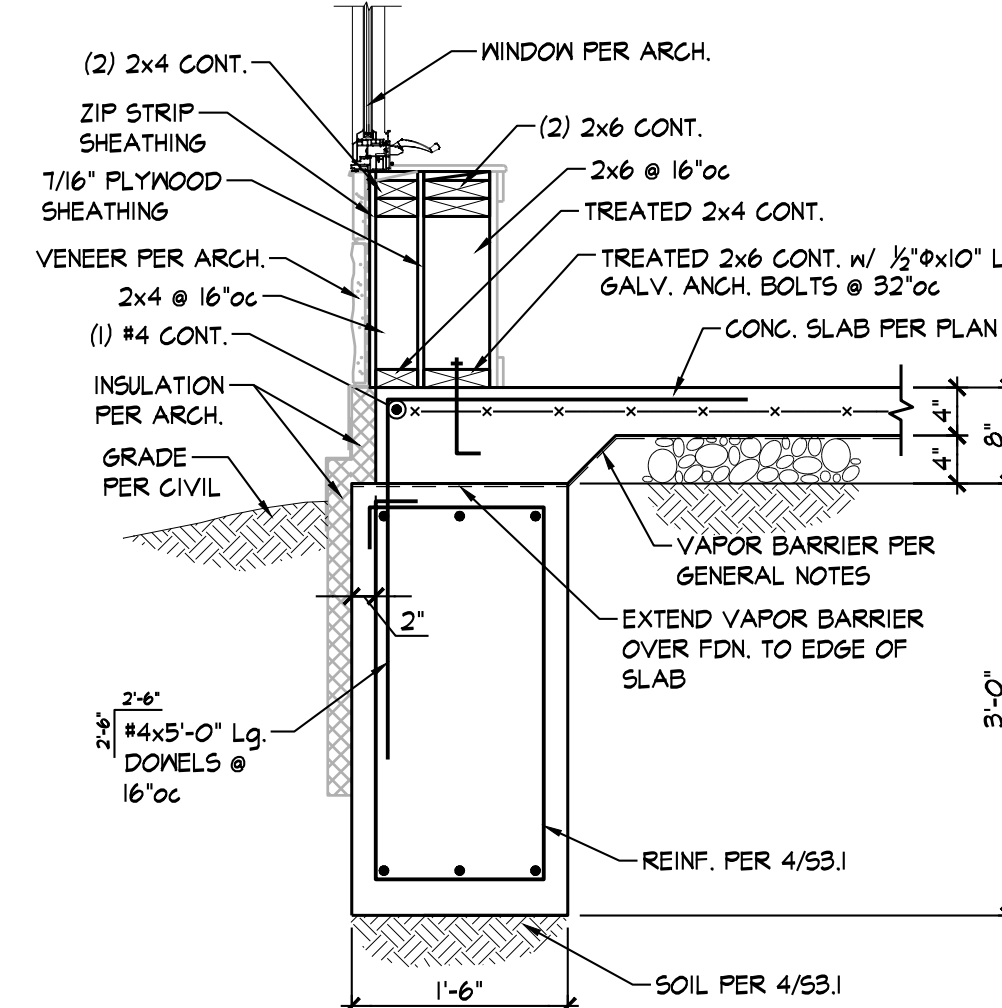
SECTION 10  
3/4\" = 1'-0\" S3.1



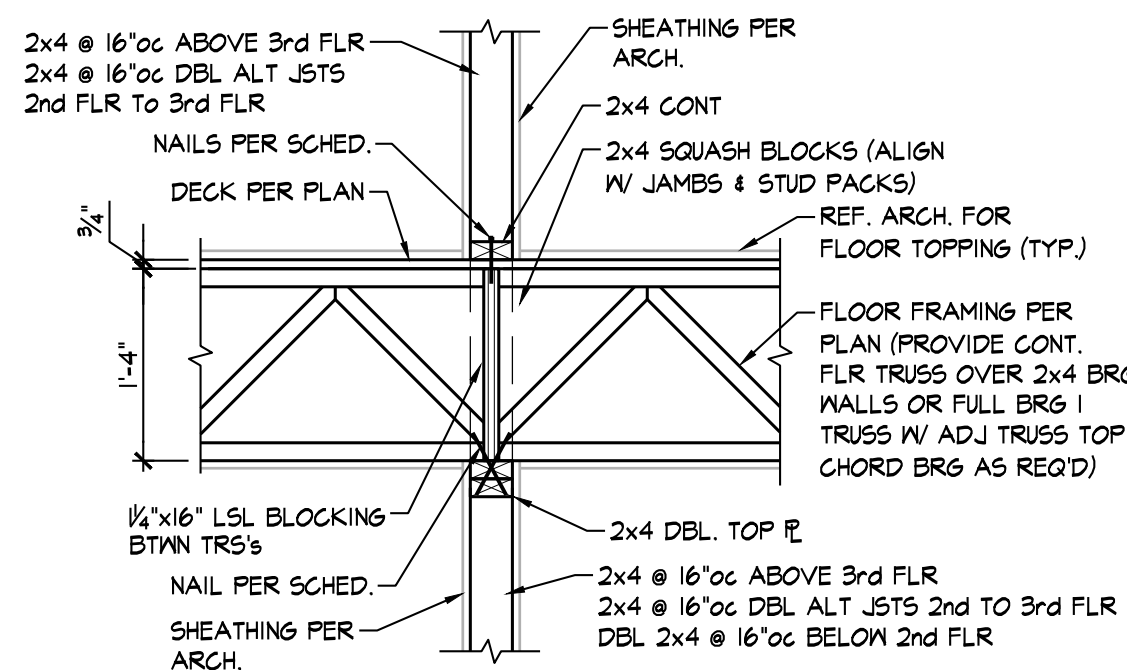
SECTION 11  
3/4\" = 1'-0\" S3.1



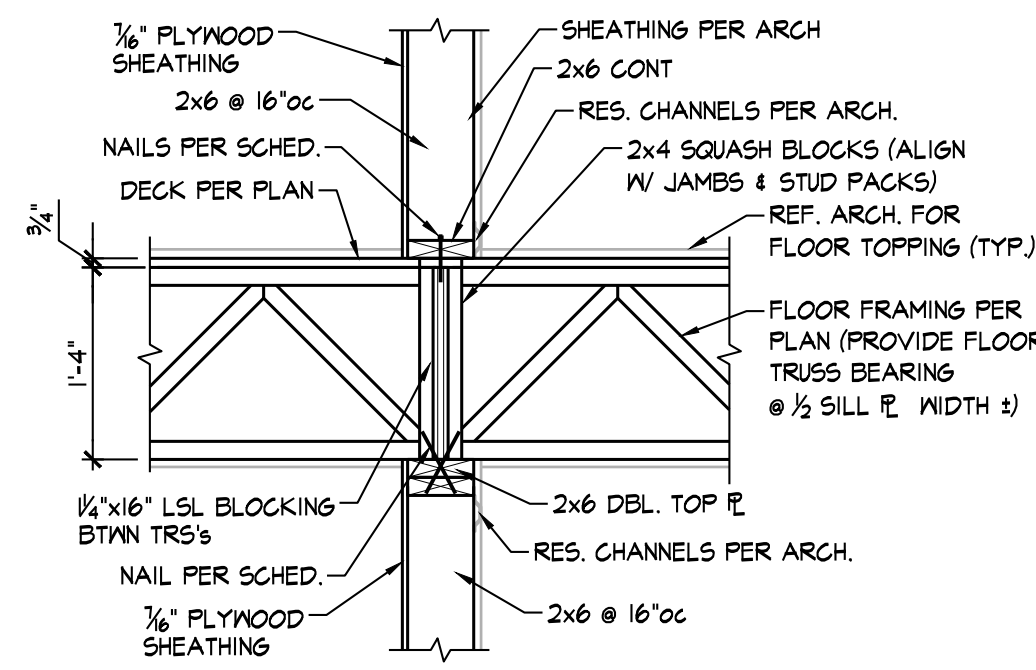
SECTION 12  
3/4\" = 1'-0\" S3.1



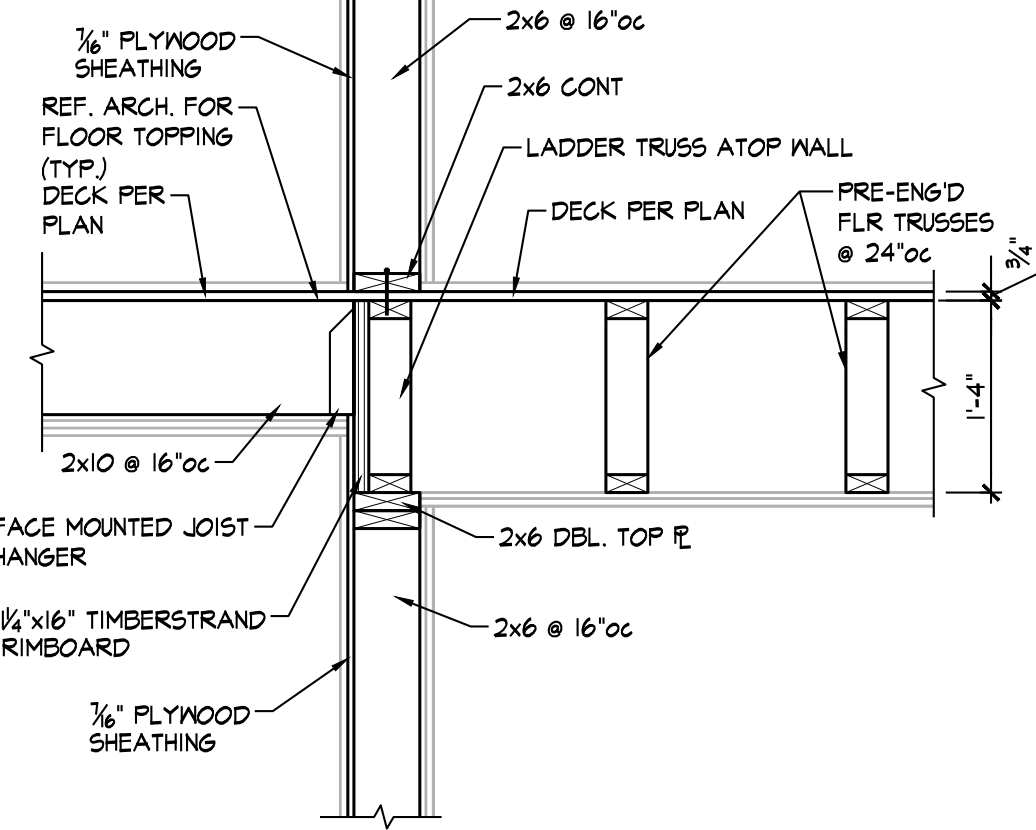
SECTION 13  
3/4\" = 1'-0\" S3.1



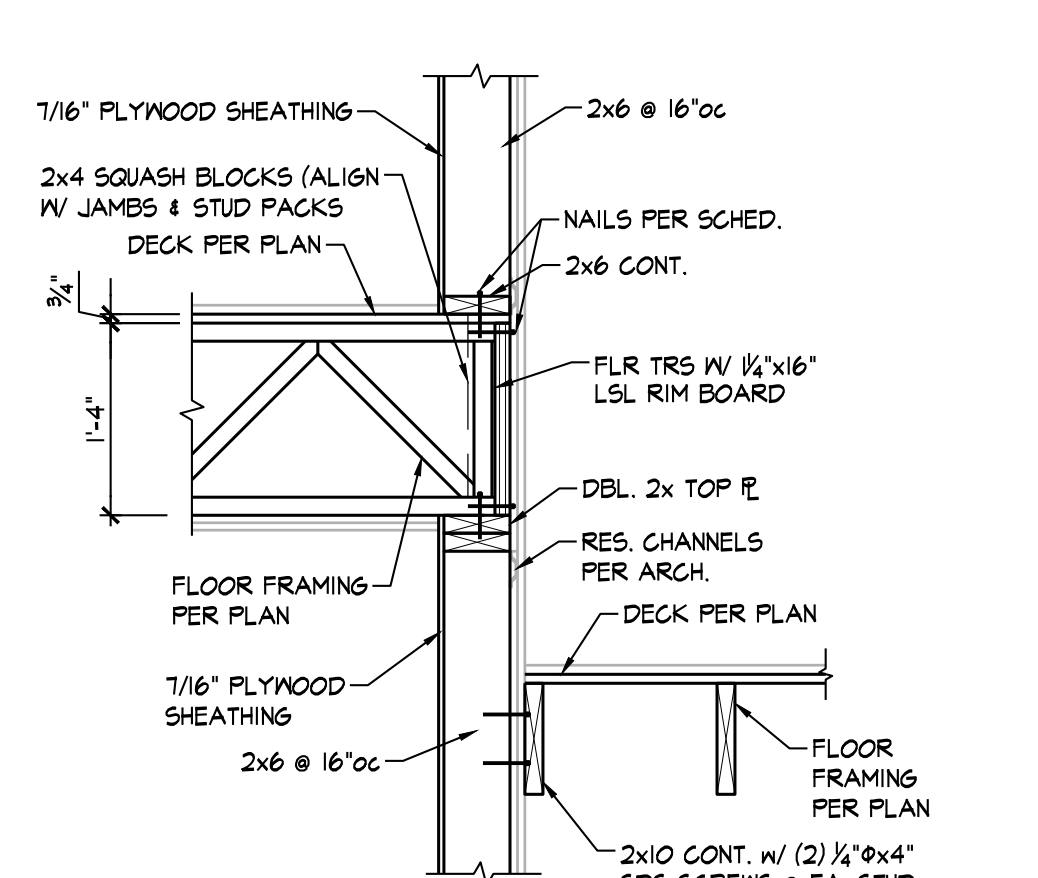
SECTION 1  
3/4" = 1'-0" S3.2



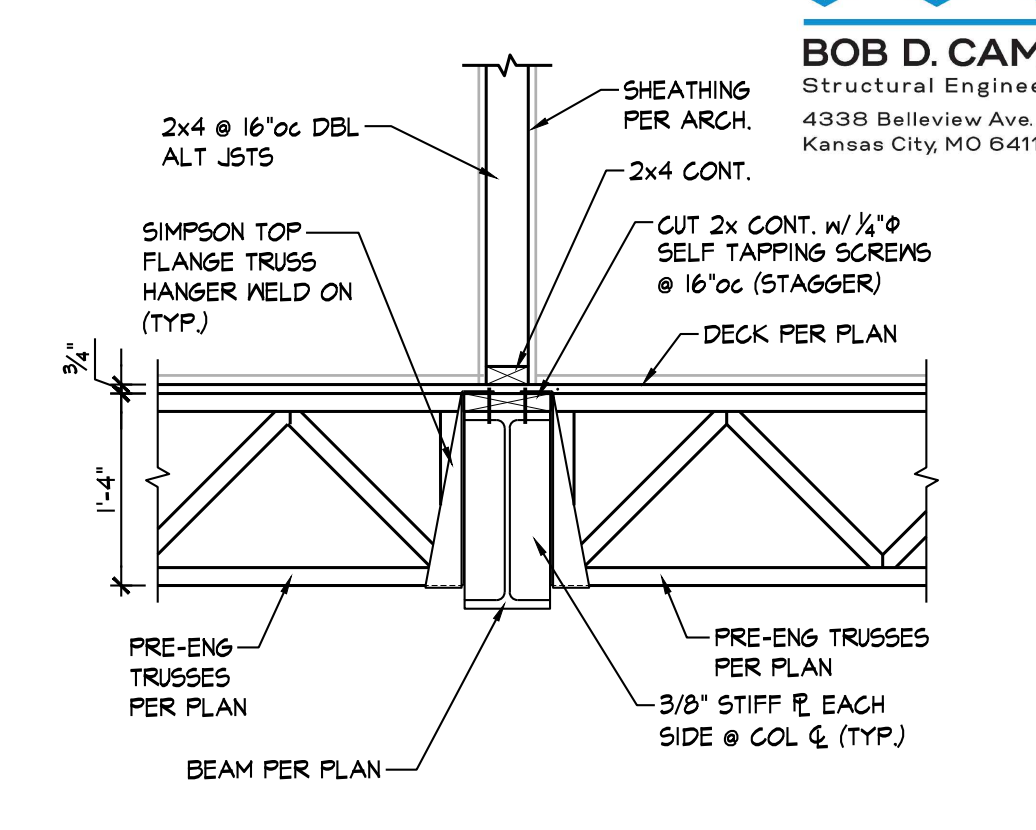
SECTION 2  
3/4" = 1'-0" S3.2



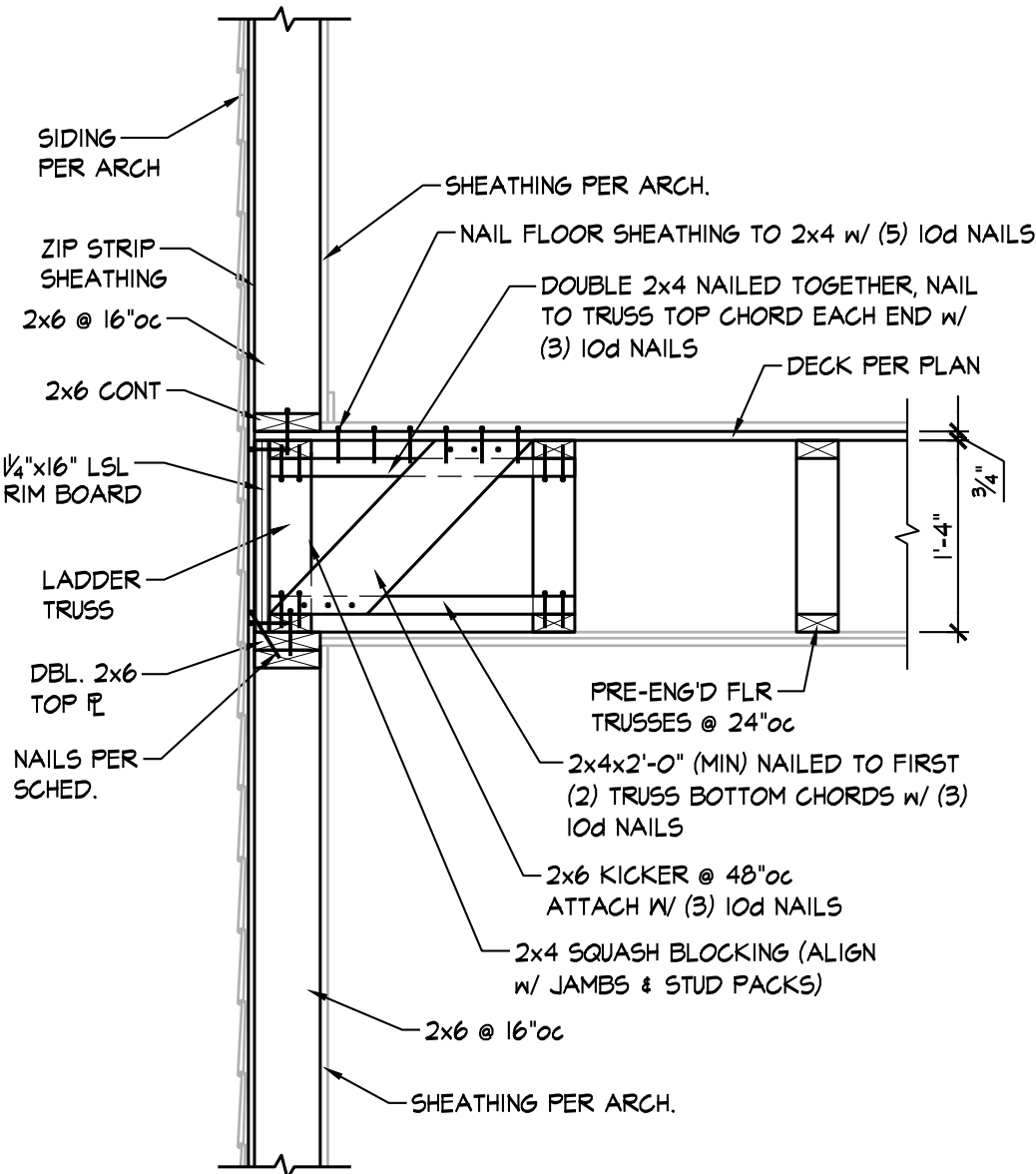
SECTION 3  
3/4" = 1'-0" S3.2



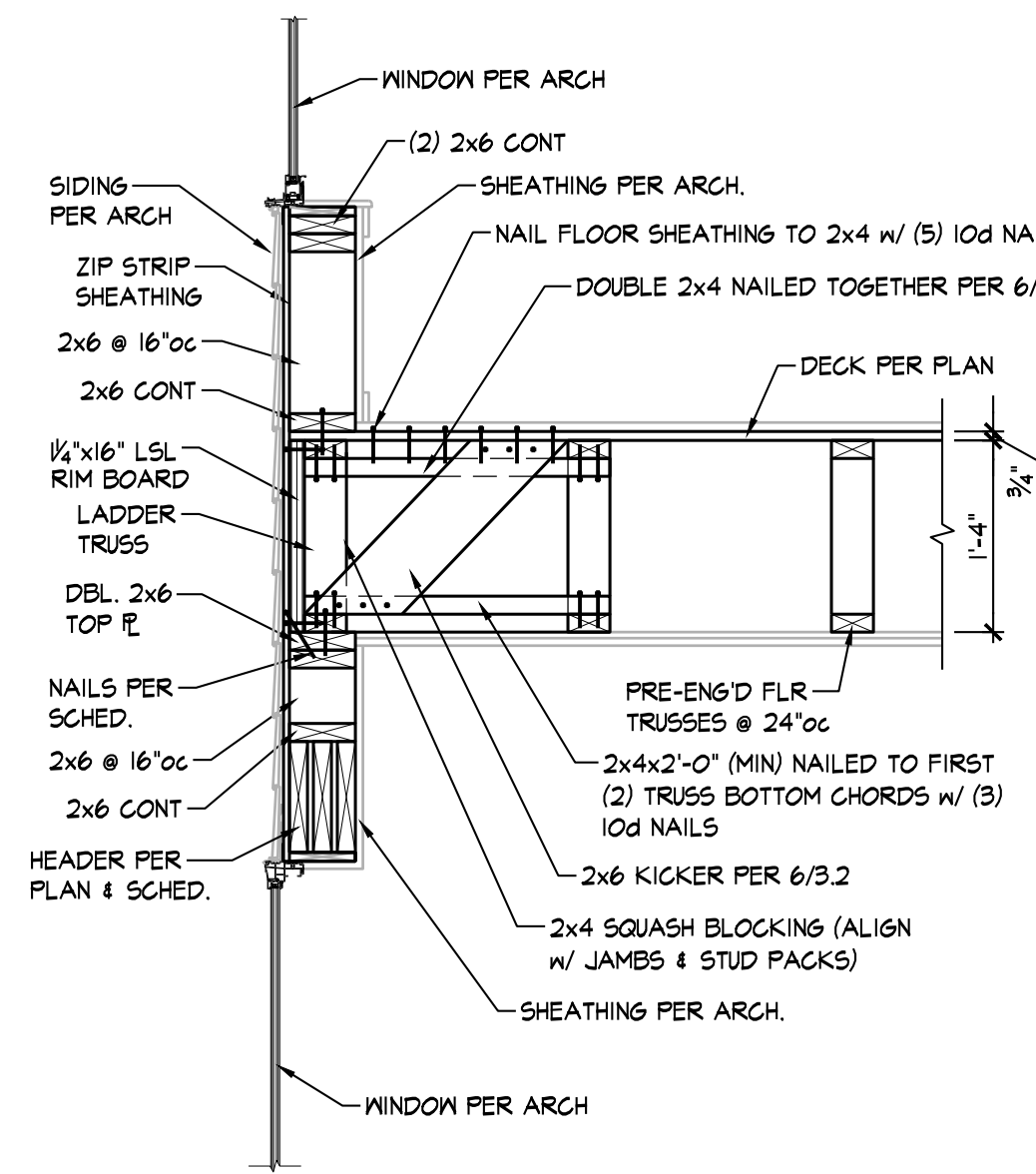
SECTION 4  
3/4" = 1'-0" S3.2



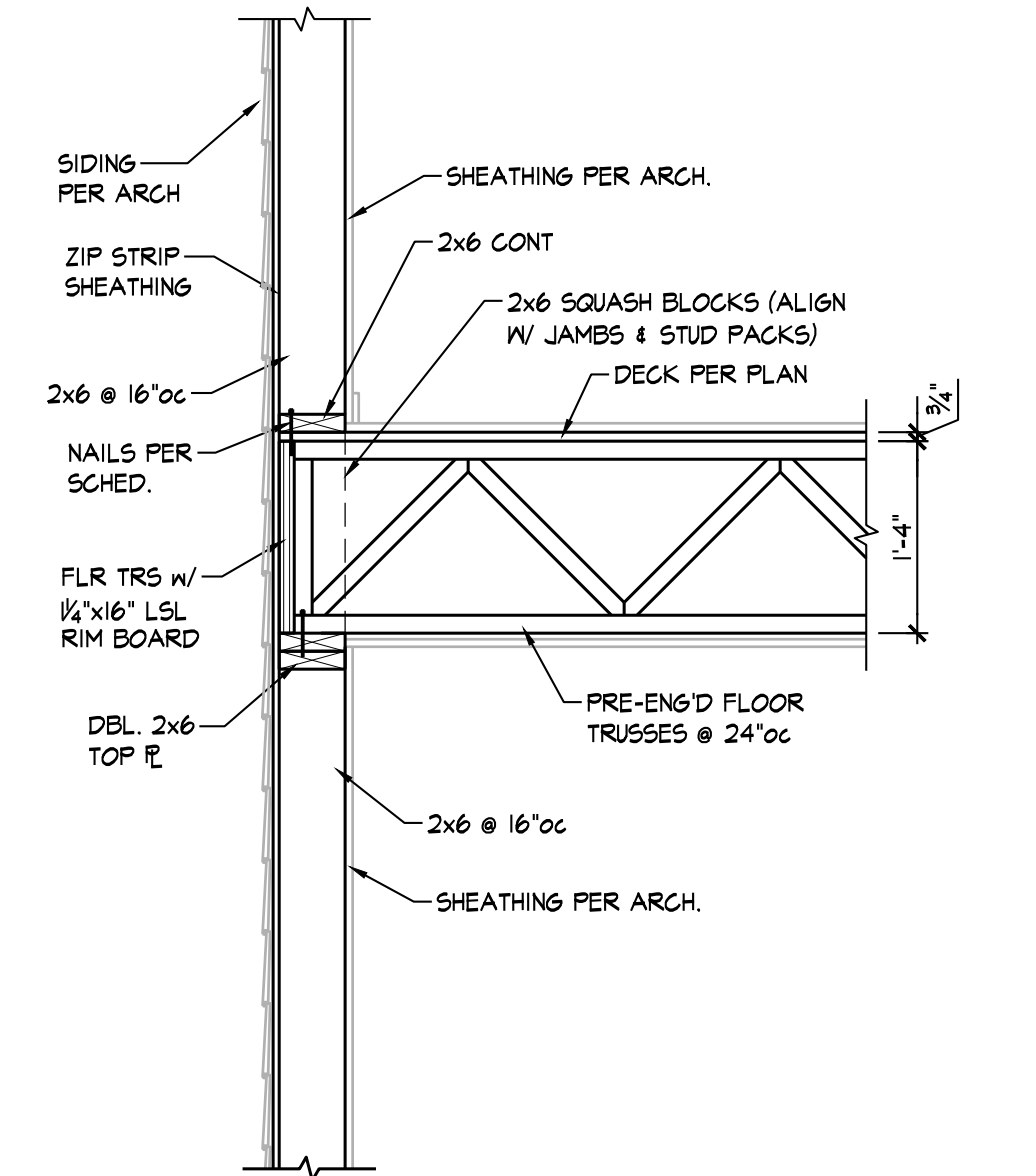
SECTION 5  
3/4" = 1'-0" S3.2



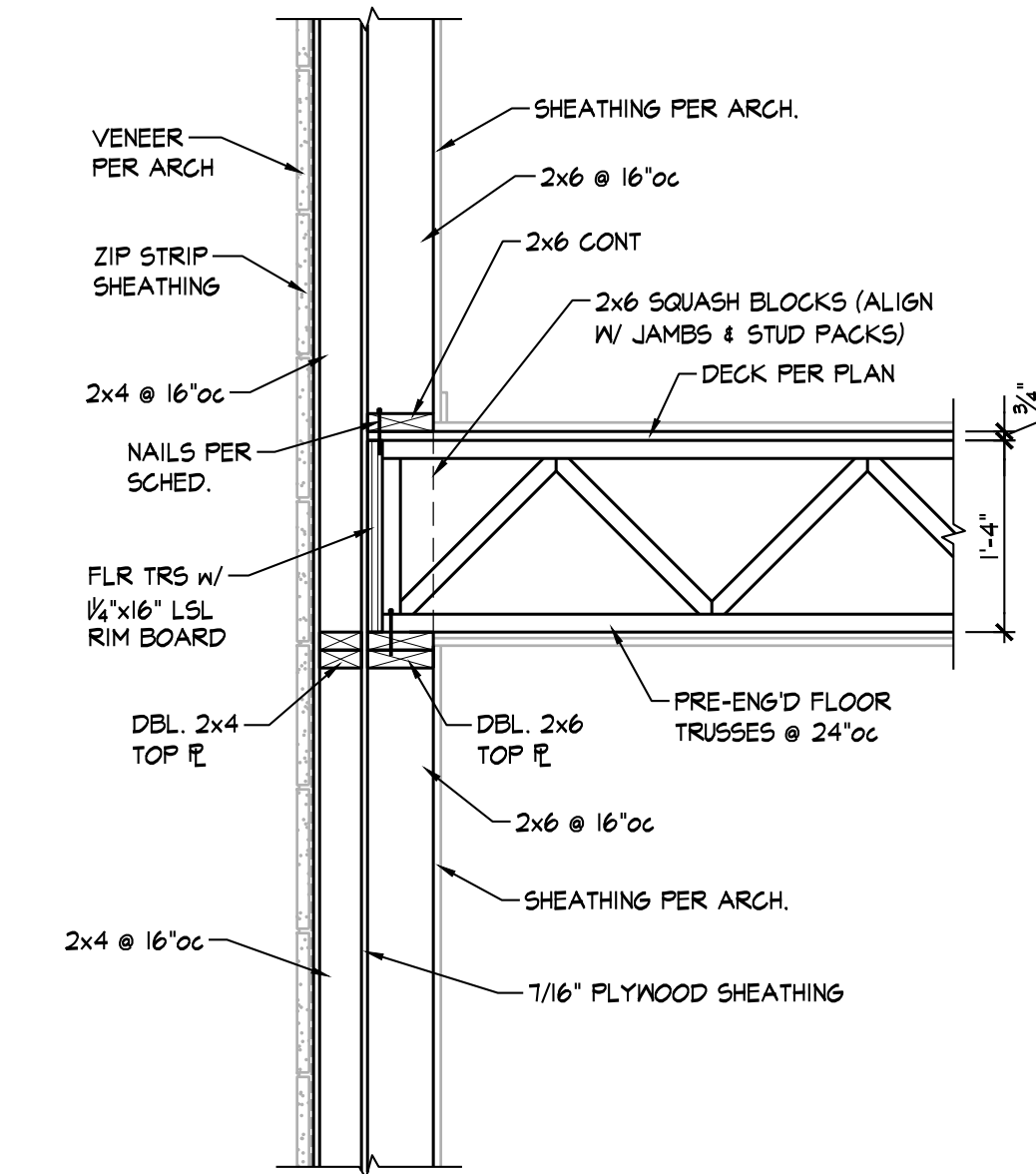
SECTION 6  
3/4" = 1'-0" S3.2



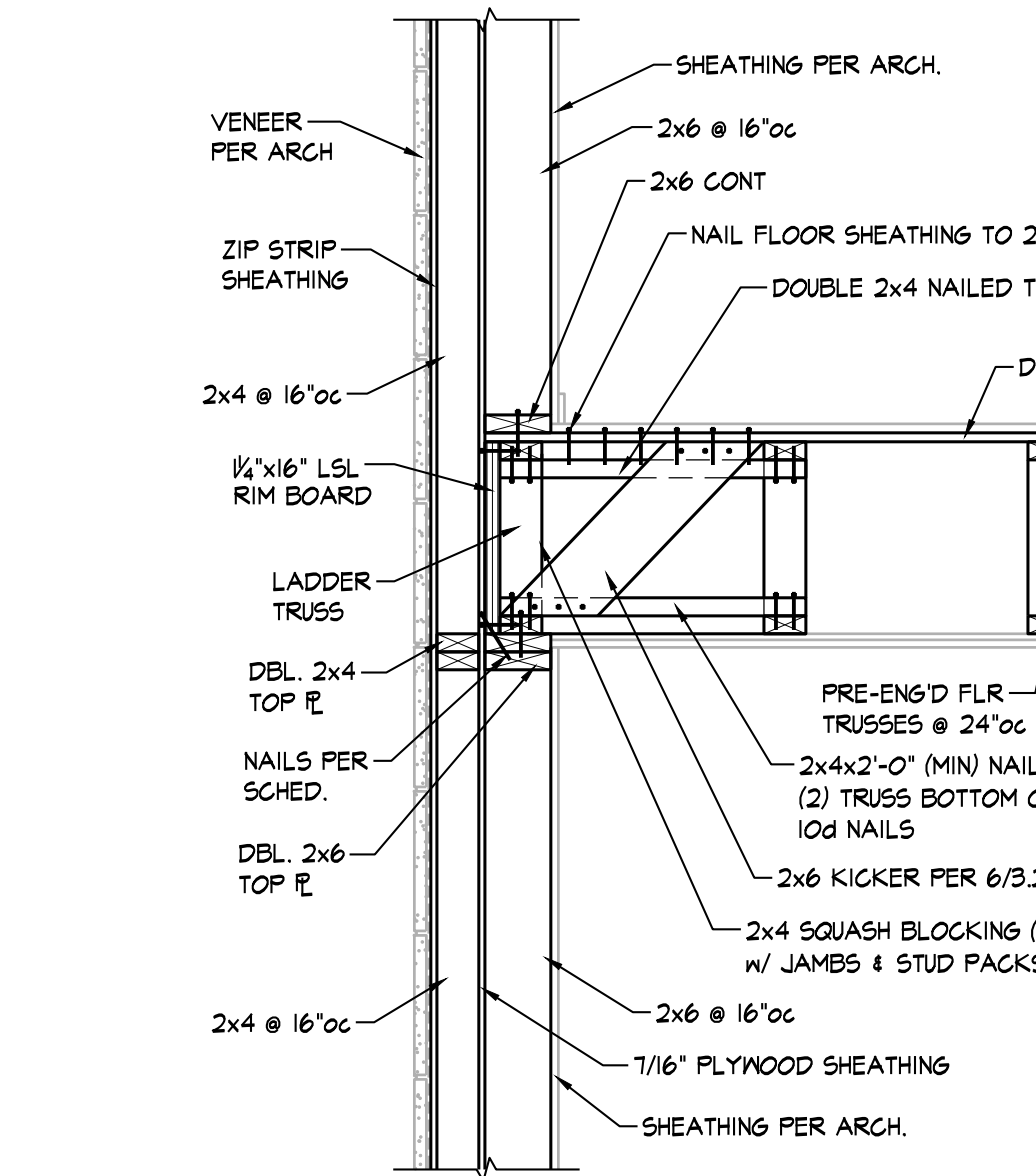
SECTION 6A  
3/4" = 1'-0" S3.2



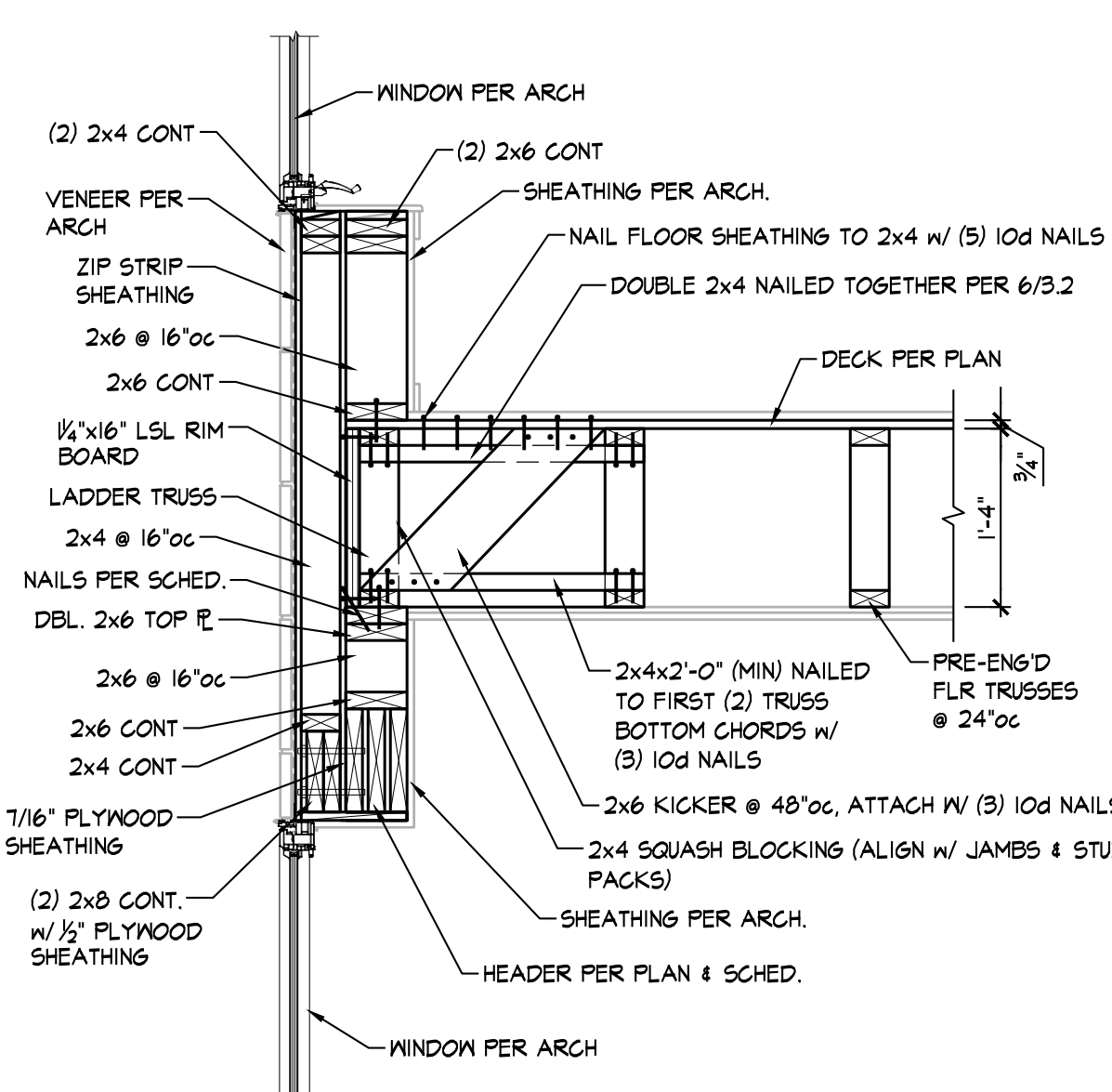
SECTION 7  
3/4" = 1'-0" S3.2



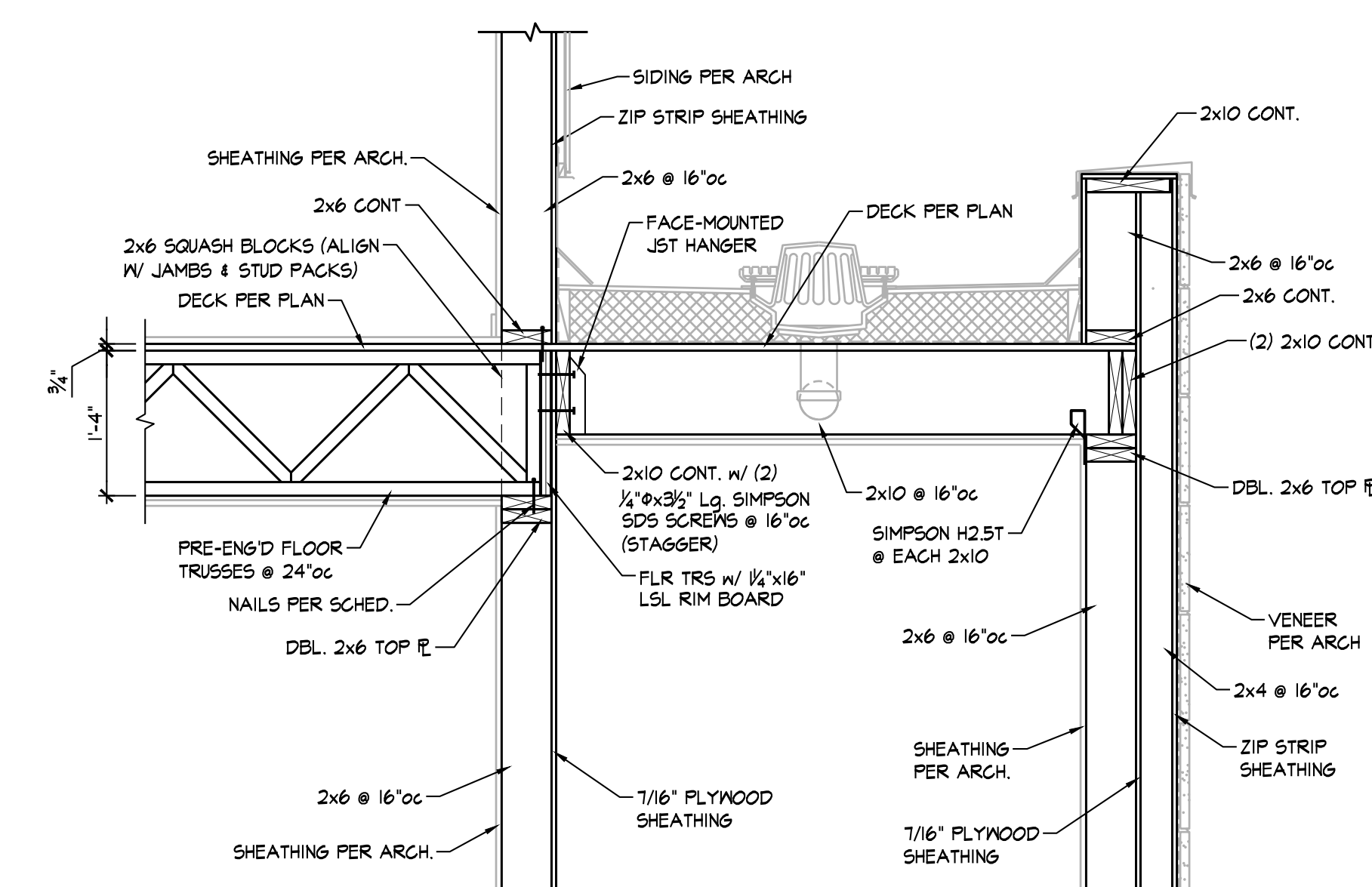
SECTION 7A  
3/4" = 1'-0" S3.2



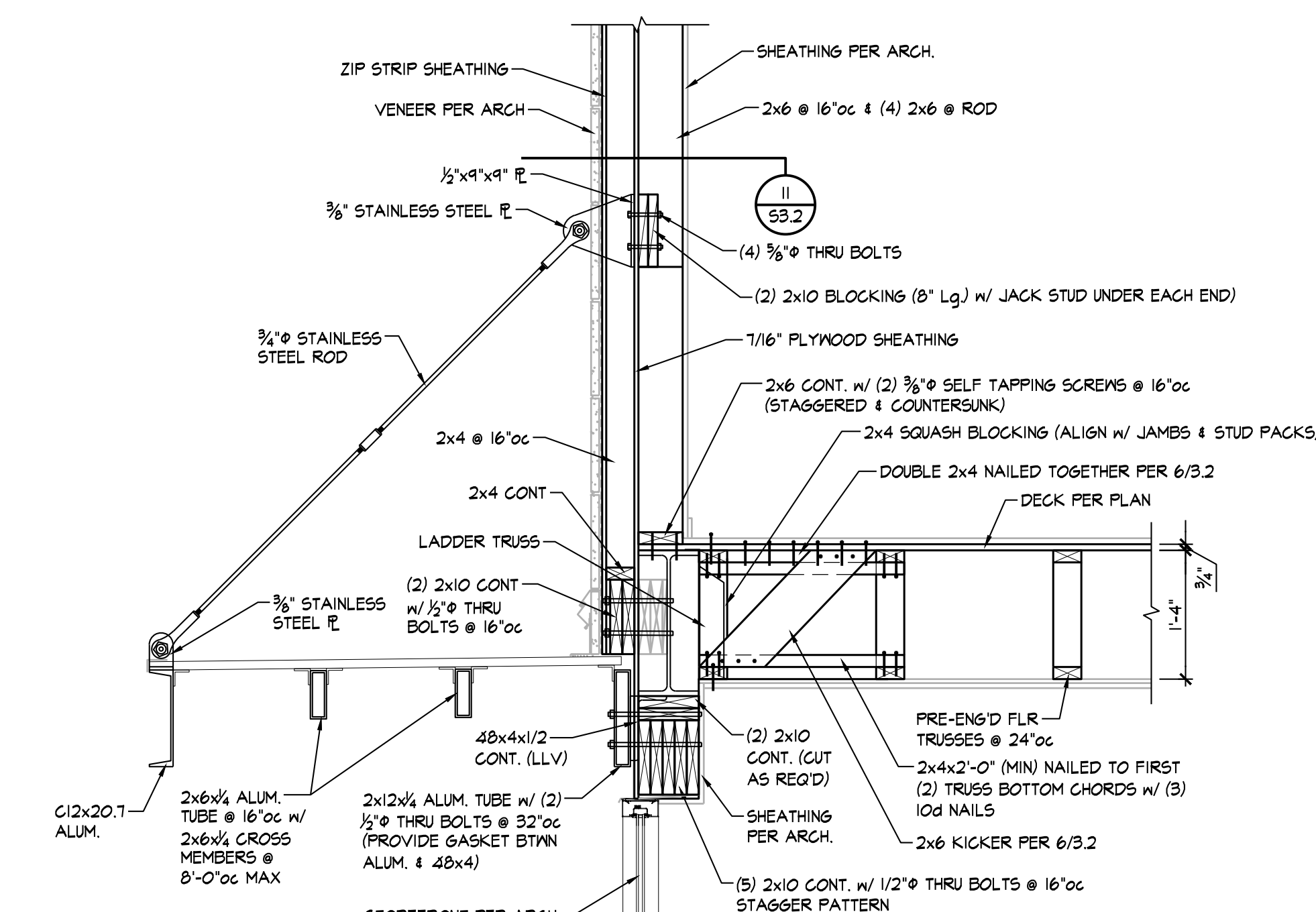
SECTION 8  
3/4" = 1'-0" S3.2



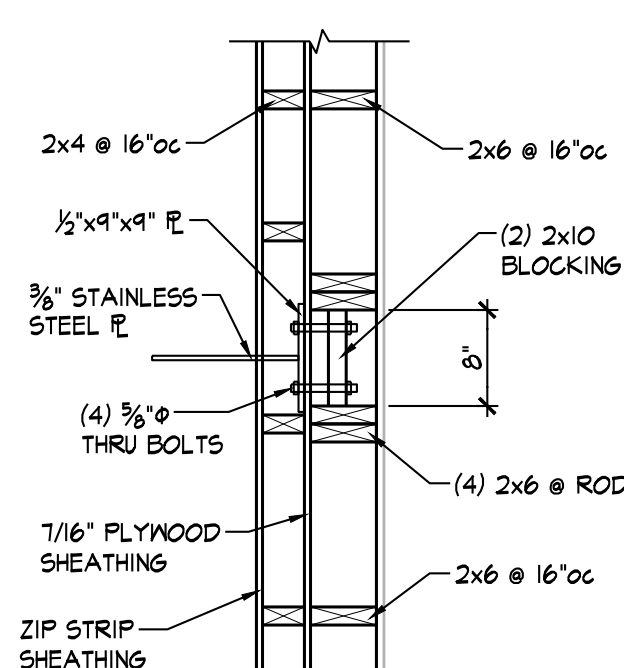
SECTION 8A  
3/4" = 1'-0" S3.2



SECTION 9  
3/4" = 1'-0" S3.2



SECTION 10  
3/4" = 1'-0" S3.2



SECTION 11  
3/4" = 1'-0" S3.2

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785.827.0386 jgr@jgarchitects.com

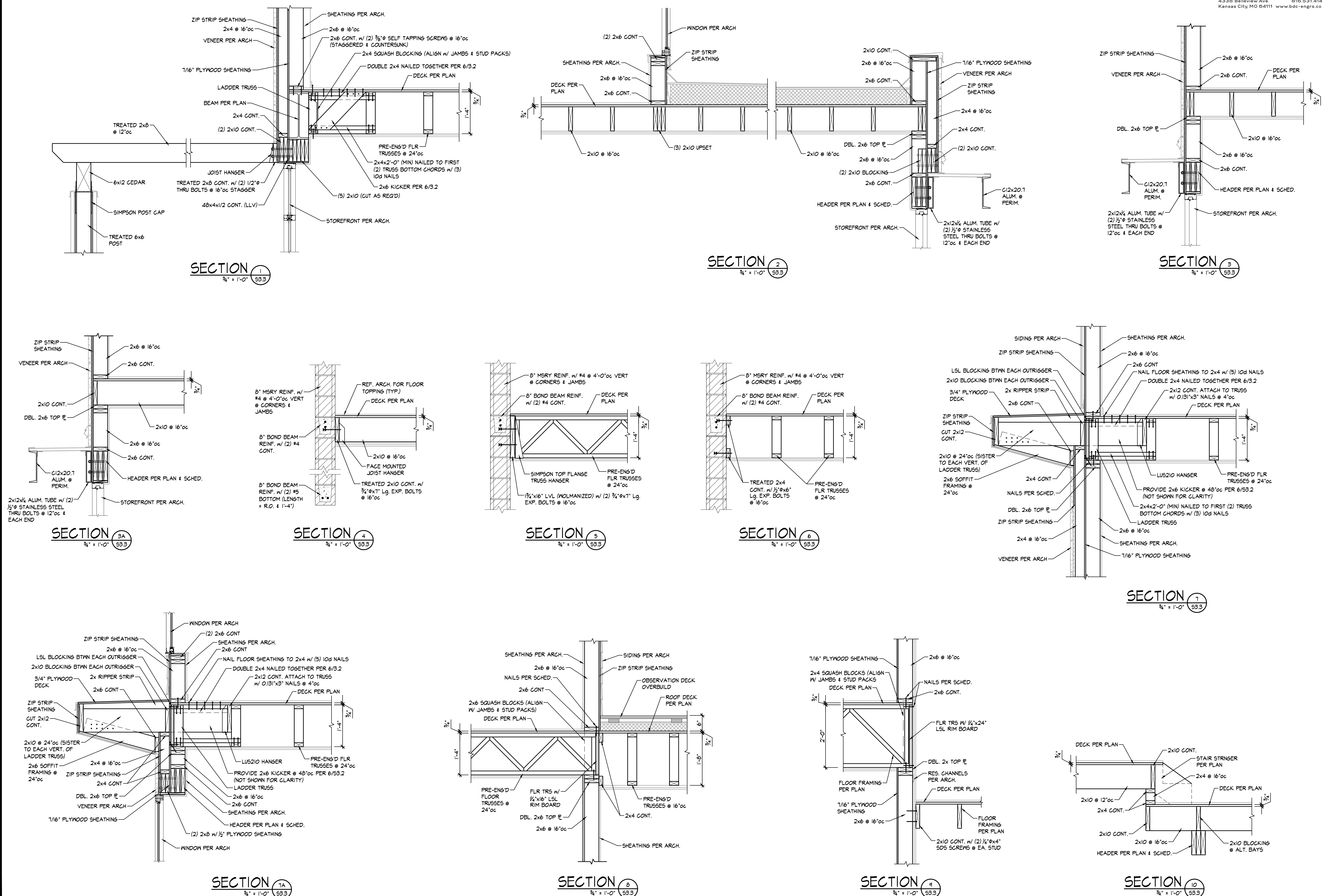
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NEW APARTMENTS  
ANDOVER, KANSAS

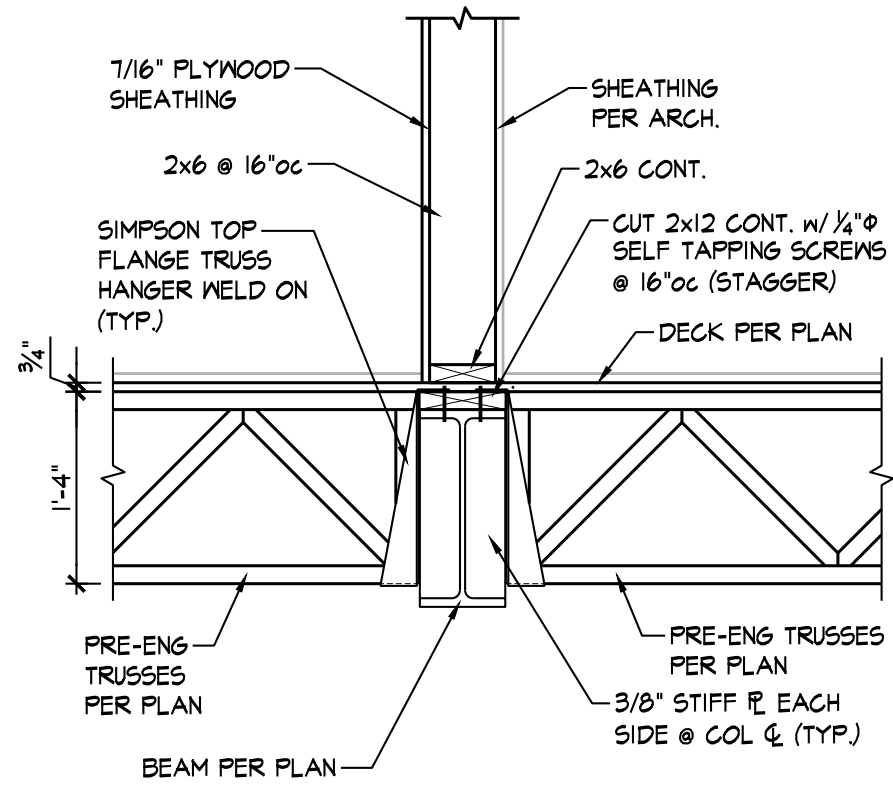
PROFESSIONAL ENGINEER  
10296  
1.3.2022  
MICHAEL J. FALSBY

REVISION:  
DATE: 1-03-2025  
JOB: 24-3385  
SHEET NO.:

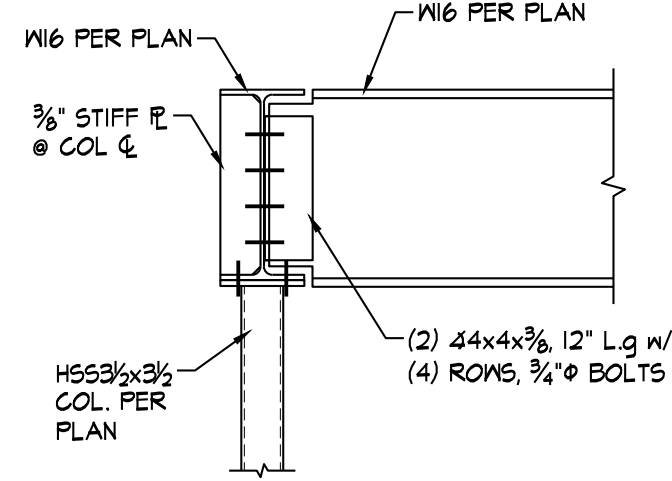
**S3.2**

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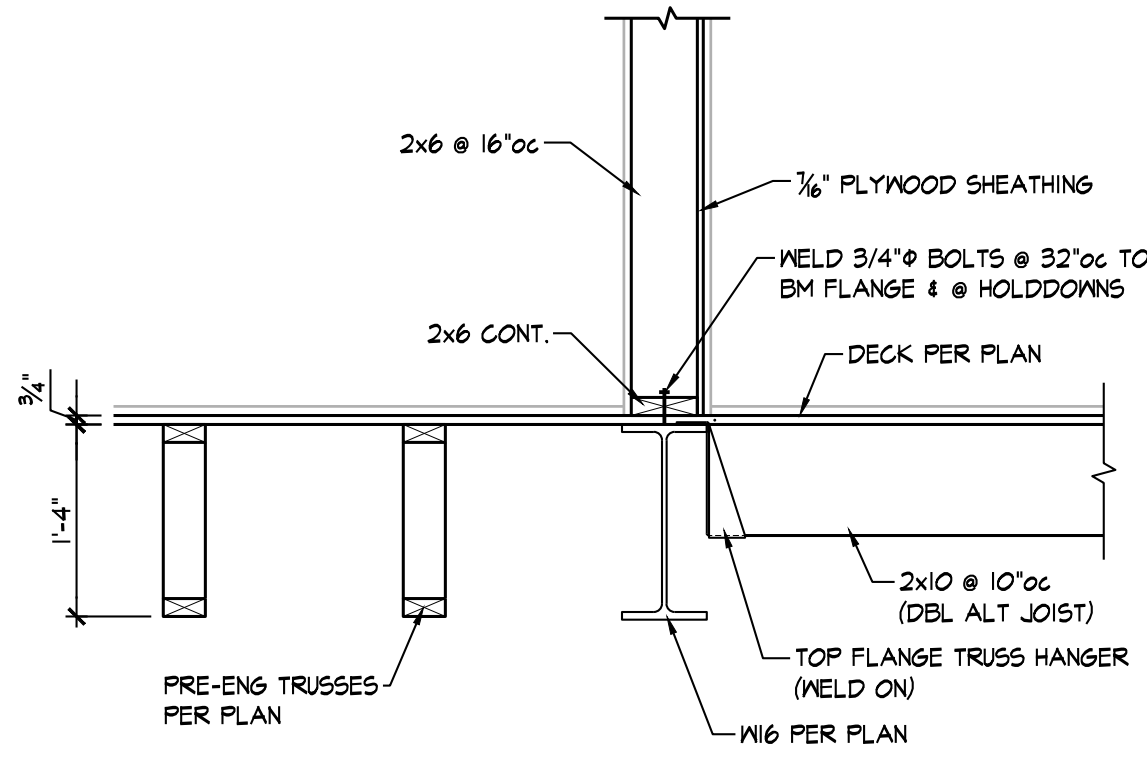


SECTION 1  
3/4" = 1'-0" S3.4

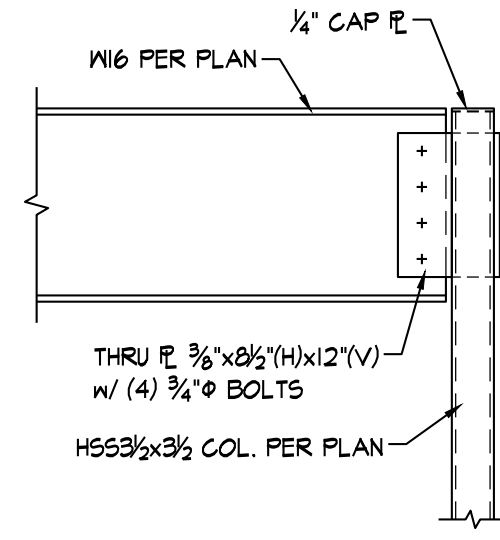


SECTION 2  
3/4" = 1'-0" S3.4

NOTE: WOOD FRAMING  
OMITTED FOR CLARITY.

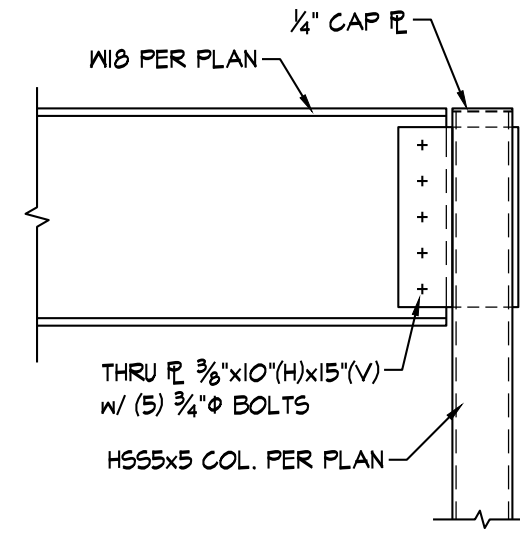


SECTION 3  
3/4" = 1'-0" S3.4



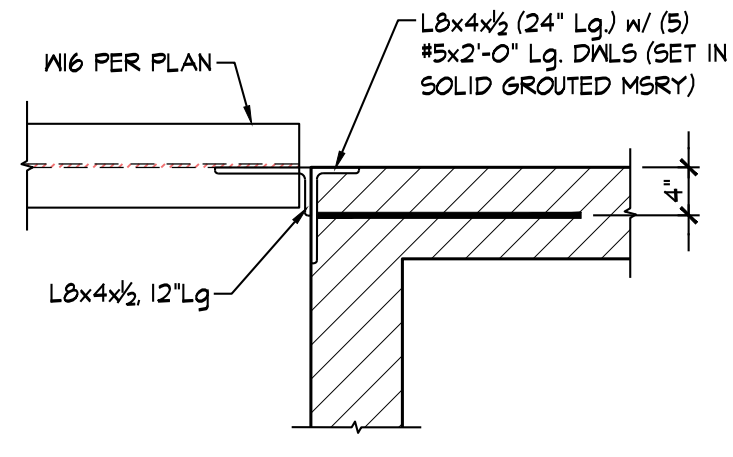
SECTION 4  
3/4" = 1'-0" S3.4

NOTE: WOOD FRAMING  
OMITTED FOR CLARITY.

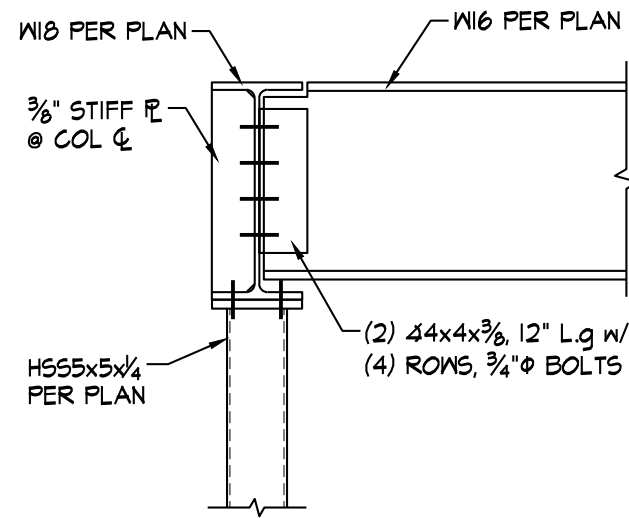


SECTION 5  
3/4" = 1'-0" S3.4

NOTE: WOOD FRAMING  
OMITTED FOR CLARITY.

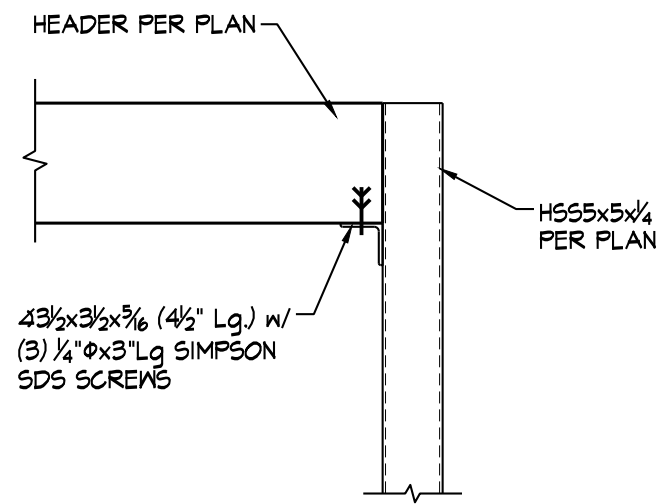


DETAIL 6  
3/4" = 1'-0" S3.4



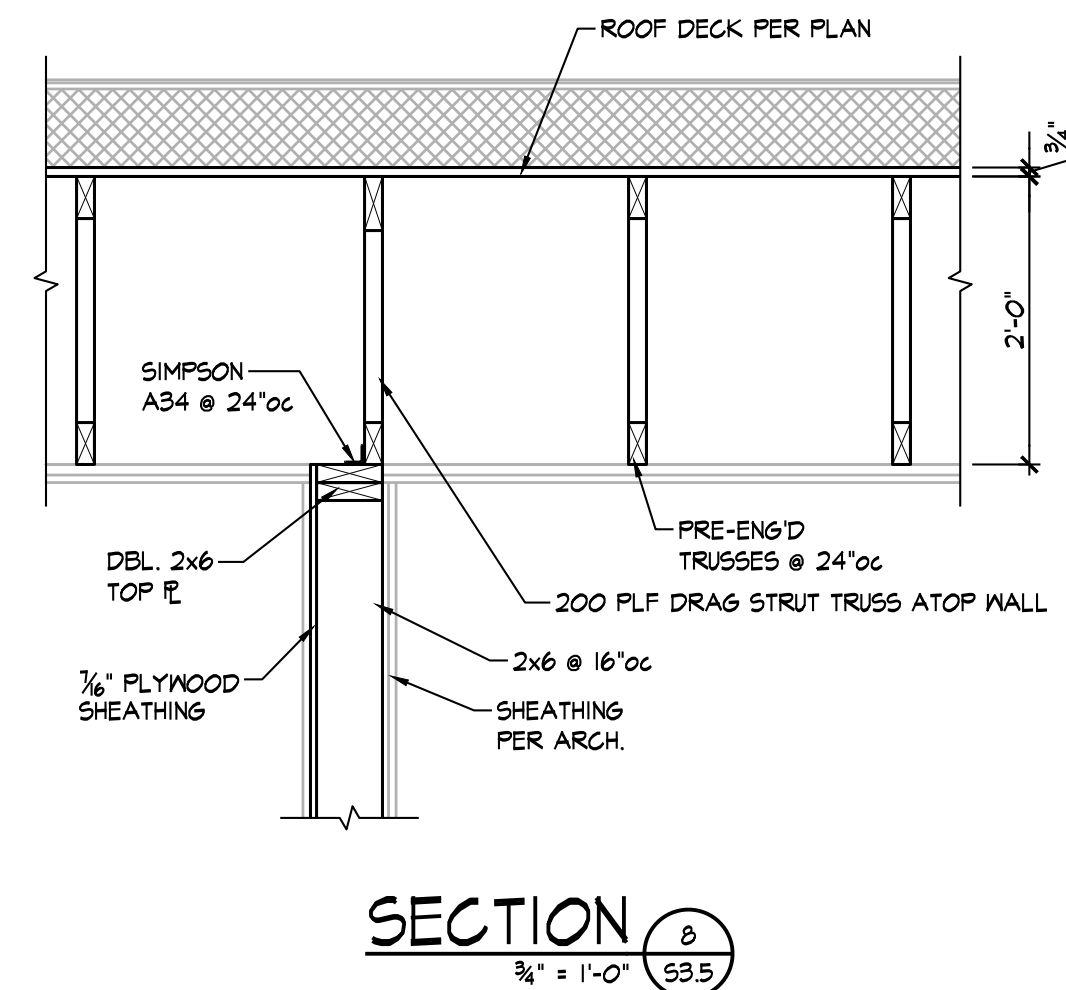
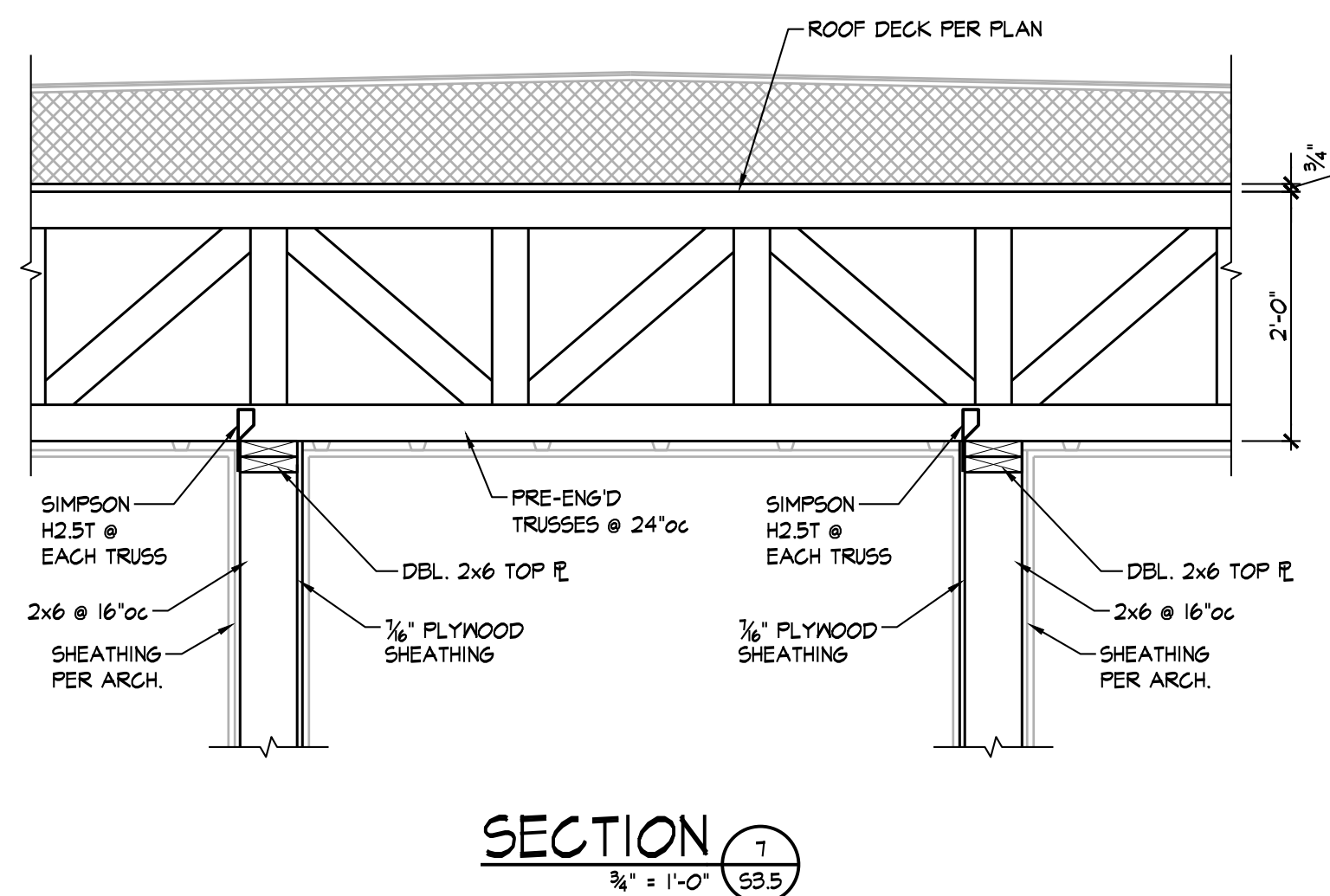
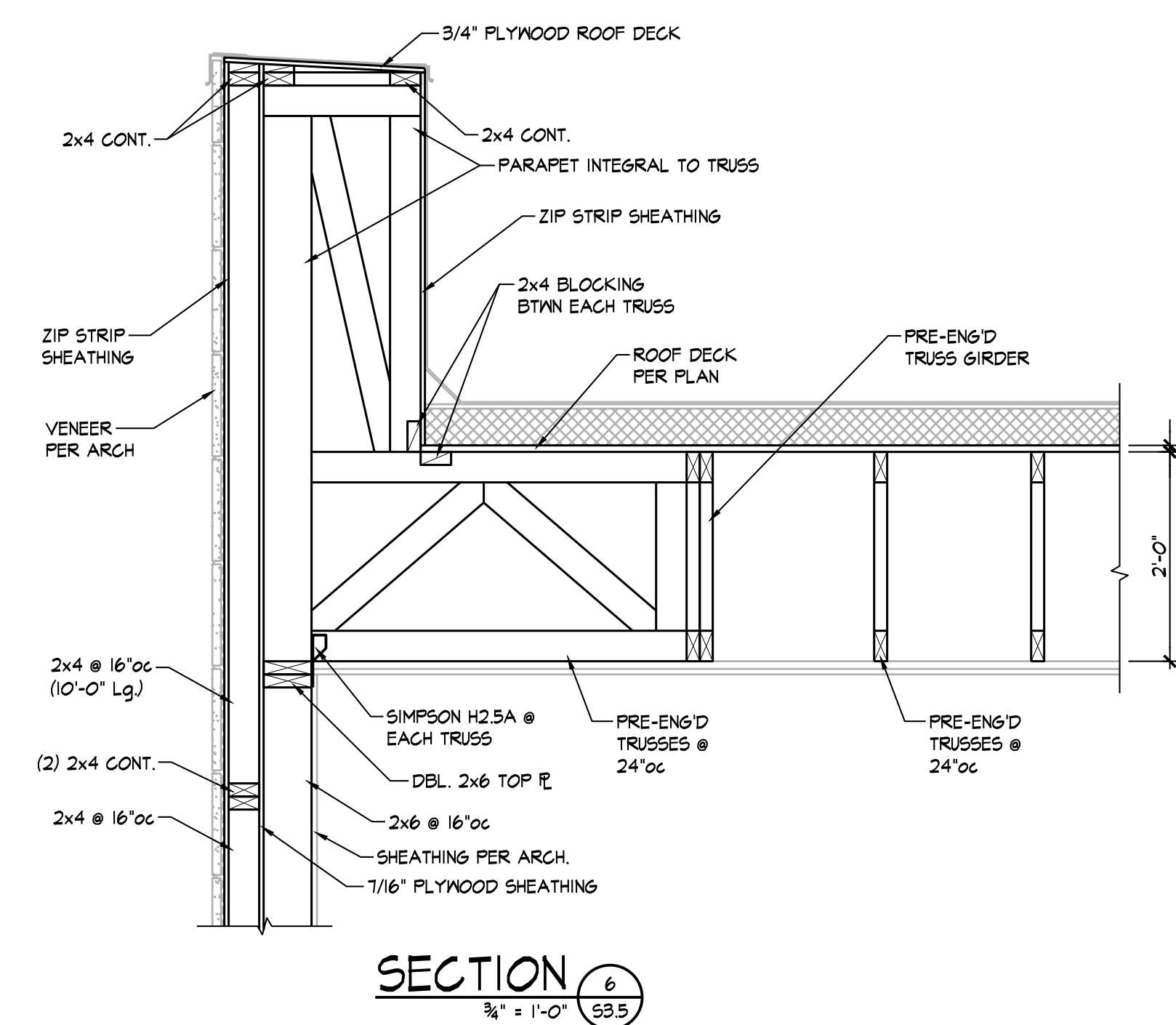
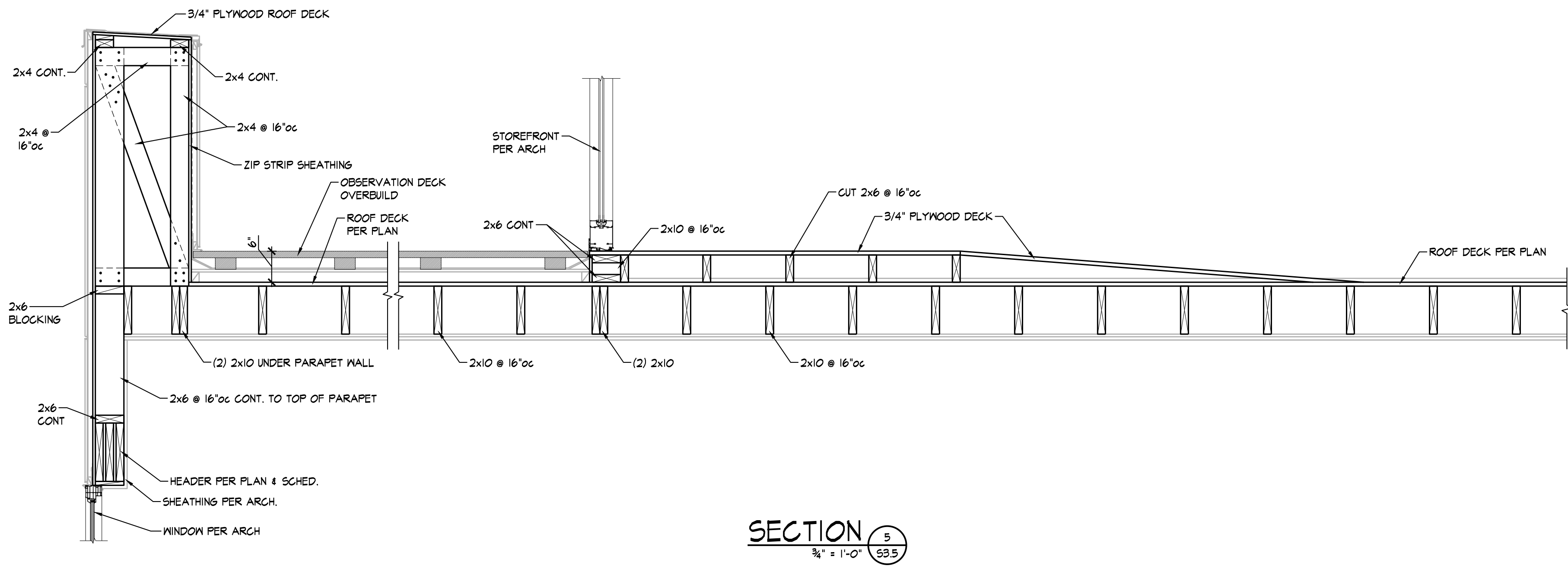
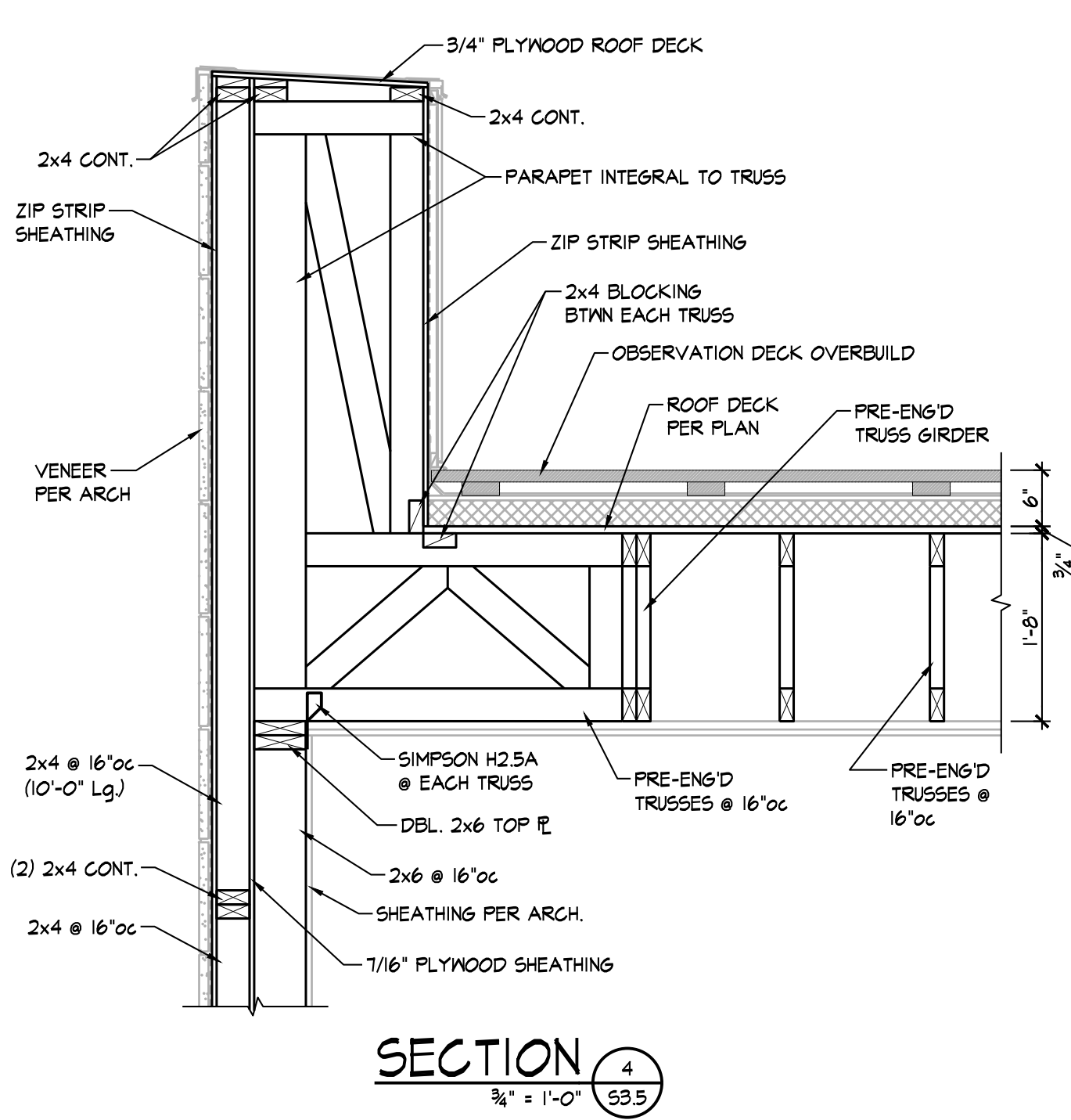
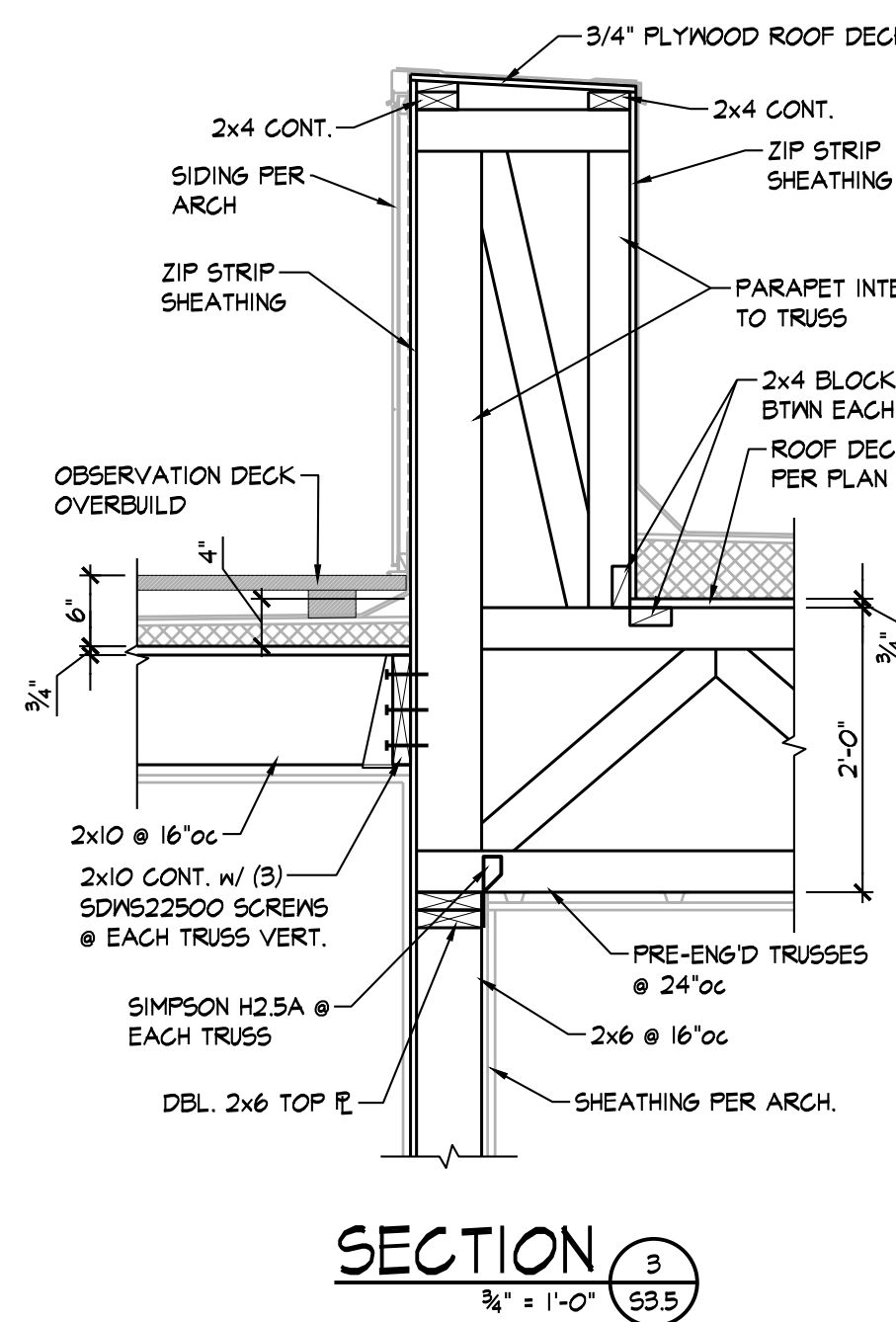
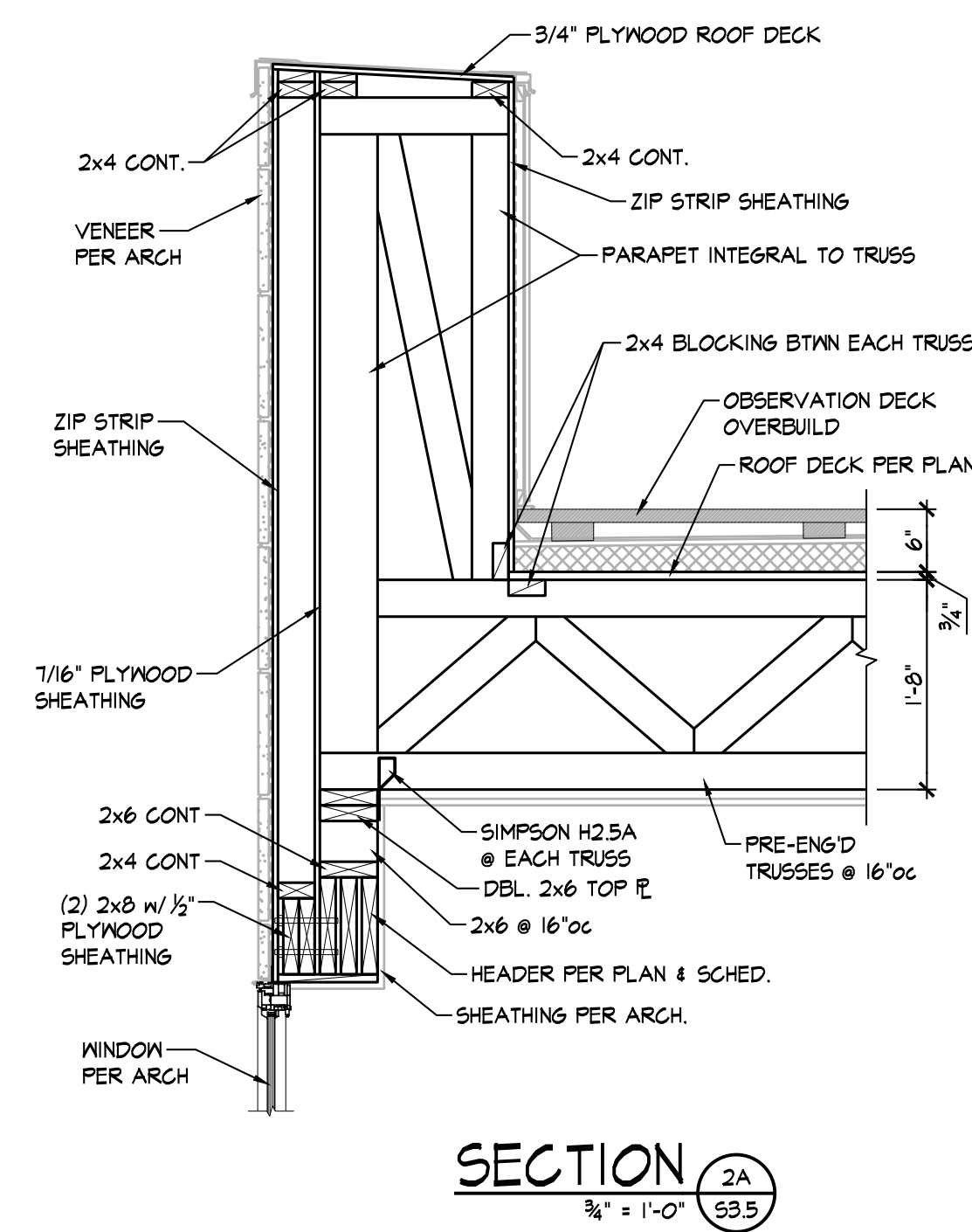
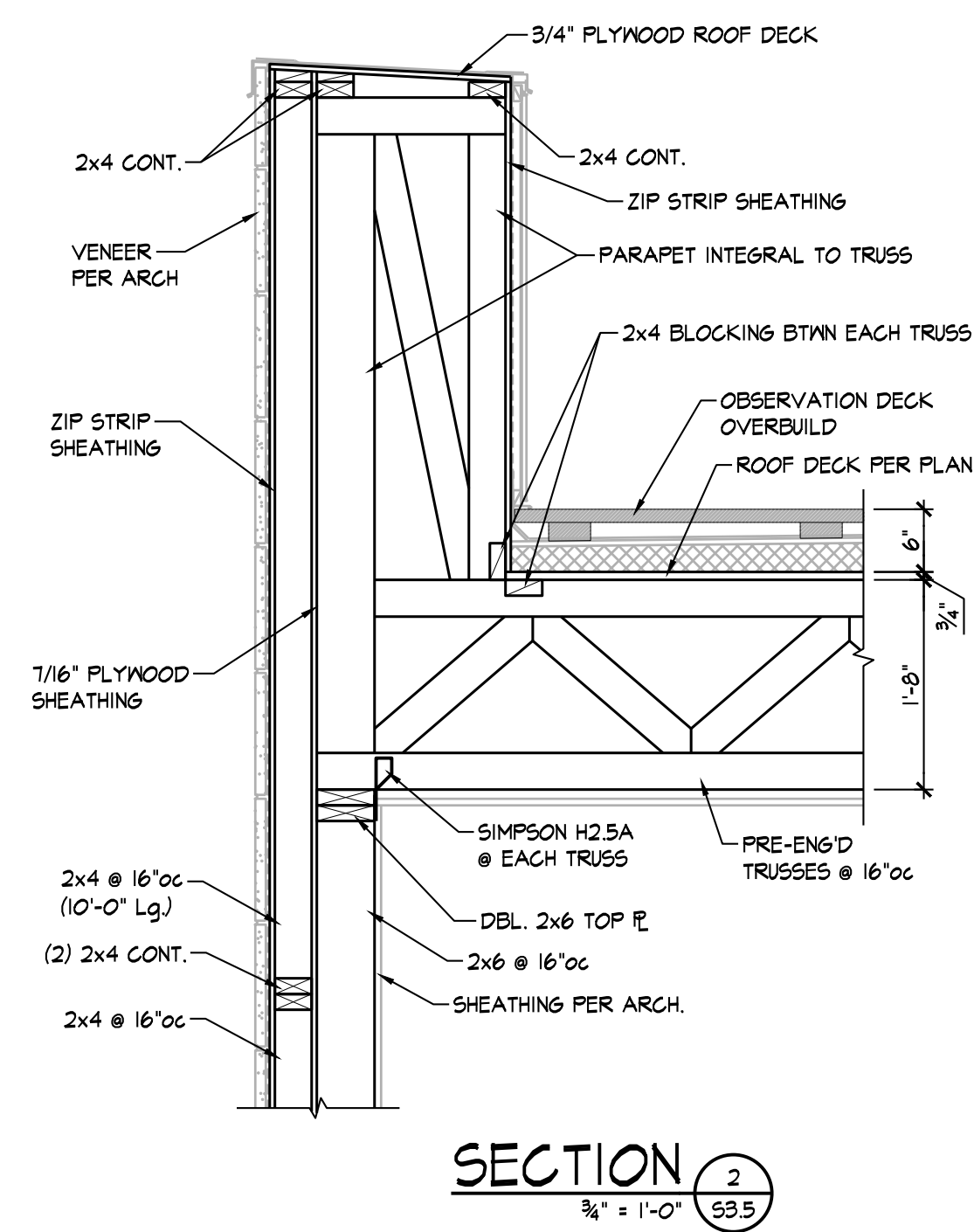
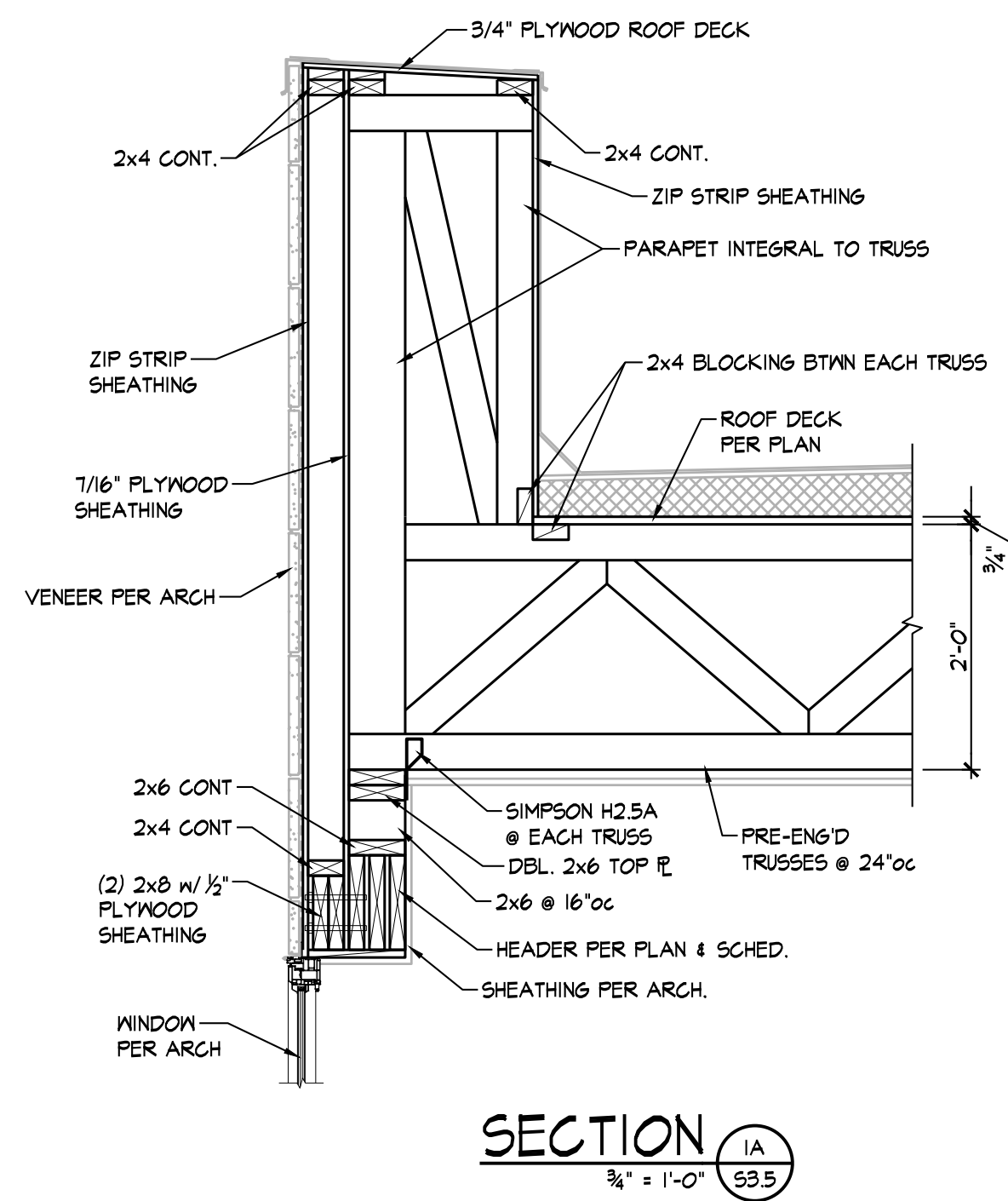
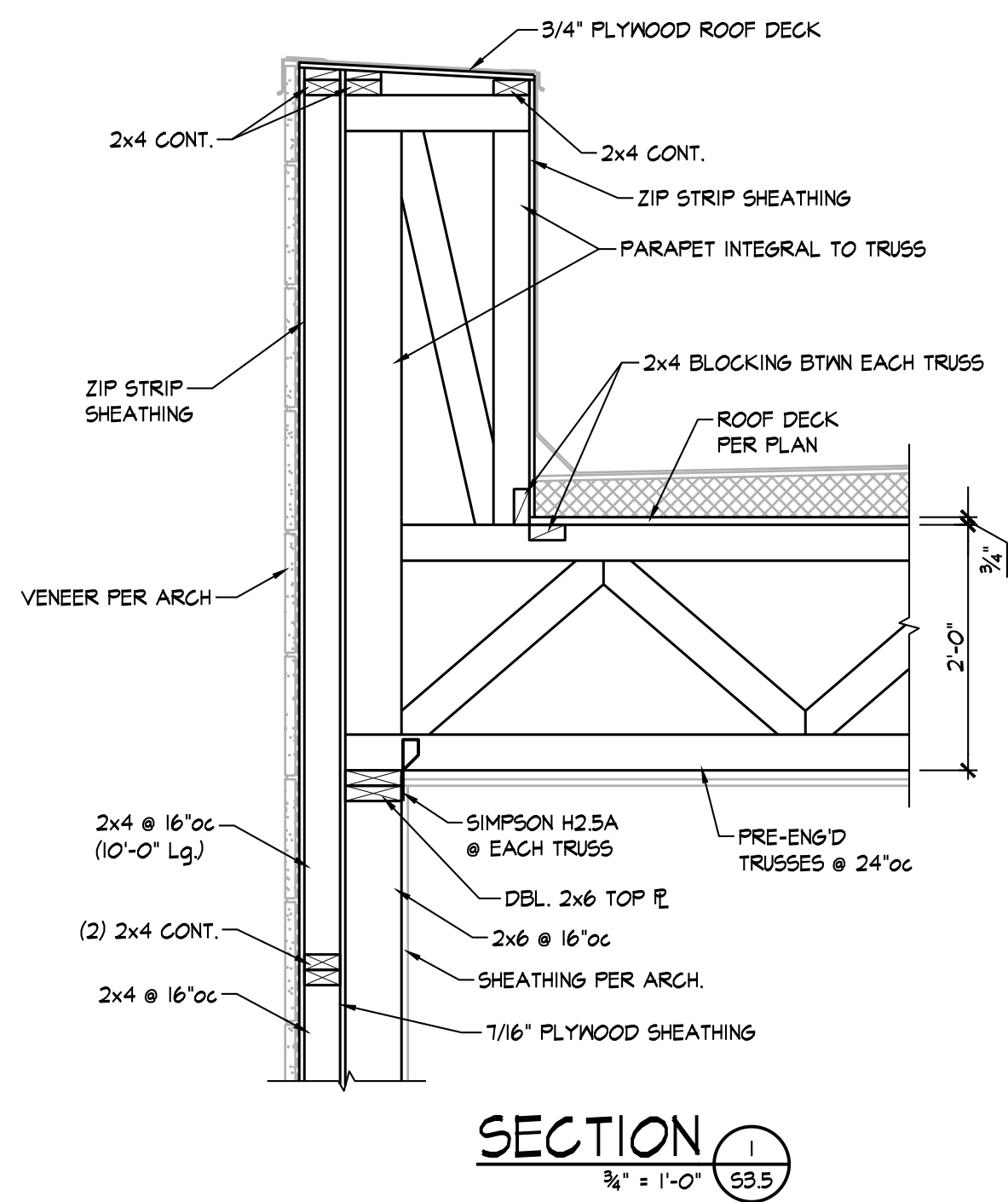
SECTION 7  
3/4" = 1'-0" S3.4

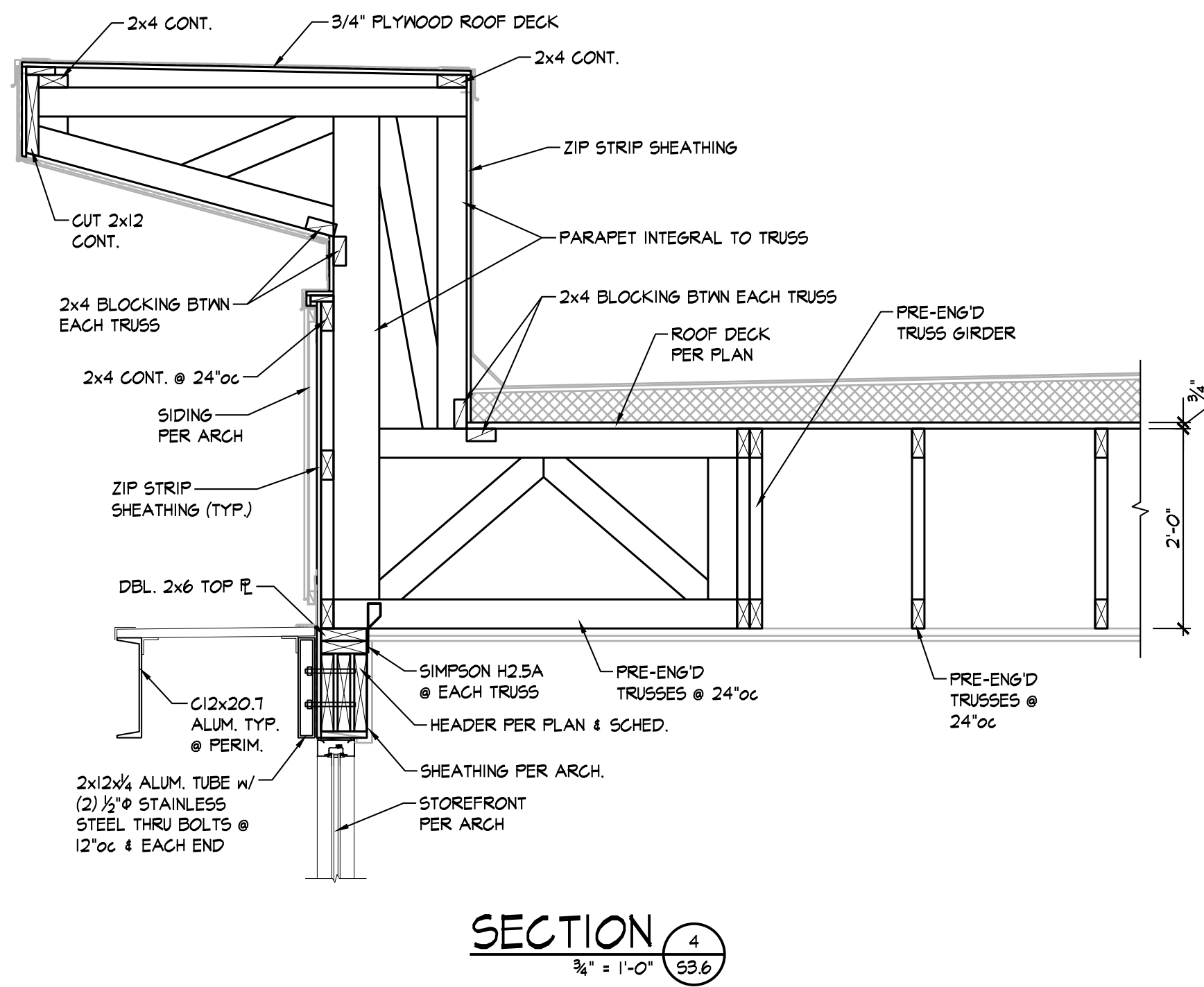
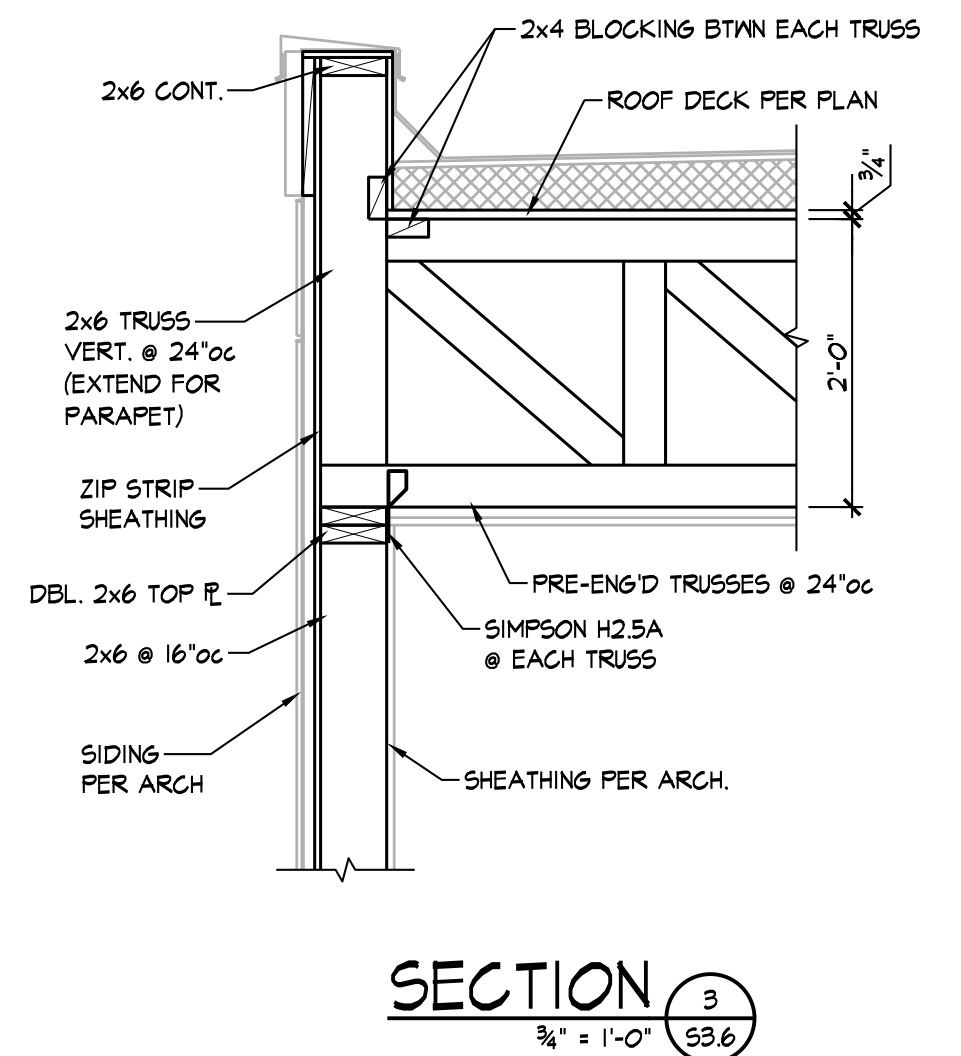
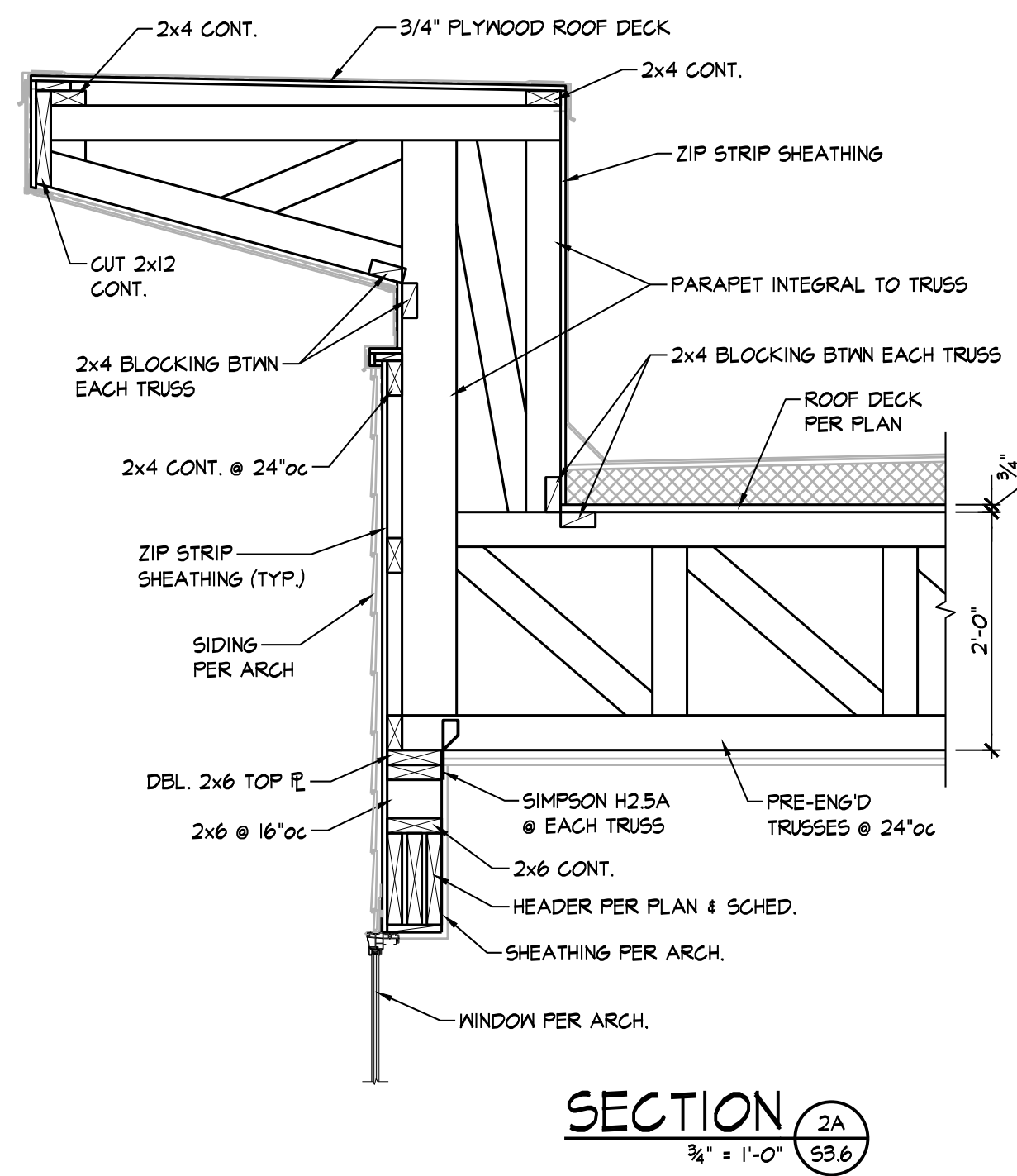
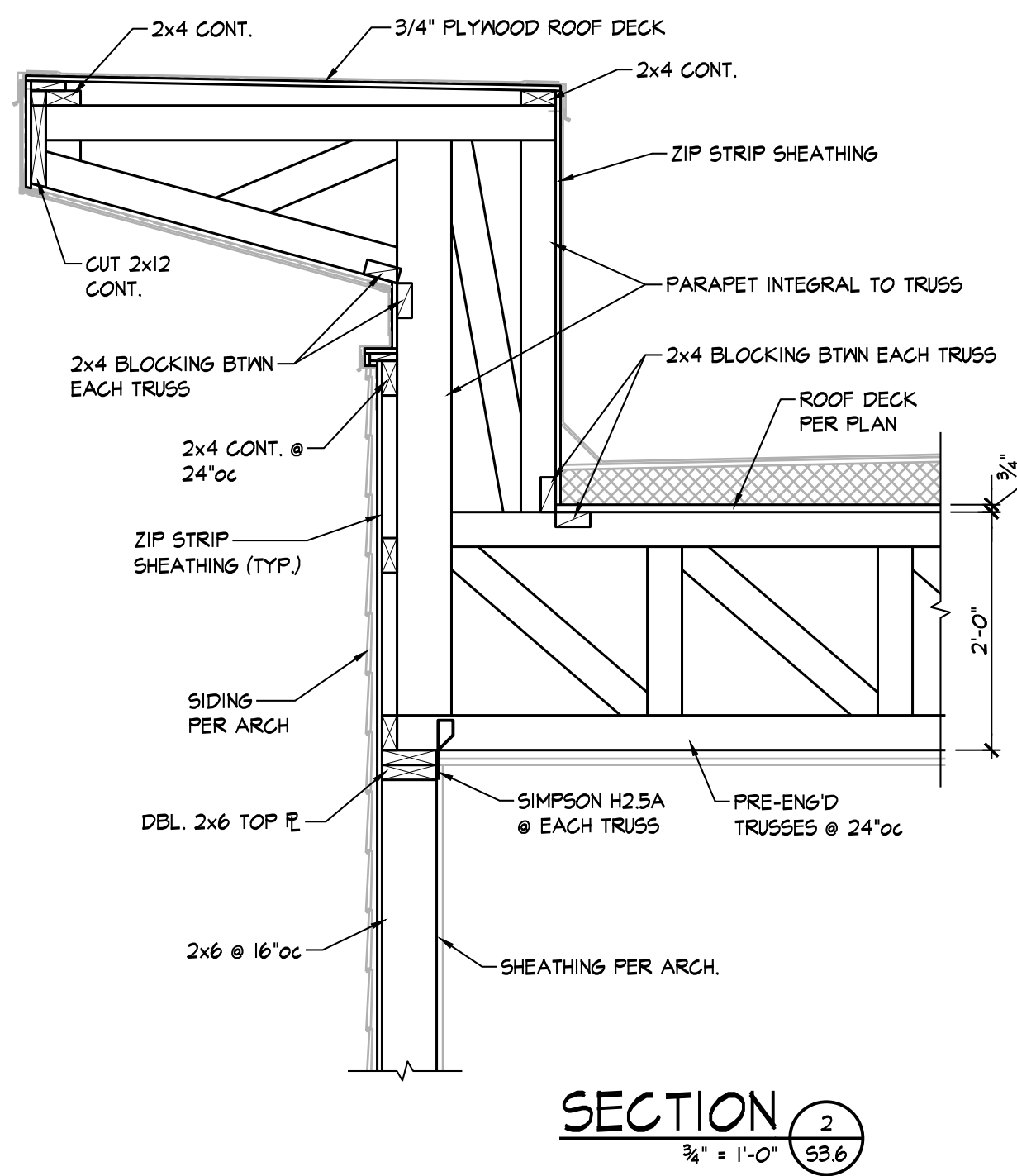
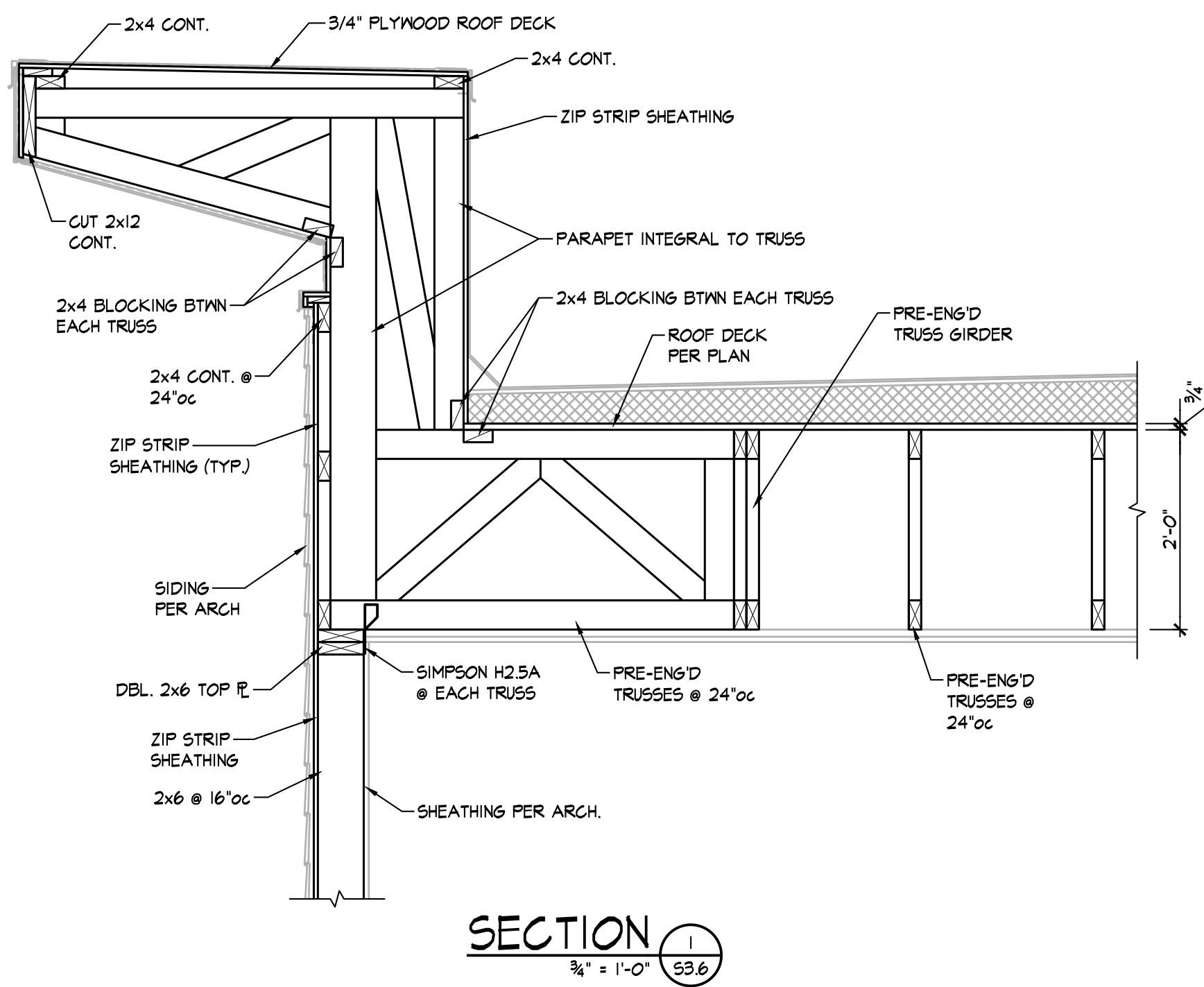
NOTE: WOOD FRAMING  
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SECTION 8  
3/4" = 1'-0" S3.4

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730 N. Ninth Street, Suite 301  
Solina, KS 67401 Kansas City, MO 64108  
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