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NOTICE TO ALL CONTRACTORS AND SUB-CONTRACTORS

October 15, 2026

Theatre Salina Expansion – Proj #22-3239

ADDENDUM NO. 4

YOU ARE INSTRUCTED TO READ AND TO NOTE THE FOLLOWING DESCRIBED CHANGES, CORRECTIONS, CLARIFICATIONS, OMISSIONS, DELETIONS, ADDITIONS, APPROVALS, AND STATEMENTS PERTINENT TO THE CONTRACT AND CONSTRUCTION DOCUMENTS. THIS ADDENDUM IS A PART OF THE CONTRACT AND CONSTRUCTION DOCUMENTS AND SHALL GOVERN IN THE PERFORMANCE OF THE WORK.

Bid Due: Tuesday, October 20th at 2:00 pm to JGR Architects, 730 N. 9th St, Salina, KS

ARCHITECTURAL – Specifications

1. Section 10850 Building Specialties, Paragraph 1.05 Expansion Joint Covers, Paragraph A. Delete reference to Hall 006 and Hall 124. Reference the attached Revised Sheet A2.1 for expansion joint cover locations at walls.
2. Section 01019 Special Provisions – Paragraph 5.f.2 – Clarification, the signage allowance is to be used for building signage that is not currently identified on the drawings. This shall include restroom signage, wall mounted room signage, dedication signage, etc. The exterior metal "Inspire" sign and the (3) vinyl wall graphic "Studio" graphics are to be in the base bid.
3. Section 10850 Building Specialties, Paragraph 1.03 Cast Metal Letters, - Clarification, this cast metal lettering is to be backlit with LED lighting.

ARCHITECTURAL – Drawings

1. Sheet A3.1 and A3.2 Exterior Elevations – The fluted CMU around the new back stage addition extends to the finish floor elevation (96'-0") there is no exposed concrete wainscot at the exterior wall. Reference attached sheets A3.1 and A3.2
2. Sheet A4.2 Wall Section B – At the new Stair S1/S2 south wall, where excavating adjacent to and below the existing elevated foundation wall, the Contractor shall provide shoring as required to retain the existing foundation wall. Shoring shall remain in place for the pouring of the new south stair concrete wall. Install blind side waterproofing over shoring. Contractor submit waterproofing system compatible with the shoring system for review. Reference the attached revised sheet A4.2.
3. Sheet A4.5 and A4.6 Wall sections - Clarification, all new walls exposed to basement areas, including Stairs S1/S2 shall be waterproofed with membrane waterproofing and protection board per specifications. Reference the attached revised sheet A4.5 and A4.6.
4. Sheet A4.9 Wall Section B – The east wall of the east dance studio is to continue around the corner extending North. Reference the attached revised sheet A4.9.
5. Sheet A5.1 Roof Plan – Reference the revised roof plan sheet A5 clarifying the location for the new roof access ladder.
6. Sheet A5.1 Roof Plan – The existing portion of the roof located over Homework Area 116 and Stair S4 shall be replaced. Remove the existing roof membrane to the existing substrate, and install new TPO roof system and associated insulation cricket and cant to slope to new roof drains. Remove existing RTU as required to replace the roof membrane and reinstall.
7. Sheet A10.1 Interior Finish Schedule – Door 109 shall be an insulated metal door, Type C. Door and frame to be painted.

STRUCTURAL – Drawings

1. Reference attached revised structural drawings.

Receipt of this Addendum shall be noted on the Bid Form.

END OF ADDENDUM NO. 4

Attachments

Sheet A2.1, Sheet A3.1, Sheet A3.2, Sheet A4.2, Sheet A4.5, Sheet A4.6, Sheet A4.9, Sheet A5.1
Revised Structural drawings

TYPE A

LIQUID APPLIED AIR/VAPOR BARRIER & 5/8\" DENS GLASS SHEATHING

EIFS OVER 2 1/2\" RIGID INSUL.

1-0\"

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

5/8\" TYPE X G.B.

TYPE B

LIQUID APPLIED AIR/VAPOR BARRIER & 5/8\" DENS GLASS SHEATHING

1 1/2\" CONT. RIGID INSUL. & 2\" \"Z\" FURRING STRIPS

WOOD CLADDING (PE RAINSCREEN)

1-0 1/2\"

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

5/8\" TYPE X G.B.

TYPE C

LIQUID APPLIED AIR/VAPOR BARRIER & 5/8\" DENS GLASS SHEATHING

1 1/2\" CONT. RIGID INSUL.

1\" AIR SPACE

4\" FLUTED CMU REF. STRUCT.

1-4\"

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

5/8\" TYPE X G.B.

TYPE D

LIQUID APPLIED AIR/VAPOR BARRIER & 5/8\" DENS GLASS SHEATHING

1 1/2\" CONT. RIGID INSUL. & \"Z\" FURRING STRIPS

PREFIN. MTL WALL PANELS

1-0 1/2\"

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

5/8\" TYPE X G.B.

TYPE E

1 1/2\" RIGID INSUL. & 5/8\" DENS GLASS SHEATHING

LIQUID APPLIED AIR/VAPOR BARRIER & 5/8\" DENS GLASS SHEATHING

\"Z\" CHANNELS 24\" O.C.

PREFIN. METAL WALL PANEL SYSTEM (FLAT) INSTALL PER MANUF. REQUIREMENTS.

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

5/8\" TYPE X G.B.

TYPE F

LIQUID APPLIED AIR/VAPOR BARRIER & 5/8\" DENS GLASS SHEATHING

1 1/2\" CONT. RIGID INSUL. & \"Z\" FURRING CHANNELS 24\" O.C.

PREFIN. METAL WALL PANEL SYSTEM (FLAT) INSTALL PER MANUF. REQUIREMENTS.

1-1 1/2\"

6\" BATT INSUL.

8\" MTL STUDS 16\" O.C.

5/8\" TYPE X G.B.

ALTERNATES

#1 - INSTALL EIFS (E) THE DANCE STUDIO ADDITION FACIA LIEU OF ACM PANELS

#2 - ELIMINATING THE PAINTING THE STEEL STRUCTURE, DUCTWORK, CONDUIT, PIPING, ETC. AT THE SCENE SHOP

#3 - EIFS SYSTEM WARRANTY OF 20 YEARS

#4 - DANCE STUDIO FLOOR

2. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS & CONDITIONS. IF THERE IS A DISCREPANCY BETWEEN DRAWINGS AND FIELD CONDITIONS NOTIFY ARCHITECT PRIOR TO PROCEEDING WITH WORK SO THAT ANY ISSUES MAY BE CLARIFIED.
3. ALL WORK SHALL BE IN CONFORMANCE W/ APPLICABLE BUILDING CODES & ORDINANCES.
4. ALL NEW CONSTRUCTION SHALL BE IN CONFORMANCE TO ADA REQUIREMENTS. REFERENCE ADA FOR TYPICAL MINIMUM CLEARANCE REQ'D
5. INSTALL MATERIALS AND/OR FINISHES AS INDICATED, IMPLIED OR AS REQUIRED FOR COMPLETE & FINISHED INSTALLATION.
6. ALL NEW DOOR HARDWARE SHALL BE LEVER TYPE LATCH SETS UNLESS NOTED OTHERWISE. COORDINATE W/ MFR FOR ADA INSTALLATION REQUIREMENTS. COORDINATE KEY REQUIREMENTS WITH OWNER.
7. DOORS ARE TYPICALLY LOCATED WITH HINGE-SIDE JAMB 4" FROM ADJACENT WALL UNLESS NOTED OTHERWISE OR REQUIRED TO MEET LATCH-SIDE CLEARANCE PER ADA.
8. DIMENSIONS TO NEW INTERIOR WALLS ARE TO THE FACE OF G.B. U.N.O.
9. ALL FURNITURE SHOWN IS TO BE PROVIDED BY OWNER.
10. FEC - INDICATES NEW RECESSED FIRE EXTINGUISHER CABINET.
11. CG - INDICATES LOCATION OF FULL HEIGHT CORNER GUARDS.

#1 - INSTALL EIFS @ THE DANCE STUDIO ADDITION FACADE IN LIEU OF ACM PANELS
#2 - ELIMINATING THE PAINTING THE STEEL STRUCTURE, & DUCTWORK, CONDUIT, PIPING, ETC. AT THE SCENE SHOP ADDITION
#3 - EIFS SYSTEM WARRANTY OF 20 YEARS
#4 - DANCE STUDIO FLOOR

• ALL G.B. WALLS FULL HEIGHT TO B.O. DECK, U.N.O.

TYPE 1
 1. TYPICALLY DIMENSIONED AS 5" ON PLANS
 Components: 5/8" TYPE X G.B. EA. SIDE, 3-5/8" MTL STUDS @ 16"O.C., 3" BATT INSULATION, 4 1/2"

TYPE 2
 2. TYPICALLY DIMENSIONED AS 8" ON PLANS
 Components: 5/8" TYPE X G.B. EA. SIDE, 5/8" TYPE X M.R. G.B. AT WET SIDE, 6" MTL STUDS @ 16"O.C., 7 1/4", 6" SOUND BATT INSULATION

TYPE 3
 3. TYPICALLY DIMENSIONED AS 8" ON PLANS
 Components: 5/8" TYPE X M.R. G.B. EA. SIDE, 2-ROWS 3 5/8" MTL STUDS @ 16"O.C., VARIES (REF. PLAN), 3" SOUND BATT INSULATION

TYPE 4
 4. TYPICALLY DIMENSIONED AS 5" ON PLANS
 Components: 5/8" TYPE X G.B. EA. SIDE (ADHERED TO CONC. GUARDRAIL WALL), 6" MTL STUDS @ 16"O.C., 6", 6" BATT INSULATION, 2hr RATED

TYPE 5
 5. TYPICALLY DIMENSIONED AS 8" ON PLANS
 Components: 2 LAYERS 5/8" TYPE X G.B. EA. SIDE, 1/2" RESILIENT CHANNEL (ROOM SIDE), 8" MTL STUDS @ 16"O.C., 11", L-5 THERMAFIBER SAFB

TYPE 6
 6. TYPICALLY DIMENSIONED AS 8" ON PLANS
 Components: 2 LAYERS 5/8" TYPE X G.B. EA. SIDE, 1/2" RESILIENT CHANNEL (ROOM SIDE), 2-ROWS 2 1/2" MTL STUDS @ 16"O.C., 9", L-1 1/2" THERMAFIBER SAFB, STC RATING: 80

F1 2hr RATED TYPE F1

2hr 1" LINER PANEL
2 LAYERS 5/8" TYPE X G.B.
2 1/2" H-STUDS & 2hr LINER PANEL
FILL VOID W/ BATT INSUL.
STL COLUMN REIN. STRUCT.
ADJACENT WALL
t₀

Diagram of TYPE F1 fire-rated wall assembly. It shows a cross-section of a wall with a 2-hour rated liner panel on the exterior. The main wall consists of two layers of 5/8" Type X Gypsum Board (G.B.) separated by two 1/2" H-studs and a 2-hour liner panel. The void between the G.B. layers is filled with batt insulation. A steel column reinforcement structure is shown on the interior side. The wall is adjacent to another wall. The total thickness is labeled t₀.

Diagram of TYPE F2 fire-rated wall assembly. It shows a cross-section of a wall with a 5/8" Type X G.B. on the exterior. The main wall consists of two 1/2" MTL studs with 16" O.C. spacing, separated by 3" batt insulation. The wall is adjacent to another wall. The total thickness is labeled t₀.

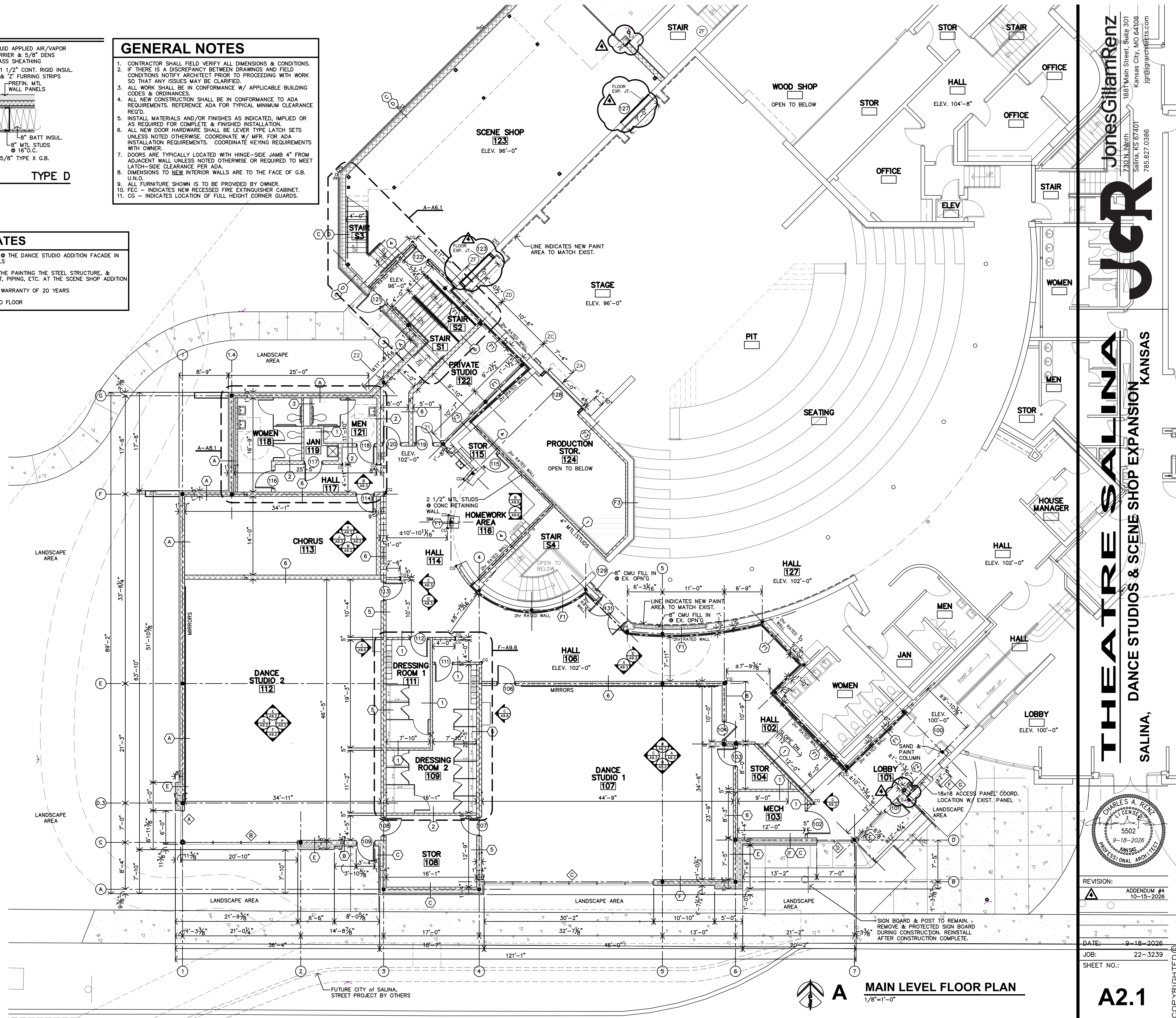
F2 TYPE F2

5/8" TYPE X G.B.
2 1/2" MTL STUDS @ 16" O.C.
3" BATT INSULATION
ADJACENT WALL
t₀

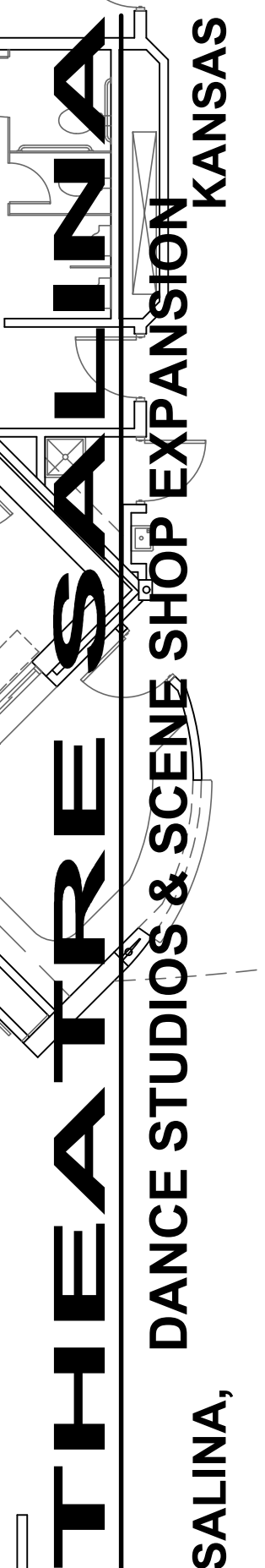
Diagram of TYPE F3 fire-rated wall assembly. It shows a cross-section of a wall with a 5/8" Type X G.B. on the exterior. The main wall consists of three 5/8" MTL studs with 16" O.C. spacing. The wall is adjacent to another wall. The total thickness is labeled t₀.

F3 TYPE F3

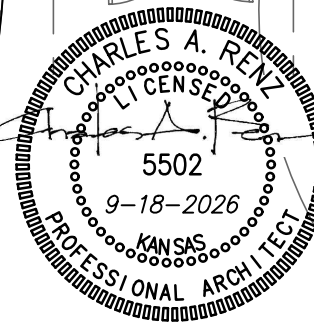
5/8" TYPE X G.B.
3 5/8" MTL STUDS @ 16" O.C.
ADJACENT WALL
t₀



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THEATRE SALINA
DANCE STUDIOS & SCENE SHOP EXPANSION
SALINA, KANSAS

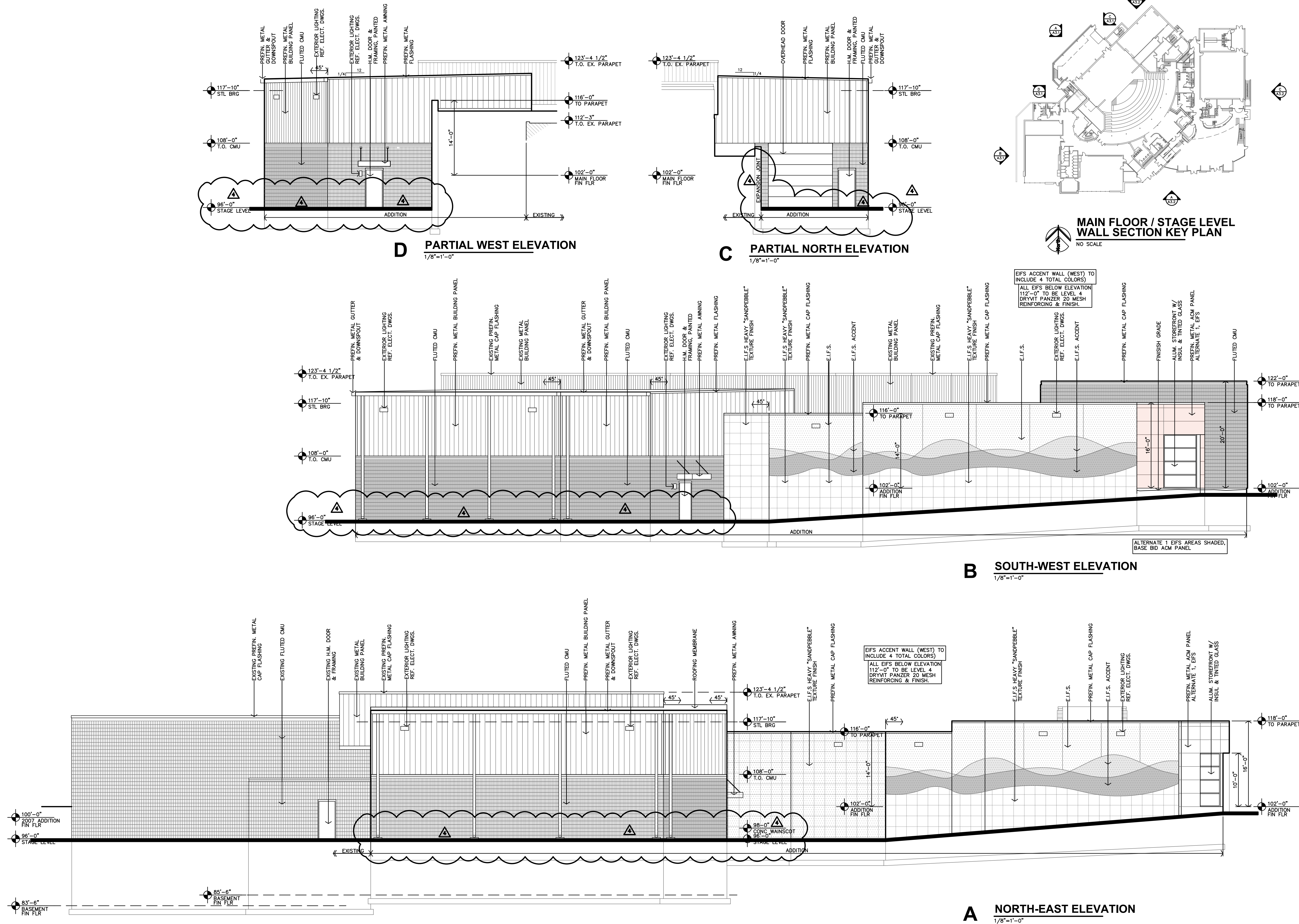


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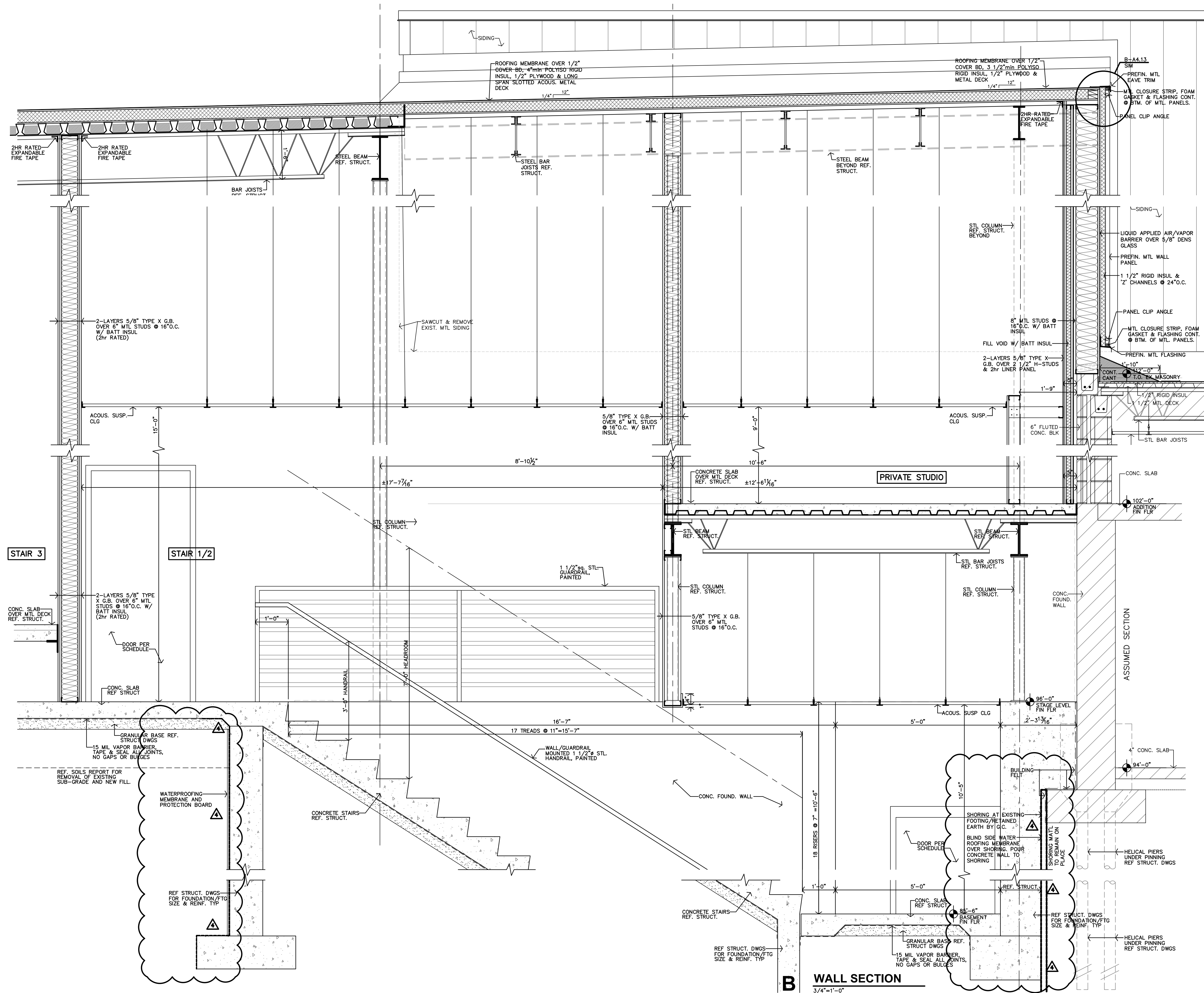
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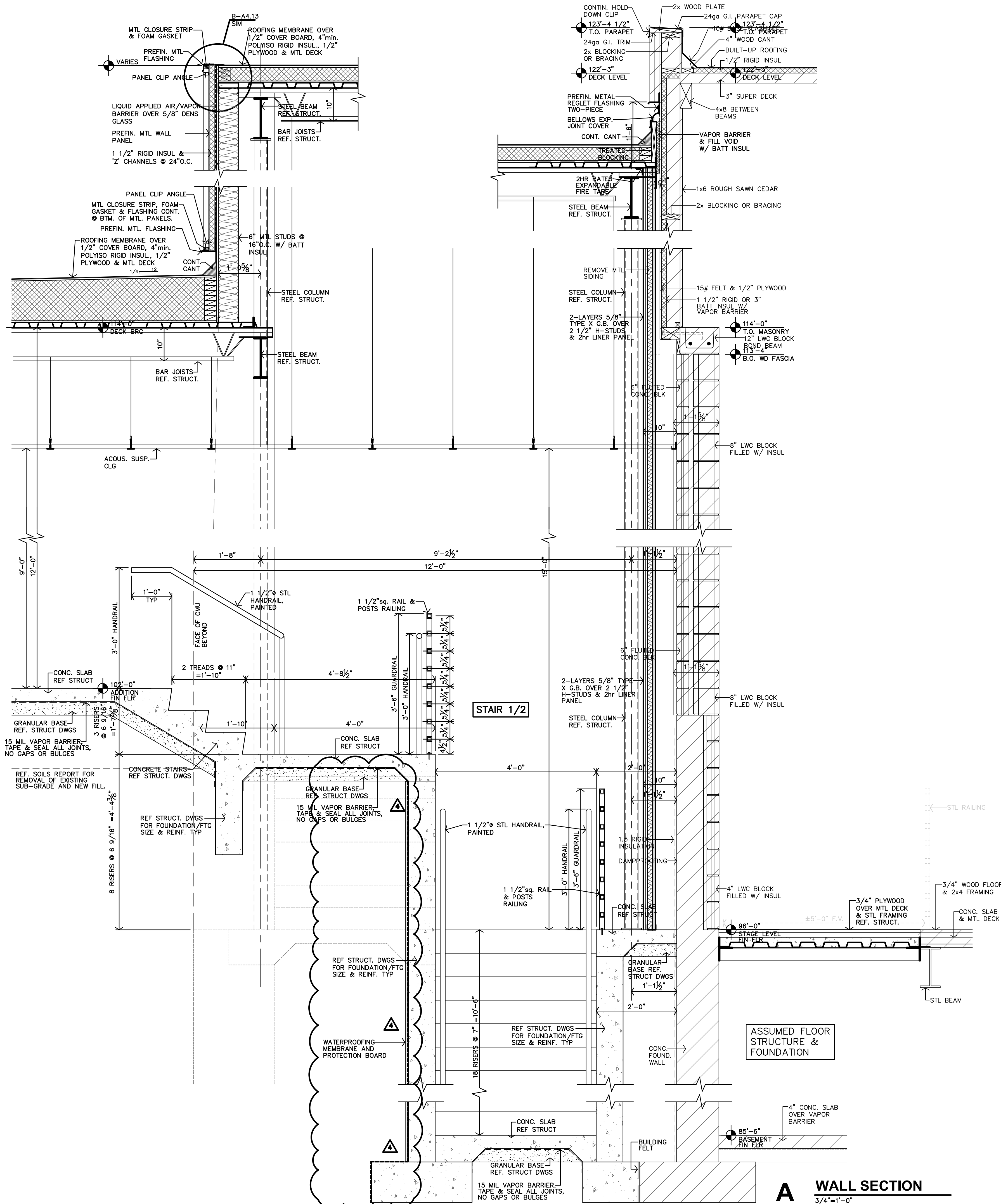
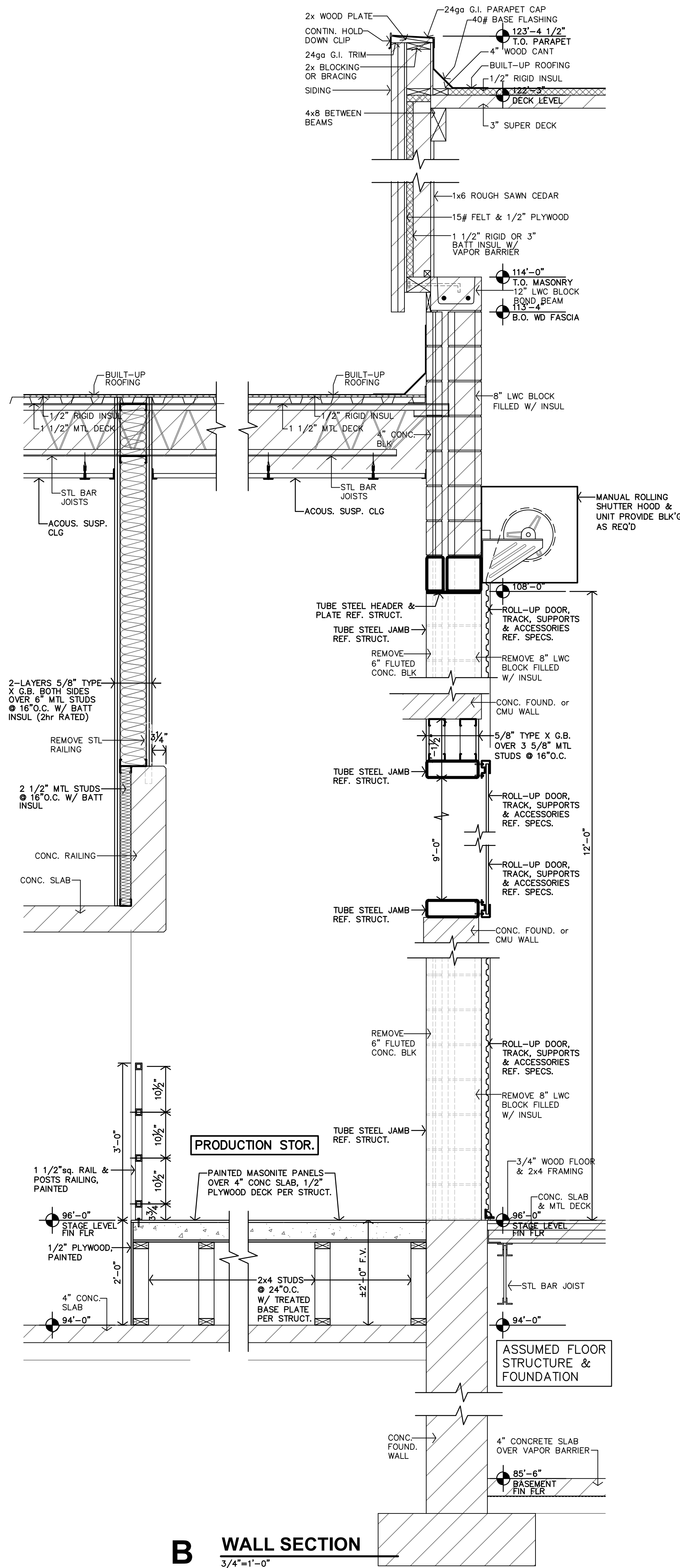
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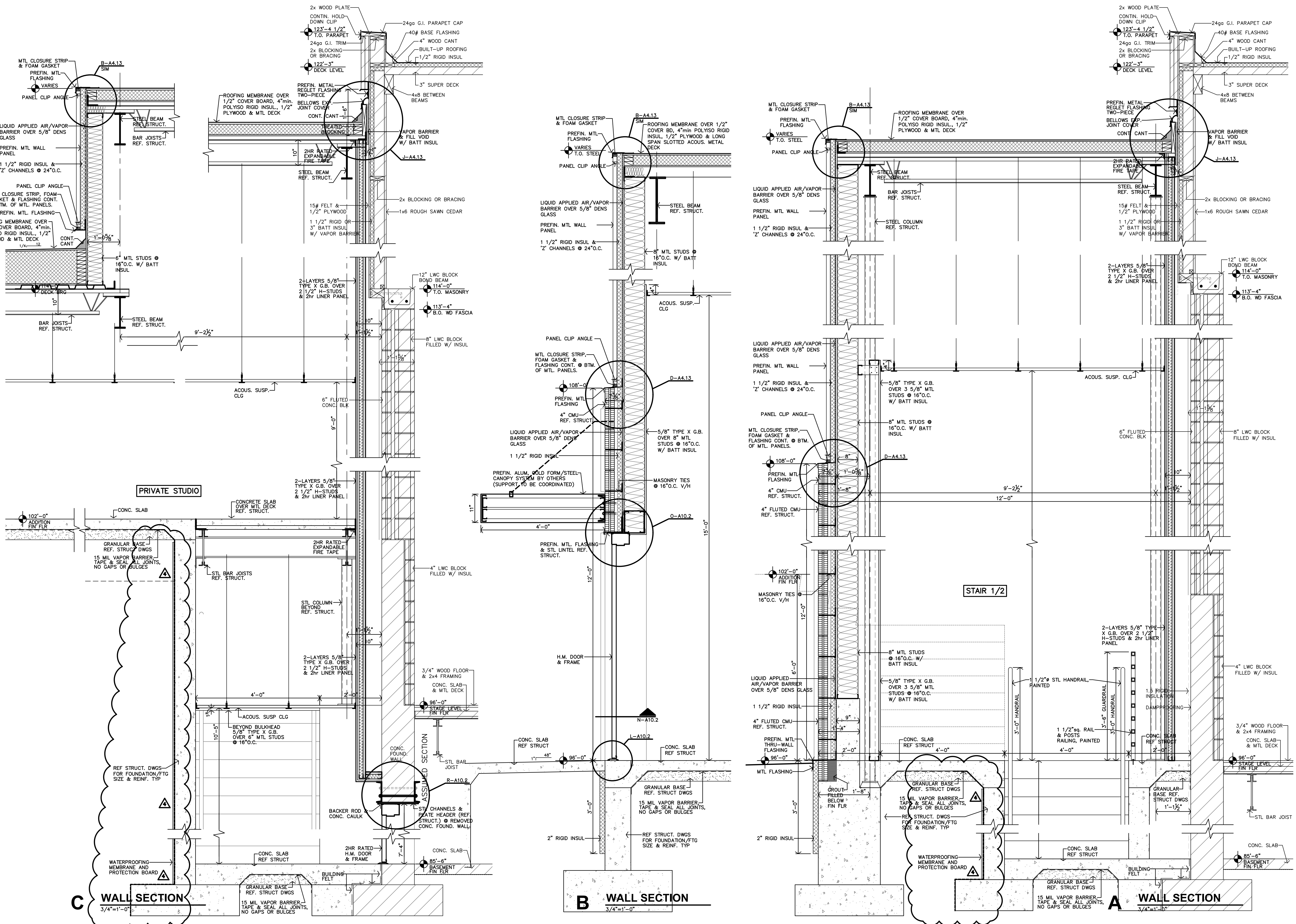
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THEATRE SALINA

DANCE STUDIOS & SCENE SHOP EXPANSION

SALINA, KANSAS

CHARLES A. RENZ
REGISTERED ARCHITECT
5502
9-18-2026
KANSAS
PROFESSIONAL ARCHITECT

REVISION:
ADDENDUM #4
10-15-2026

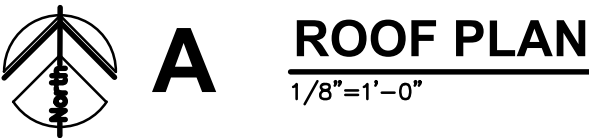
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1. ROOFING INSTALLATION: MANUFACTURERS DETAILS ARE GENERIC/GENERAL. CONTRACTOR SHALL COMPLY WITH MANUFACTURER'S DETAILS & RECOMMENDATIONS AND THOSE RECOMMENDED BY ACRA "THE ROOFING & WATERPROOFING MANUAL".
2. CONTRACTOR MUST COMPLY WITH ALL STATE & LOCAL CODES & REGULATIONS.
3. GUTTERS SHALL BE INSTALLED WITH SEAMLESS JOINTS & TRANSITIONS.
4. DIMENSIONS ON ROOF PLAN REFLECT DIMENSIONS PARALLEL WITH FLOOR PLANE.
5. ROOF SHALL BE 2" MIN. THICK POLYURETHANE INSULATION.
6. ALL METAL MATERIALS (I.E. FLASHINGS, ETC.) SHALL BE .0217" (26 GA.) THICK PREPRESSED GALVANIZED OR ALUM. ZINC ALLOY. ALL FASTENERS MUST BE COMPATIBLE WITH ASSOCIATED METALS/MATERIALS. METALS MUST BE INSTALLED PER ACRA'S "ARCHITECTURAL METALS".
7. ALL DOWNSPOUTS & ROOF DRAINS TO BE TIED TOGETHER AND CONNECTED TO STORM SYSTEM, REF. CIV. DWGS.

1 MECHANICALLY ATTACH NEW 60-MIL PVC ROOF MEMBRANE OVER EXIST. COVER BOARD, RIGID INSUL, DECKING & METAL DECK. PROVIDE ALL TERMINATION BARS, FLASHINGS, SEALANT, & CAULKING. INSTALL PARAPET METAL FLASHING CAPS & PREFIN. MTL GUTTER/FLASHING. 20-YEAR FULL COVERAGE WARRANTY.



1. General Information:

- A. The contractor shall verify dimensions and conditions before construction and notify the engineer of any discrepancies, inconsistencies, or difficulties affecting the work before proceeding.
- B. The contractor shall coordinate all disciplines, verifying size and location of all openings, whether shown on structural drawings or not, as called for on architectural, mechanical, or electrical drawings. In the case of work in an existing building the contractor shall scan existing structure to locate all rebar in the area of the new core/opening using ground penetrating radar and notify the engineer of record for review prior to core/cutting. Conflicts, inconsistencies, or other difficulties affecting structural work shall be called to the architect or engineer's attention for direction before proceeding.
- C. All design and construction work for this project shall conform to the requirements of the 20XX International Building Code, as amended by the City of Salina, Kansas.
- D. These drawings are for this specific project and no other use is authorized.

2. Structural Design Load Criteria:

- A. Floor Live= 100psf
B. Roof Live= 20psf
C. Snow= Pg= 20psf, Pf=15.4psf, Is= 1, Gs=1.0, C1=1.1, Drift per ASCE/SEI T-16
D. Lateral Loads:
1) Wind Vel=10 mph, Exposure C
Occupancy [Risk] Category II, Iw=1.0
GcPf=+/-0.18
Design wind pressures to be used for the design of exterior component and cladding materials on the designated zones of wall and roof surfaces shall be per section 30.7.1 and Table 30.7-2 of ASCE/SEI T-16. Tabulated pressures shall be multiplied by effective area reduction factors, exposure adjustment factors, and topographic factors where applicable.
- 2) Seismic: S_s = 0.076, S_i = 0.048
Occupancy [Risk] Category II, Ie=1.0
Site Classification B, S_{ds}=0.026, S_{d1}=0.077
Seismic Design Category B
Basic Seismic Force-resisting System:
H. Steel System not Specifically Detailed for Seismic Resistance
Equivalent Lateral Force Procedure
(R=3, V=_____, Omega=3, Cd=3)
- E. This project is designed to resist the most critical effects resulting from the load combinations of section 1605.3 of the 2018 International Building Code.

3. Concrete:

- A. All concrete for foundations (walls, grade beams, footings and piers) shall develop minimum ultimate compressive design strength of 3500 psi in 28 days, but not less than 500 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 6 gallons of water per 100 pounds of cement and not over 4 inches of slump.
- B. All concrete for interior slabwork shall develop minimum ultimate compressive design strength of 4000 psi in 28 days, but not less than 540 pounds of cement shall be used per cubic yard of concrete regardless of strengths obtained, not over 540 gallons of water per 100 pounds of cement and not over 4 inches of slump. Concrete mix shall contain testing data proving concrete design mix shrinkage is less than 0.034% at 28 days when tested according to ASTM C157 (air drying method only).
- C. All concrete for exterior slabwork shall have a minimum design compressive strength of 4500 psi in 28 days, but not less than 560 pounds of cement per cubic yard concrete, not over 5 gallons of water per 100 pounds of cement, with 6% +/- 1% air entrained, and a maximum of 4 inches of slump.
- D. The preceding minimum mix requirements may have water-reducing admixtures conforming to ASTM C494 added to the mix at manufacturer's dosage rates for improved workability.
- E. The preceding minimum mix requirements may have up to 15% maximum of the cement content, replaced with an approved ASTM C618 Class C fly ash, provided the total minimum cementitious content is not reduced.
- F. All interior concrete slabs on grade shall be placed over 15 mil, Class A Vapor Barrier per ASTM E1145 with less than 0.01 perms, tested after mandatory conditioning. All joints shall be lapped and sealed per manufacturer's recommendations. All penetrations, as well as damaged vapor barrier material shall also be sealed per manufacturer's recommendation prior to concrete placement. Install barrier per manufacturer recommended details at all discontinuous edges (at interior columns, exterior edge of slab, etc.) to ensure terms of warranty are followed. The vapor barrier shall be placed over free-draining granular material as prescribed by the project soils report.
- G. Basement foundation walls shall be braced at the base and top of wall by the contractor until the slab on grade at the base and the floor framing/slab at the top of wall is complete and the concrete has achieved 75% of the design strength. The contractor is responsible for engineering and design of the wall bracing, if required.
- H. All concrete is reinforced concrete unless specifically called out as unreinforced. Reinforce all concrete not otherwise shown with same steel as in similar sections or areas. Any details not shown shall be detailed per ACI 318 and meet requirements of ACI 318, current editions.
- I. Control joints in dirt formed slab to be as shown on plans. Where not shown, limit controlled areas to not more than 144 square feet, or 12 feet on any side. Slab panel side ratio shall not exceed 1 1/2 to 1.
- J. Contractor shall verify that all concrete inserts, reinforcing and embedded items are correctly located and rigidly secured prior to concrete placement.
- K. Construction joints in beams, slabs, and grade beams shall occur at midspan (middle third) unless noted otherwise. Provide 2 x 4 horizontal keys at construction joints for shear transfer.
- L. No aluminum items shall be embedded in any concrete.

4. Reinforcing Steel:

- A. All reinforcing steel shall conform to the requirements of ASTM A615 or A706 grade 60 steel. Welded plain wire fabric shall be supplied in sheets and conform to the requirements of ASTM A1064.
- B. Clear coverage of concrete over reinforcing steel shall be as follows:
Concrete placed against earth 3"
Slabs 1"
Beams or Columns 1 1/2"
Other 2"
All coverage shall be nominal bar diameter minimum.
- C. All details shall be the same size and spacing as adjoining main bars (splice up 48 bar diameters or 24" minimum unless noted otherwise).
- D. At corners of all walls, beams, and grade beams supply corner bars (minimum 2'-0" in each direction or 48 bar diameters) in outside face of wall, matching size and spacing of horizontal bars. Where there are no vertical bars in outside face of wall, supply 3 - #4 vertical support bars for corner bars.
- E. Bars marked continuous shall be lapped and embedded, unless shown otherwise. Splice top bars near midspan and splice bottom bars over supports, unless noted otherwise.
- F. At all holes in concrete walls and slabs, add 2 - #5 bars (opening dimension plus 16 diameters long) at each of four sides and add 2 - #5 x 5'-0" diagonally at each of four corners of hole. Openings in 8" thick walls are reinforced similar, but with 1 - #5 instead of 2 - #5, respectively.
- G. Unless otherwise covered on architectural plans or specifications, vertical control joints in concrete wall shall be spaced at a maximum of 20'-0" on center and coordinated with the architect. Every other horizontal wall reinforcing bar shall be discontinuous at control joints except heavy top and bottom bars unless noted otherwise. Provide base seal waterstop style number 712 (by Greenstreak Inc. or approved equal) on dist face side of wall at all walls below grade.
- H. Accessories shall be as specified in latest edition of the ACI Detailing Handbook and the concrete Reinforcing Steel Institute Design Handbook. Maximum accessory spacing shall be 4'-0" on center, and all accessories on exposed surfaces are to have plastic coated feet.
- I. All slabs and stairs not shown otherwise shall be 6" thick with #4 bars at 12" on center each way. All exterior porches and stoops not otherwise detailed may be constructed in any standard manner, solid or hollow, but must be reinforced with #4 bars at 12" on center each way minimum. Porches shall be dovealed to adjacent walls or grade beams with #4 bars at 12" on center, hooked or embedded 48 diameters into both members. Slope porches 1/8" per foot for drainage unless noted otherwise.
- J. Allow 1/2 ton of reinforcing bars #4 or larger to be used as directed in the field for special conditions by the engineer of record (labor for placing same to be included).

5. Structural Steel:

- A. All structural steel beams and columns shall be ASTM A992, grade 50 steel and all miscellaneous steel shall be ASTM A36 grade steel (except at moment connections where plates shall be ASTM A572, grade 50). Hollow Structural Sections (HSS) shall be ASTM A500, grade C. Fabrication and erection shall be in accordance with AISC 308-05 "Code of Standard Practice for Steel Buildings and Bridges" in the 13th Edition of the AISC Steel Construction Manual.
- B. All welding shall conform to the recommendations of the AWS.
- C. All exterior steel and connections, and brick relief angles shall be hot-dip galvanized.
- D. All bolts not otherwise specified shall be 3/4" diameter high strength (ASTM A325-N). All bolts shall be fully pretensioned. All span connections shall be designed per the AISC Steel Construction Manual "Framed Beam Connections" for the indicated reactions or at least 0.4 x beam total shear capacity, Vn/Omega, shown in the maximum total uniform load tables, whichever is greater; and, shall account for eccentricity when the bolt line is more than 24" from the center of the support. All connections must be two bolt minimum. Additional connection elements may not be specifically shown in the conceptual details in this set but may be required by the final connection design, such as stiffener plates, doubler plates, supplement/reinforcing plates or other connection material. Connection design and shop drawing preparation shall be completed under the direct supervision of a professional engineer licensed in the state the project is located and shop drawings and connection calculations shall be submitted to the building official for review and approval. Design calculations for deferred submittals shall be submitted at the same time as the shop drawings for review. Design calculations shall be prepared and sealed by a Professional Engineer licensed in the state of the project. The deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official.
- E. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall:
- 1) Review each submittal for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.
 - 2) Revisions and approve each submittal.
 - 3) Stamp each submittal as approved.
- F. Bob D. Campbell and Company, Inc. shall assume that no submittal comprises a variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation.
- G. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC.
- 1) Concrete mix designs and material certificates including admixtures and compounds applied to the concrete after placement.
 - 2) Reinforcing steel shop drawings including erection drawings and bending details. Bar list will not be reviewed for correct quantities.
 - 3) Grout mix designs (for CMU).
 - 4) Construction and control joint plans and/or elevations.
 - 5) Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit filler joist details on non-structural drawings for Bob D. Campbell and Company, Inc. review.
 - 6) Deferred Submittal: Structural steel connection design calculations submitted concurrently with structural steel shop drawings.
 - 7) Miscellaneous anchors shown on the structural drawings.
 - 8) Standard details and bridging information for light gage metal framing. Erection details for details for light gage metal joists and lintels spanning more than 6'-0" shall be submitted. Standard wall framing need not be submitted.
 - 9) Deferred Submittal: Cold-Formed metal framing for exterior walls.
 - 10) Deferred Submittal: Railings and guardrails.
 - 11) Deferred Submittal: Metal stair framing.
- H. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material or submissions without GC approval stamp.

6. Post-installed Anchors:

- A. Post-installed anchors shall be used only where specified on the drawings unless approved in writing by the engineer of record. See drawings for anchor diameter, spacing and embedment. Performance values of the anchors shall be obtained for specified products using appropriate design procedures and/or standards as required by the governing building code. Anchors installed in concrete shall have an ICC-ES Evaluator's Report. Special inspection is required for all post-installed anchors. The general contractor shall coordinate an on-site meeting with the post-installed anchor manufacturer field representative and subcontractor performing the anchor installation to educate the construction team on the anchor installation guidelines and requirements. The contractor shall send a record copy of the meeting meetings to the design team.
- B. Mechanical anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ACI 308.2 and ICC-ES AC108. All anchors shall be installed per the anchor manufacturer's written instructions.
- C. Adhesive anchors used in cracked and uncracked concrete shall have been tested and qualified for use in accordance with ICC-ES AC308. All anchors shall be installed per the anchor manufacturer's written instructions.
- D. Mechanical anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC01. All anchors shall be installed per the anchor manufacturer's written instructions.
- E. Adhesive anchors used in solid grouted masonry shall have been tested and qualified for use in accordance with ICC-ES AC508. All anchors shall be installed per the anchor manufacturer's written instructions.
- F. Anchors used in hollow concrete masonry shall have been tested and qualified in accordance with ICC-ES AC106 or ICC-ES AC58 as appropriate. All anchors shall be installed per the anchor manufacturer's written instructions with appropriate screen tubes used for adhesives.

7. Foundations:

- A. The soil investigation was prepared by UES, the report number is A26125.00474.000 and their telephone number is 316-554-0725.
- B. Spread footings, grade beams, thickened slabs, and retaining walls are designed to bear on an engineered system of rammed aggregate piers spaced at a maximum of 10'-0" on center and capable of safely sustaining 6,000 psf and limiting overall settlement to 1" maximum with differential settlement of 1/2" maximum.
- C. Retaining structures are designed for a lateral load of 52 psf equivalent fluid pressure and an at-rest lateral load of 73 psf.
- D. Contractor shall provide for dewatering at excavations from either surface water or seepage.
- E. All foundation excavations shall be inspected by a qualified soil engineer, approved by the architect and/or structural engineer, prior to placement of steel or concrete. This inspection shall be at the owner's expense.
- F. All concrete in the structural portion retaining the backfill shall have attained its design strength prior to being backfilled.
- G. Moisture content in soils beneath building locations should not be allowed to change after footing excavations and after grading for slabs on grade are completed. If subgrade materials become desiccated or softened by water or other conditions, recompact materials to the density and water content specified for engineered fill. Do not place concrete on frozen ground.
- H. Provide helical piers where indicated on plan capable of safely sustaining the loads indicated on the foundation plan. The helical pier designer shall utilize the geotech to determine the quantity and length of the piers.

8. Light Gage Structural Stud Framing:

- A. All load bearing, light gage structural studs, track, and bridging shall be of the type, size, gage, and spacing as shown on the plans, minimum.
- B. All materials shall be 33,000 psi minimum yield, except studs of 16 gage or heavier shall have a minimum yield of 50,000 psi.
- C. All properties, fabrication, and erection shall be in accordance with latest editions of the AISI "Specifications for the Design of Cold-Formed Structural Members".
- D. All framing components shall be cut squarely and at an angle to fit squarely against abutting members. Splicing of axially loaded members is not permitted. Members shall be held firmly in place until properly fastened. Attachments of similar components shall be by welding, screw attachment, or bolting. Wire tying of components is not permitted.

- E. Tracks shall be securely anchored to floor and overhead members. Special anchorage requirements required for wind bracing shall be as shown on the plans.

9. Shop Drawing Review:

- A. Bob D. Campbell and Company, Inc. will review the General Contractor's (GC) shop drawings and related submittals (as indicated below) with respect to the ability of the detailed work, when complete, to be a properly functioning integral element of the overall structural system designed by Bob D. Campbell and Company, Inc. Deferred submittals shall be submitted to the architect of record for review who shall forward to the building official for review and approval. Design calculations for deferred submittals shall be submitted at the same time as the shop drawings for review. Design calculations shall be prepared and sealed by a Professional Engineer licensed in the state of the project. The deferred submittal items shall not be installed until the deferred submittal documents have been approved by the building official.
- B. Prior to submittal of a shop drawing or any related material to Bob D. Campbell and Company, Inc., the GC shall:
- 1) Review each submittal for conformance with the means, methods, techniques, sequences and operations of construction and safety precautions and programs incidental thereto, all of which are the sole responsibility of the GC.
 - 2) Revisions and approve each submittal.
 - 3) Stamp each submittal as approved.
- C. Bob D. Campbell and Company, Inc. shall assume that no submittal comprises a variation unless the GC advises Bob D. Campbell and Company, Inc. with written documentation.
- D. Shop drawings and related material (if any) required are indicated below. Should Bob D. Campbell and Company, Inc. require more than ten (10) working days to perform the review, Bob D. Campbell and Company, Inc. shall so notify the GC.
- 1) Concrete mix designs and material certificates including admixtures and compounds applied to the concrete after placement.
 - 2) Reinforcing steel shop drawings including erection drawings and bending details. Bar list will not be reviewed for correct quantities.
 - 3) Grout mix designs (for CMU).
 - 4) Construction and control joint plans and/or elevations.
 - 5) Structural steel shop drawings including erection drawings and piece details. Include joist, decking and connector submittals. Include miscellaneous framing specified on the structural drawings, but do not submit filler joist details on non-structural drawings for Bob D. Campbell and Company, Inc. review.
 - 6) Deferred Submittal: Structural steel connection design calculations submitted concurrently with structural steel shop drawings.
 - 7) Miscellaneous anchors shown on the structural drawings.
 - 8) Standard details and bridging information for light gage metal framing. Erection details for details for light gage metal joists and lintels spanning more than 6'-0" shall be submitted. Standard wall framing need not be submitted.
 - 9) Deferred Submittal: Cold-Formed metal framing for exterior walls.
 - 10) Deferred Submittal: Railings and guardrails.
 - 11) Deferred Submittal: Metal stair framing.
- E. Bob D. Campbell and Company, Inc. shall review shop drawings and related materials with comments provided that each submission has met the above requirements. Bob D. Campbell and Company, Inc. shall return without comment unrequired material or submissions without GC approval stamp.

10. Statement of Structural Special Inspection:

- A. The structural design for this project is based on completion of special inspections during construction in accordance with section 1704 of the 2012 International Building Code. The owner shall employ one or more qualified special inspectors to provide the required special inspections.
- B. The following inspections and tests are required with the frequency (continuous or periodic) as defined within the referenced section or standard listed below. The General Contractor shall provide notification to the inspector when items requiring inspection are ready to be inspected and provide access for those inspections.
- 1) Shop Fabrication - structural steel and steel bar joist per Section 1705.1 and AISC 341.
 - 2) Steel Construction per Section 1705.2 and the quality assurance requirements of AISC 341 chapter J (as referenced by AISC 360).
 - 3) Cold-Formed Steel Deck per Section 1705.2.2 and the quality assurance requirements of SDI GA/VAC.
 - 4) Concrete Construction per Section 1705.3 and Table 1705.3
a. Reinforcing Steel Placement
b. Reinforcing Steel Welding
c. Cast in Place Anchors
d. Post Installed Anchors
e. Design Mix Verification
f. Concrete Sampling and Testing
g. Concrete Placement
h. Concrete Curing
i. Formwork Shape, Location and Dimensions
 - 5) Verification of Soils per Table 1705.6
 - 6) Inspections and Tests of Driven Deep Foundation per Table 1705.7
- C. The special inspector shall submit inspection reports to the building official, specifying the location and description of any other designated person.
- D. All discrepancies shall be brought to the immediate attention of the contractor for correction, then, if uncorrected, to the proper design authority, building official and structural engineer.
- E. The special inspector shall submit a final signed report stating that the work requiring special inspection was, to the best of the inspector's knowledge, in conformance with the approved plans and specifications and the applicable workmanship provisions of the building code.

11. Copyright and Disclaimer:

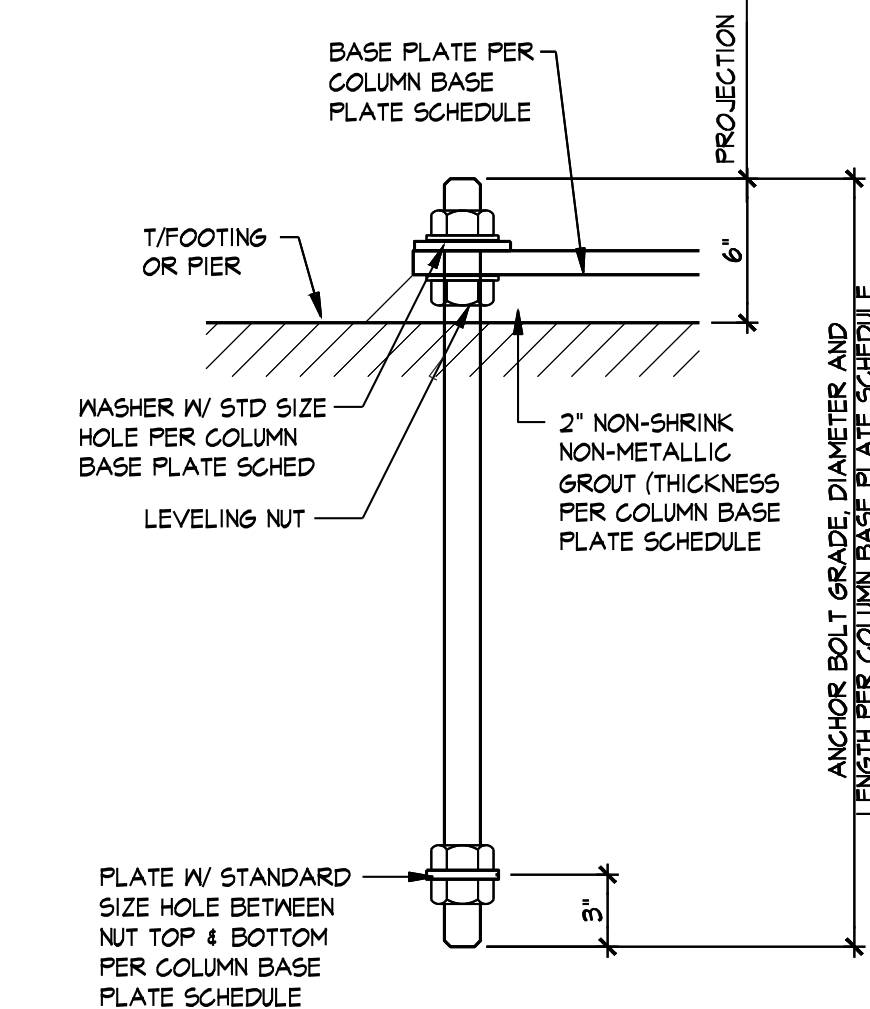
- A. All drawings in the structural set (5-series drawings) are the copyrighted work of Bob D. Campbell and Company, Inc. These drawings may not be photographed, traced, or copies in any manner without the written permission of Bob D. Campbell and Company, Inc. Exception: Original drawings may be printed for distribution to the owner, architect, and general contractor for coordination, bidding, and construction. Subcontractors may not reproduce these drawings for any purpose or in any manner.
- B. I, Christopher A. Bevin, P.E., registered engineer and a representative of Bob D. Campbell and Company, Inc. do hereby accept professional responsibility as required by the professional registration laws of this state for the structural design drawings consisting of 5-series drawings. I hereby disclaim responsibility for all other drawings in the construction document package, they being the responsibility of other design professionals whose seals and signature statements may appear elsewhere in the construction document package.

LEGEND:

- SPAN DIRECTION OF DECK AND DECK/SLAB TYPE PER SCHED. ON SO-1
- HSS 6x6x1/4 COLUMN SIZE
- BASE PLATE MARK - SEE SCHEDULE ON THIS SHEET
- FOOTINGS MARK PER SCHED ON THIS SHEET

- MOMENT CONNECTION TO DEVELOP 50% OF MOMENT CAPACITY OF BEAM AND 40% OF SHEAR CAPACITY

- M2X44 CONT. (25) c=1"
- CAMBER
OF 3/4" DIA x 5'Lg STUDS
BEAM CONTINUOUS OVER COLUMN (WHERE INDICATED)
BEAM SIZE



TYPICAL ANCHOR BOLT DETAIL
SECTION I
3/4\"/>

CONCRETE FOOTING SCHEDULE			
SOIL PRESSURE :		CONCRETE F _c :	REBAR F _y :
1500 psf		3500 psi	60,000 psi
TYPE	FOOTING SIZE (FT) x THICKNESS (IN)	REBAR SIZE & SPACING EA. MAX ³	
2P	2'-0" x 5'-0" x 36" PILE CAP	REFER TO 12/53.0	
3	3'-0" x 3'-0" x 18"	#4 @ 6" OC BOTTOM	
3A	3'-0" x 3'-0" x 32"	#4 @ 6" OC TOP & BOTTOM	
4A	4'-0" x 4'-0" x 32"	#4 @ 6" OC TOP & BOTTOM	
4	4'-0" x 4'-0" x 18"	#4 @ 6" OC BOTTOM	
5	5'-0" x 5'-0" x 28"	#4 @ 6" OC BOTTOM	
5P	5'-0" x 5'-0" x 36" PILE CAP	REFER TO 11/53.0	

NOTES:

1. EXTERIOR FOOTINGS OR FOOTING AT GRADE BEAM SHALL MATCH GRADE BEAM DEPTH AND BE PLACED WITH GRADE BEAM. PROVIDE SPECIFIED REBAR TOP AND BOTTOM WITH 4 STANDEES TO SUPPORT MATS.
2. CENTER FOOTINGS ON COLUMNS AND/OR WALL CENTER LINES PER PLAN, UNLESS OTHERWISE NOTED.
3. FOOTING REINFORCEMENT TO BE PLACED @ BOTTOM OF FOOTING UNO. IN SCHED.

AT	FS	FAR SIDE	PERP	PERPENDICULAR
AND, DIAMETER	FT6	FOOTING	PL	PLATE
ADTL	FT6	FIELD VERIFY	FL	FOUNDS PER LINEAR FOOT
ARCH	FV	FAR SIDE	PJP	PARTIAL JOINT PENETRATION
BLDG	FT6	FOOTING	PSF	POUNDS PER SQUARE FOOT
B/	FA	FIELD VERIFY	PSI	POUNDS PER SQUARE INCH
BOTT	GALV	GALVANIZE(D)	QTY	QUANTITY
BRG	GEN	GENERAL	RAD	RADIUS
C	GR	GRADE	RD-#	ROOF DECK TYPE
CD-#	HORIZ	HORIZONTAL	REF	REFERENCE
CJ	HORIZ	HOLLOW STRUCTURAL SECTION	REIN	REINFORCEMENT
CL	INT	INTERIOR	REQD	REQUIRED
CMU	JST	JOIST	REV	REVISION
COL	WT	WELD	RL	ROOF LIVE LOAD
CONC	INT	INTERIOR	RTU	ROOF TOP UNIT
CONN	JOIST	JOIST	SC	SLIP CRITICAL
CONT	WT	WELD	SC	SCHEDULE
COORD	WT	WELD	SCHED	SCHEDULE
COV, CVR	WT	WELD	SECT	SECTION
DBL	KSF	KIPS PER SQUARE FOOT	SHT	SHEET
DET	KSI	KIPS PER SQUARE INCH	SIM	SIMILAR
DET	LBS, #	POUNDS	SN	SNOW JOINT
DIM	LL	LIVE LOAD	SNL	SNOW LOAD
DL	LLH	LONG LEG HORIZONTAL	SOG	SLAB-ON-GRADE
DWG	LLV	LONG LEG VERTICAL	SOG-#	SLAB-ON-GRADE TYPE
EA	LLV	LONG LEG VERTICAL	SP6C	SPACING
EAC	LONG	LONGITUDINAL	SPEC	SPECIFICATION
EAC	SLT	LONG-SLOTTED HOLE TRANSVERSE	SPT	SUPPORT
EF	MTWT	MOMENT	SQ	SQUARE
EL ELEV	MTWT	MOMENT	S6	STAINLESS STEEL
EMBED	MAX	MAXIMUM	SSLT	SHORT-SLOTTED HOLE TRANSVERSE
ENGR	MECH	MECHANICAL	STD	STANDARD
ENG	MEGR	MANUFACTURER	STIFF	STIFFENER
EOR	MIN	MINIMUM	STIR	STIRRUP
EOS	MISC	MISCELLANEOUS	STL	STEEL
EQ	MSRY	MASONRY	STRUCT	STRUCTURE, STRUCTURAL
EQUIP	MTL	METAL	T	TOP OF
EXP	NEAR	NEAR FACE	THRU	THROUGH
EXT	NEAR	NEAR SIDE	TOP	TOP OF STEEL, TOP OF SLAB
EXT, EXIST	NTS	NOT TO SCALE	TRANS	TRANSVERSE
FD-#	NN	NORMAL WEIGHT	TRS	TRANSVERSE
FDN	OC	ON CENTER	TYP	TYPICAL
FF	OC	ON CENTER	UNO	UNLESS NOTED OTHERWISE
FIN	OPNG	OPENING	V	SHEAR FORCE
FLR	OPP	OPPOSITE	VERT	VERTICAL
	OVVS	OVERSIZED HOLE	V	VERTICAL
	PAF	AXIAL FORCE	N/O	WITHOUT
	PC	POWDER-ACTUATED FASTENER	NF	NICE FLANGE
	PCF	PRECAST	NL	NOMINAL LOAD
	PMB	POUNDS PER CUBIC FOOT	NP	NOE POINT
		PRE-ENGINEERED METAL BUILDING	WPF	WELDED WIRE FABRIC

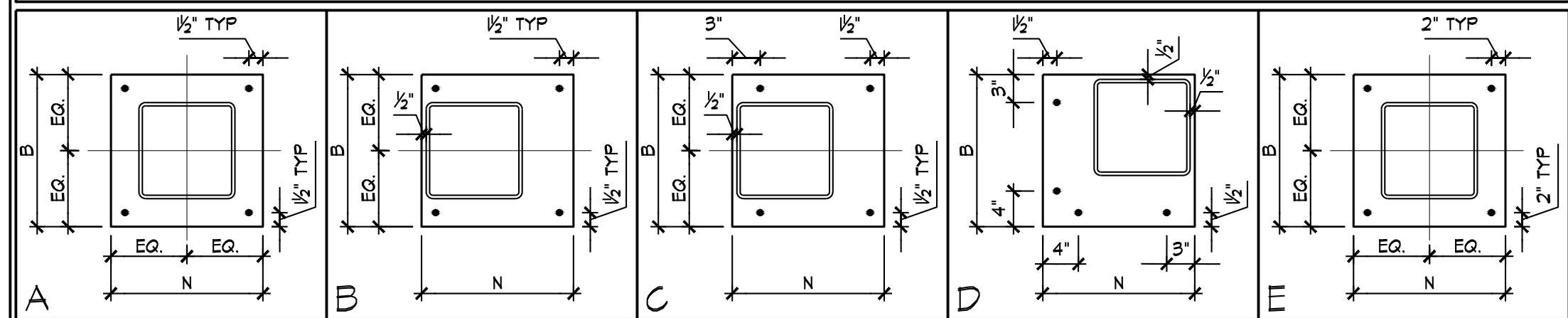
COLUMN BASE PLATE SCHEDULE

TYPE	COLUMN	BASE PLATE (LxBxN)	SHAPE	ANCHOR RODS	EMBEDMENT
1	PER PLAN	3/4\"/>	A	(4) 3/4\"/>	15"
2	PER PLAN	3/4\"/>	B	(4) 3/4\"/>	15"
3	PER PLAN	3/4\"/>	A	(4) 3/4\"/>	15"
4	PER PLAN	1 1/2\"/>	A/54.1	(6) 1\"/>	24"
5	PER PLAN	1\"/>	B	(4) 3/4\"/>	15"

NOTES:

1. SEE PLAN FOR ORIENTATION OF COLUMNS.
2. PROVIDE PLATE WASHER & EMBEDDED PLATE PER SCHEDULE @ ALL ANCHOR BOLTS.
3. UNO. ALL THREADED ROD A.B.'S SHALL BE F1554 (36ksi) MATERIAL.

BASE PLATE SHAPE (NOT TO SCALE)



STRUCTURAL DECK & SLAB SCHEDULE

MARK	DESCRIPTION
SOG-1	4\"/>
SOG-2	4\"/>
CD-1	5\"/>
CD-2	4\"/>
RD-1	1/2\"/>
RD-2	3/8\"/>

NOTES:

1. SOG = SLAB-ON-GRADE TYP.
2. CD = CONCRETE SLAB & DECK TYP.
3. RD = ROOF DECK TYP.

ANCHOR-ROD CRITERIA

ANCHOR-ROD DIAMETER	MIN. PLATE WASHER HOLE DIAMETER	MIN. PLATE WASHER SIZE	MIN. PLATE WASHER THICKNESS	EMBEDDED ANCHOR PLATE SIZE
3/4\"/>	1 1/8\"/>	2\"/>	1/4\"/>	1/2\"/>
1\"/>	1 1/8\"/>	3\"/>	3/8\"/>	5/8\"/>

NOTES:

1. HOLE SIZES PROVIDED ARE BASED ON ANCHOR ROD SIZE AND CORRELATE WITH ACI 117 (ACI, 2010)
2. CIRCULAR OR SQUARE WASHERS MEETING THE WASHER SIZE ARE ACCEPTABLE.
3. HOLE IN PLATE WASHER SHALL BE 1/16\"/>
4. WELD PLATE WASHER TO BASEPLATE AT BASEPLATE TYPE B WITH 1/8\"/>

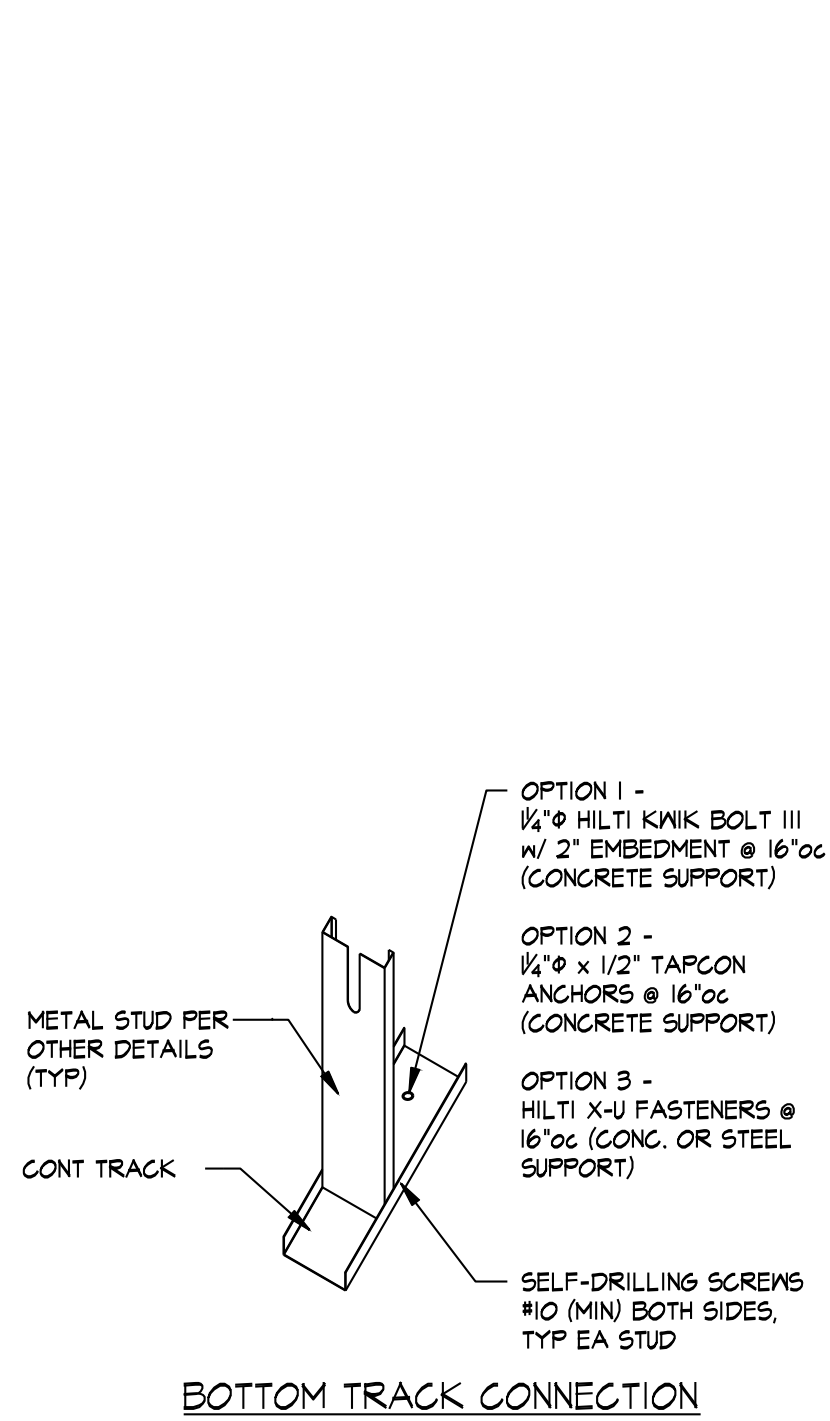
REVISION:

Addendum 1 10/15/26

DATE: 9-18-2026

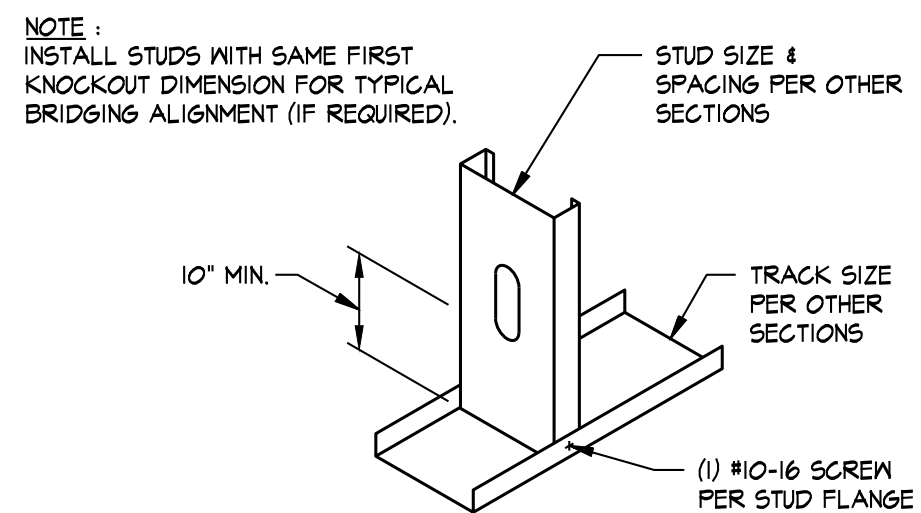
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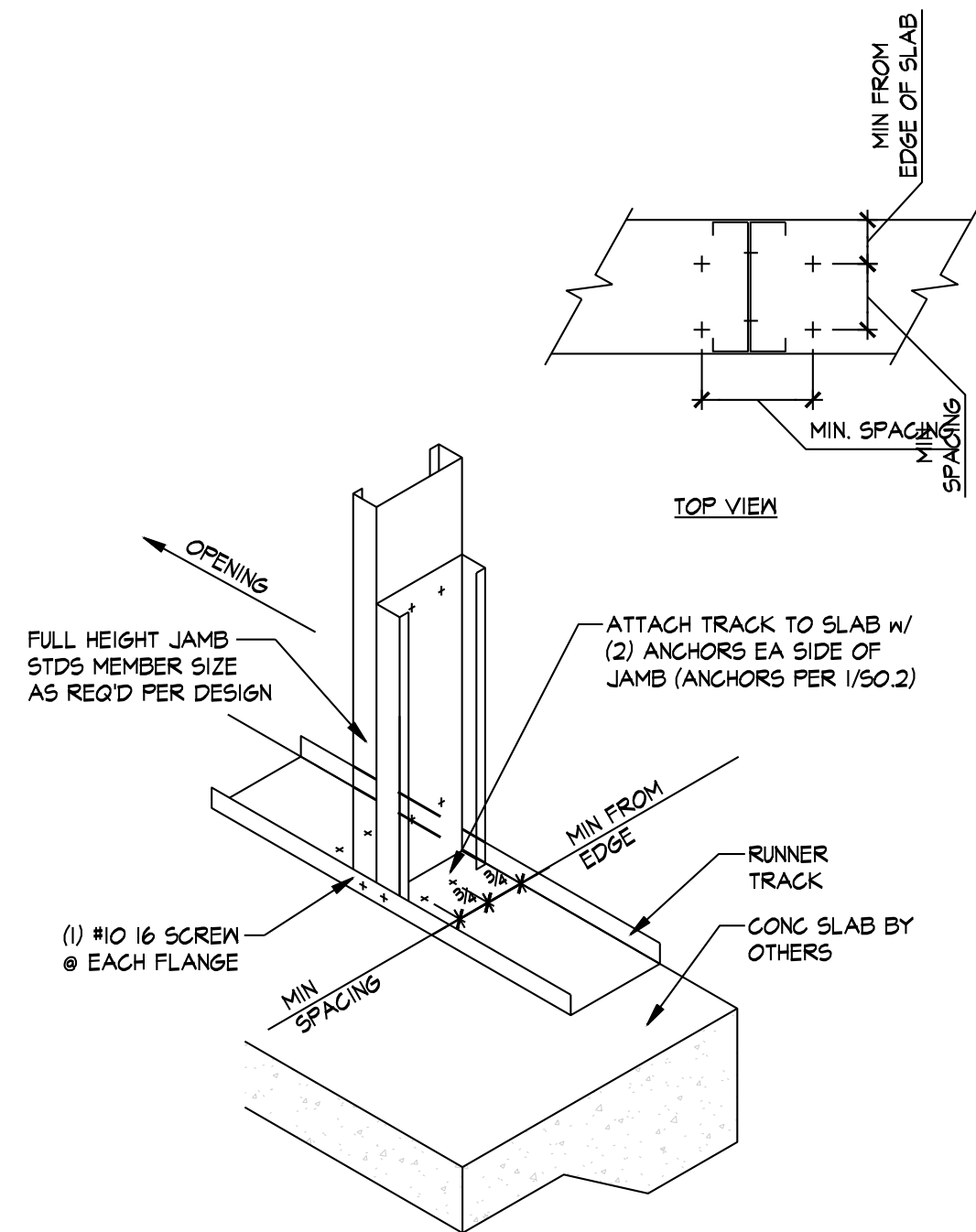
BOTTOM TRACK CONNECTION

SECTION 1
3/4" x 1'-0" SO.2



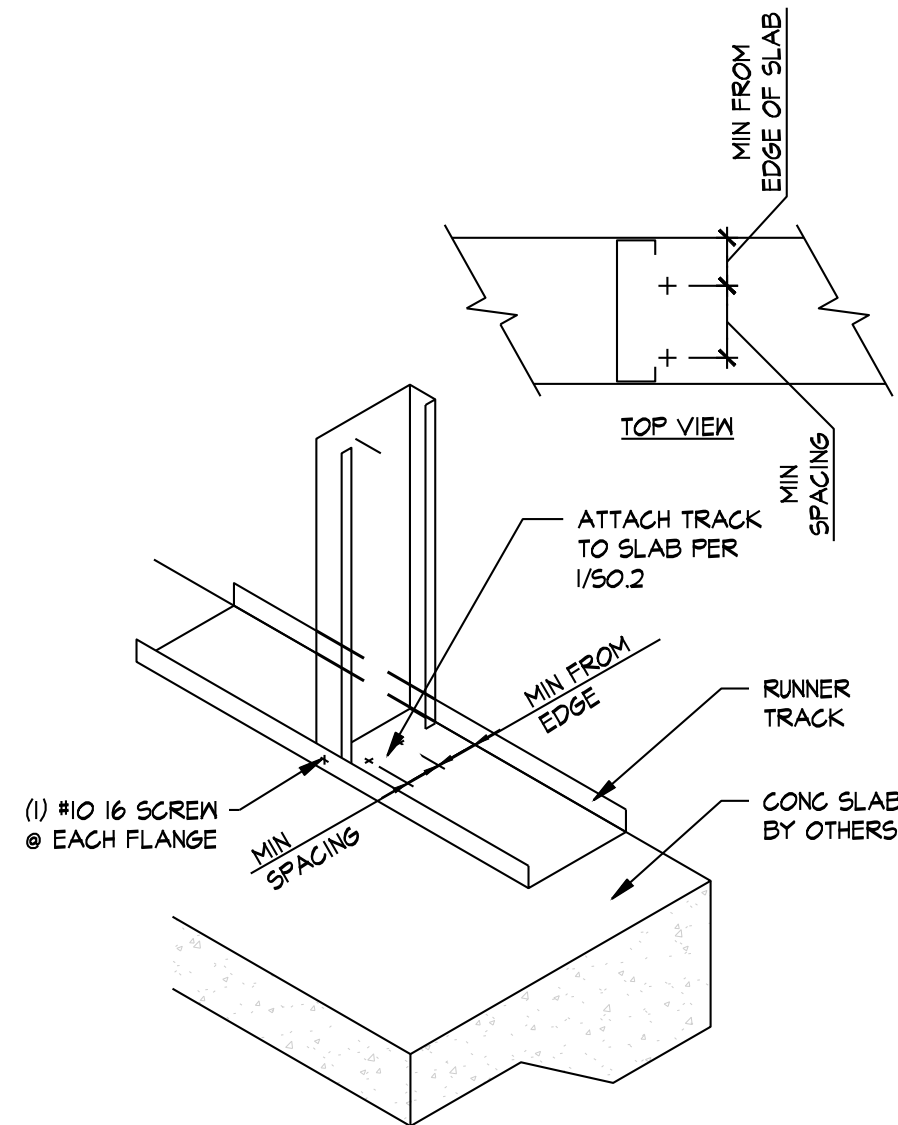
STUD-TO-TRACK CONNECTION

SECTION 6
3/4" x 1'-0" SO.2



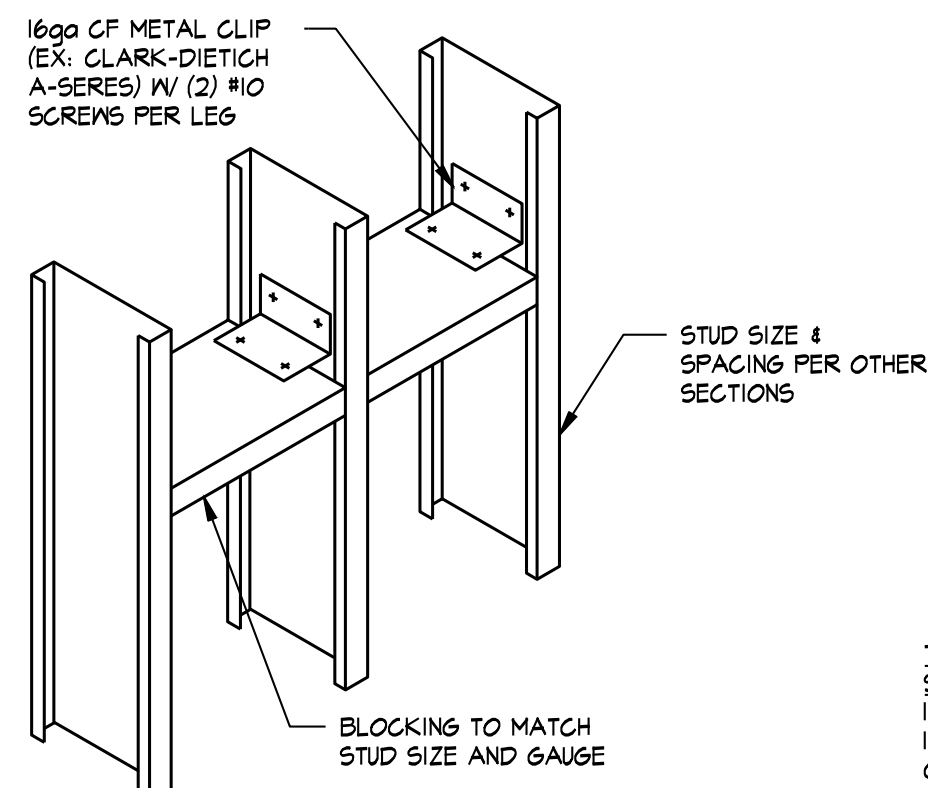
TYPICAL WINDOW JAMB ANCHORAGE

SECTION 2
3/4" x 1'-0" SO.2



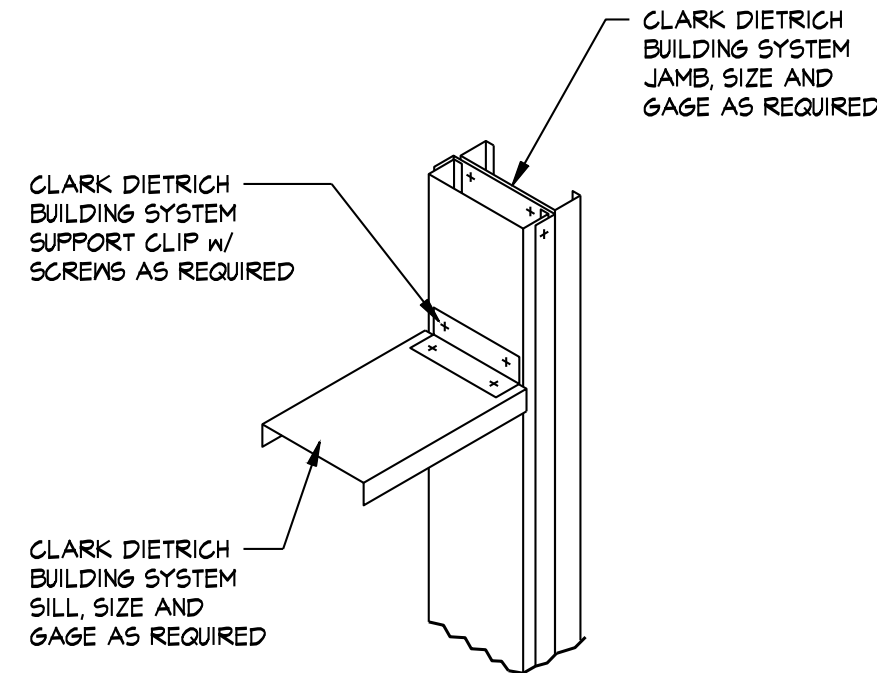
TYPICAL STUD TO TRACK DETAIL

SECTION 3
3/4" x 1'-0" SO.2



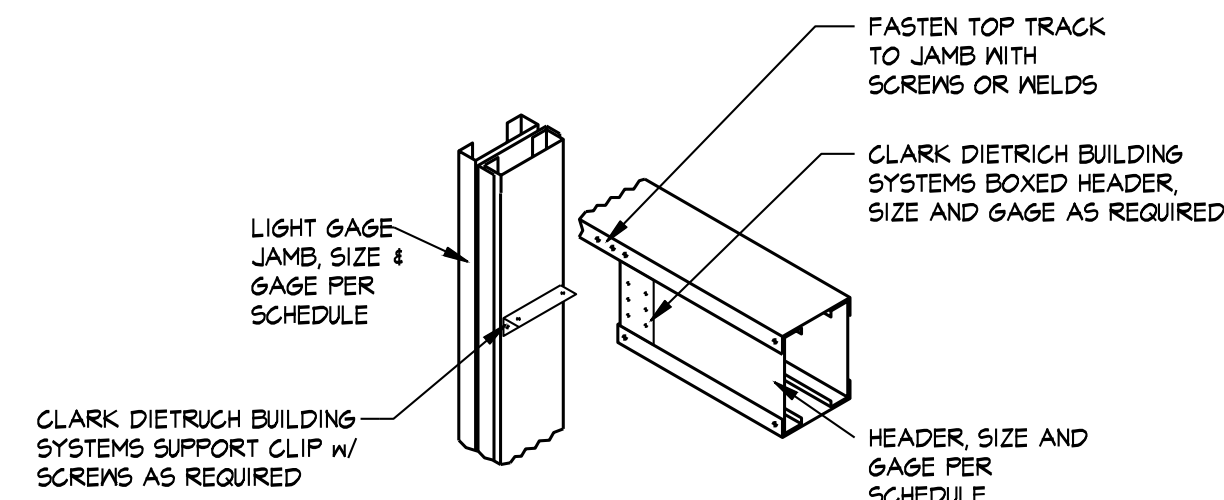
SOLID BLOCKING

SECTION 8
3/4" x 1'-0" SO.2



TYPICAL SILL DETAIL

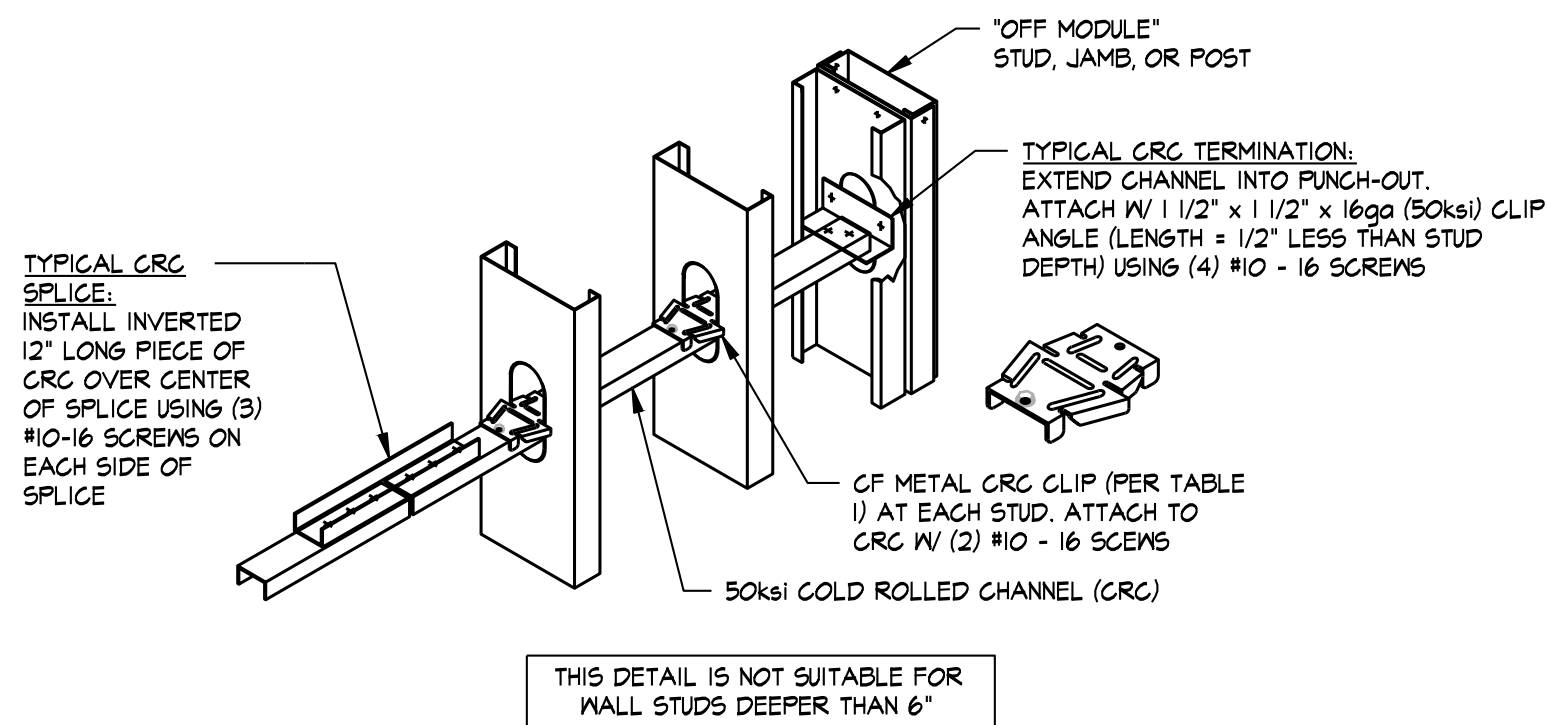
SECTION 4
3/4" x 1'-0" SO.2



TYPICAL HEADER DETAIL

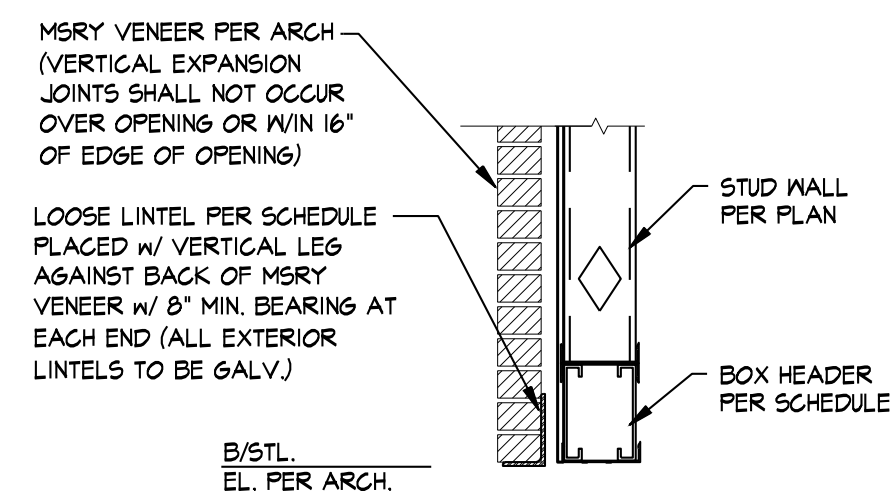
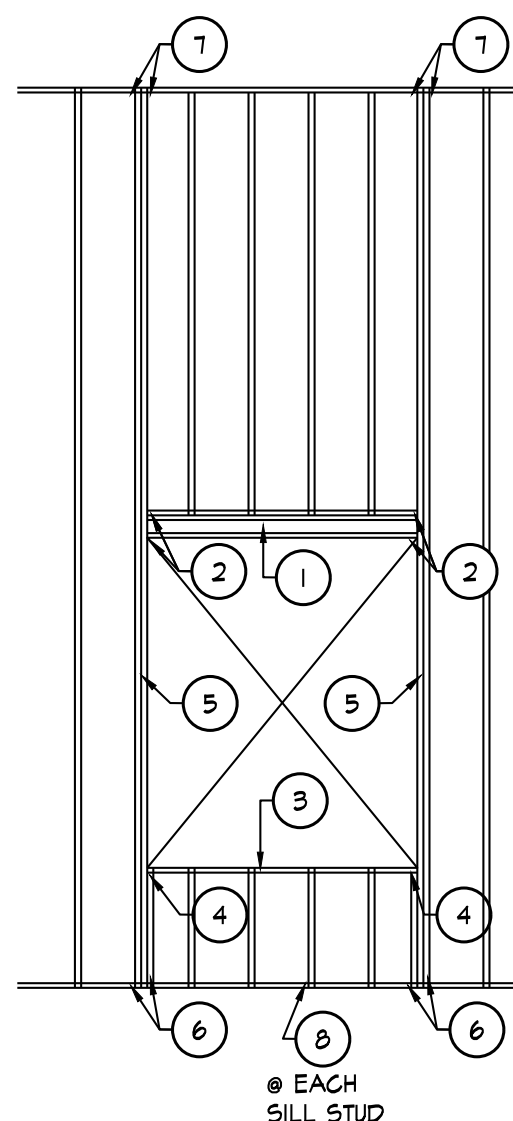
SECTION 5
1/2" x 1'-0" SO.2

TABLE 1	
STUD THICKNESS	CONNECTOR
33 - 43 MIL	18ga
54 - 91 MIL	14ga



COLD-FORMED OPENING SCHEDULE								
MARK	1 HEADER ³	2 HEADER CONNECTION	3 SILL	4 SILL CONNECTION	5 JAMB STUDS ⁵	6 JAMB STUD BASE CONNECTION	7 JAMB STUD HEAD CONNECTION	8 SILL STUDS BASE CONNECTION
CF1	(2) 800T125-43 & (2) 800S162-43	H546 HEADER HANGER EA SIDE w/ (30) #10-16 SCREWS	800T125-43	UNICLIP-2 #12 SCREWS EACH LEG OF CLIP	(2) 800S162-43 (1) 800T125-54	UNICLIP- 2 #12 SCREWS TO JAMB (2) HILTI XU TO FOUNDATION	FASTCLIP- 2 #12 SCREWS EACH LEG OF CLIPS	PER NOTE 5
CF2	(2) 800T125-54 & (2) 800S162-54	H686 HEADER HANGER EA SIDE w/ (30) 1/4"-14 SCREWS	800T125-54	UNICLIP-2 #12 SCREWS EACH LEG OF CLIP	(2) 800S162-43 (1) 800T125-54	UNICLIP- 2 #12 SCREWS TO JAMB (2) HILTI XU TO FOUNDATION	FASTCLIP- 2 #12 SCREWS EACH LEG OF CLIPS	PER NOTE 5
CF3	(2) 800T125-54 & (2) 800S162-54	H686 HEADER HANGER EA SIDE w/ (30) 1/4"-14 SCREWS	800T125-54	UNICLIP-2 #12 SCREWS EACH LEG OF CLIP	(4) 800S162-54 (2) 800T125-54	UNICLIP- 2 #12 SCREWS TO JAMB (2) HILTI XU TO FOUNDATION	FASTCLIP- 2 #12 SCREWS EACH LEG OF CLIPS	PER NOTE 5

NOTES:
1. ALL MEMBERS SHALL BE FULLY CONTINUOUS (DO NOT CUT NOR SPLICE) AND BE 660 COATED.
2. ALL MEMBERS GREATER THAN 18ga SHALL BE 50ksi.
3. CONNECT HEADER TRACKS TO EACH HEADER JOIST w/ #12 SCREW @24" OC MAX ALONG EACH TRACK FLANGE AND WEB.
4. FASTEN SHEATHING TO STUDS, TOP & BOTTOM TRACKS (BUT NOT DEFLECTION TRACKS) AT 12" OC IN FIELD OF PANEL & 6" OC AT EDGES UNO.
5. FASTEN TRACKS THROUGH EACH FLANGE TO EACH STUD w/ #10 SCREWS UNO.



TYPICAL LOOSE LINTEL DETAIL @ COLD-FORMED FRAMED WALL

SECTION 11
3/4" x 1'-0" SO.2

LOOSE LINTEL SCHEDULE	
FOR OPENINGS:	GALV. ANGLE
LESS THAN 6'-0"	5"x3 1/2"x3/8" (LLV)
6'-0" < OPENINGS < 9'-0"	6"x3 1/2"x3/8" (LLV)

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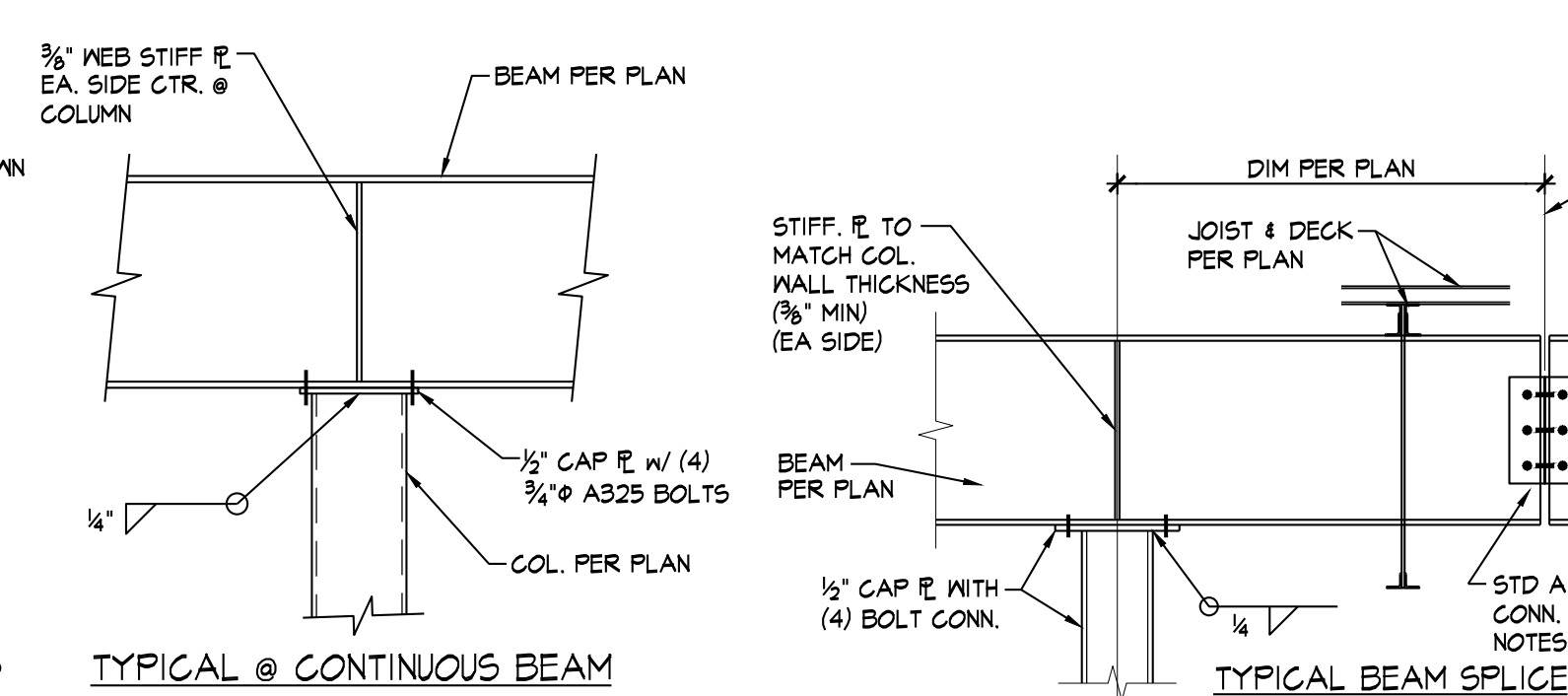
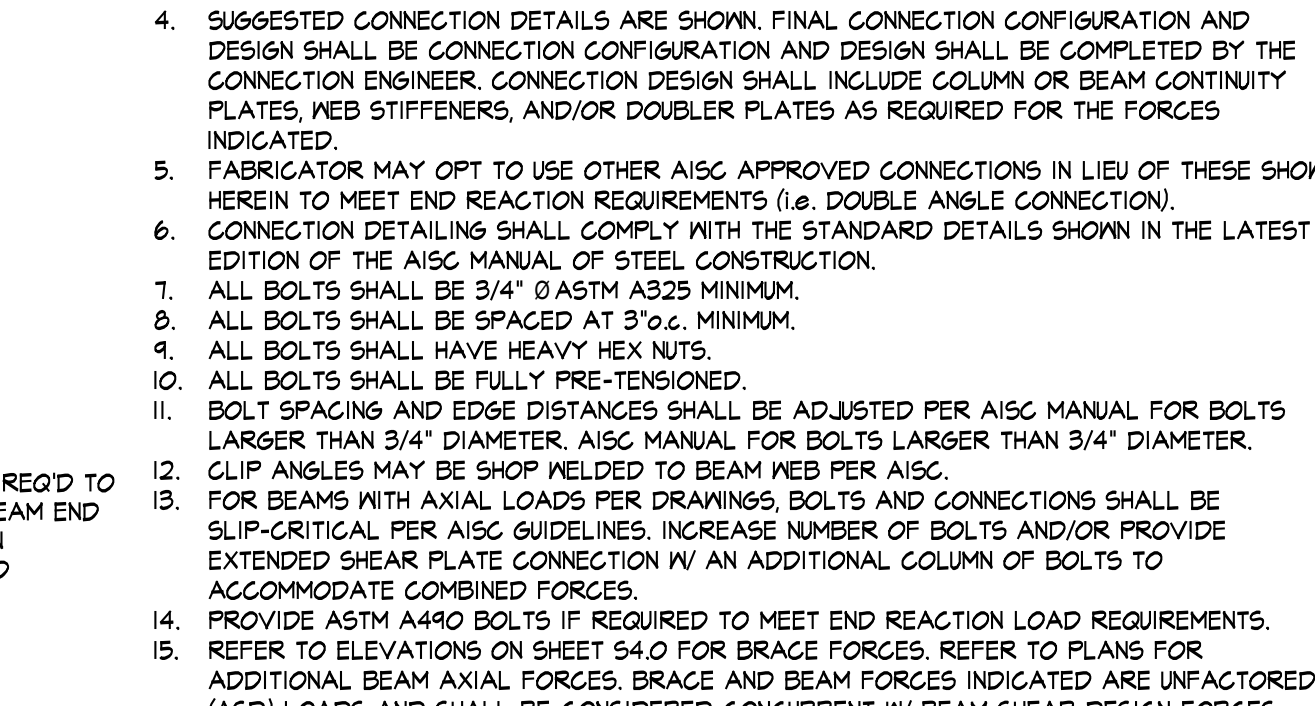
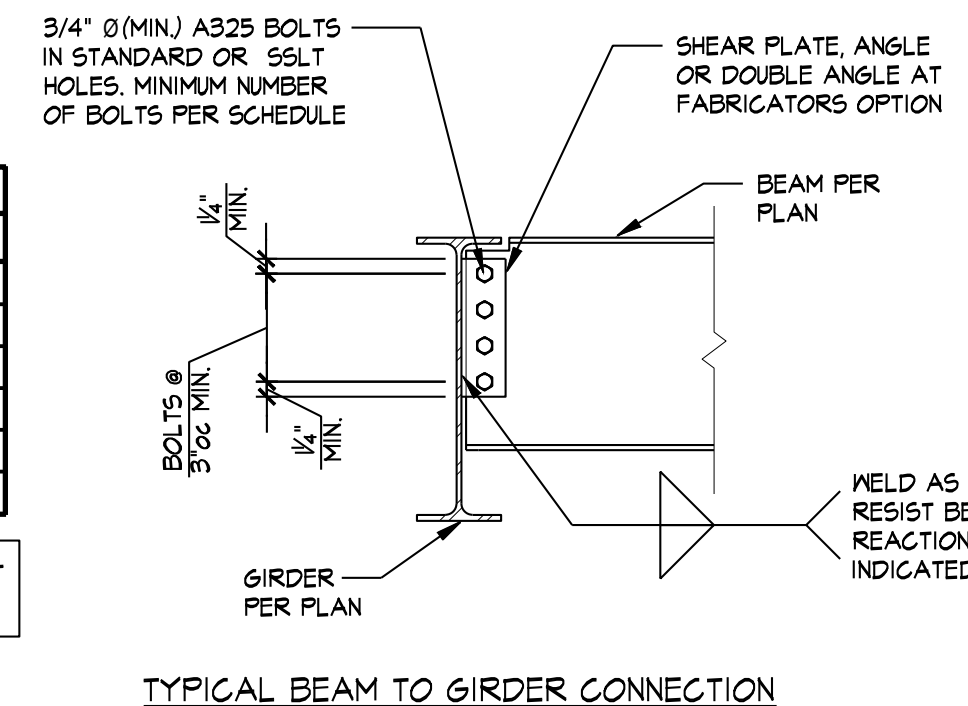
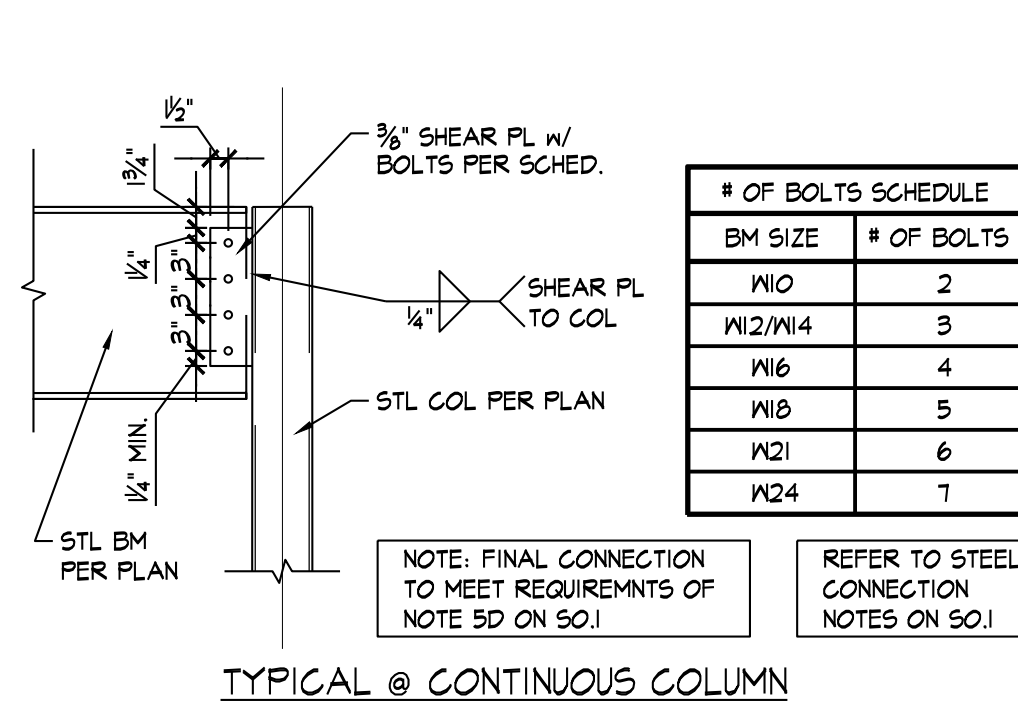
THEATRE SALINA
DANCE STUDIOS & SCENE SHOP EXPANSION
SALINA, KANSAS



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DATE: 9-18-2026
JOB: 22-3239
SHEET NO.:

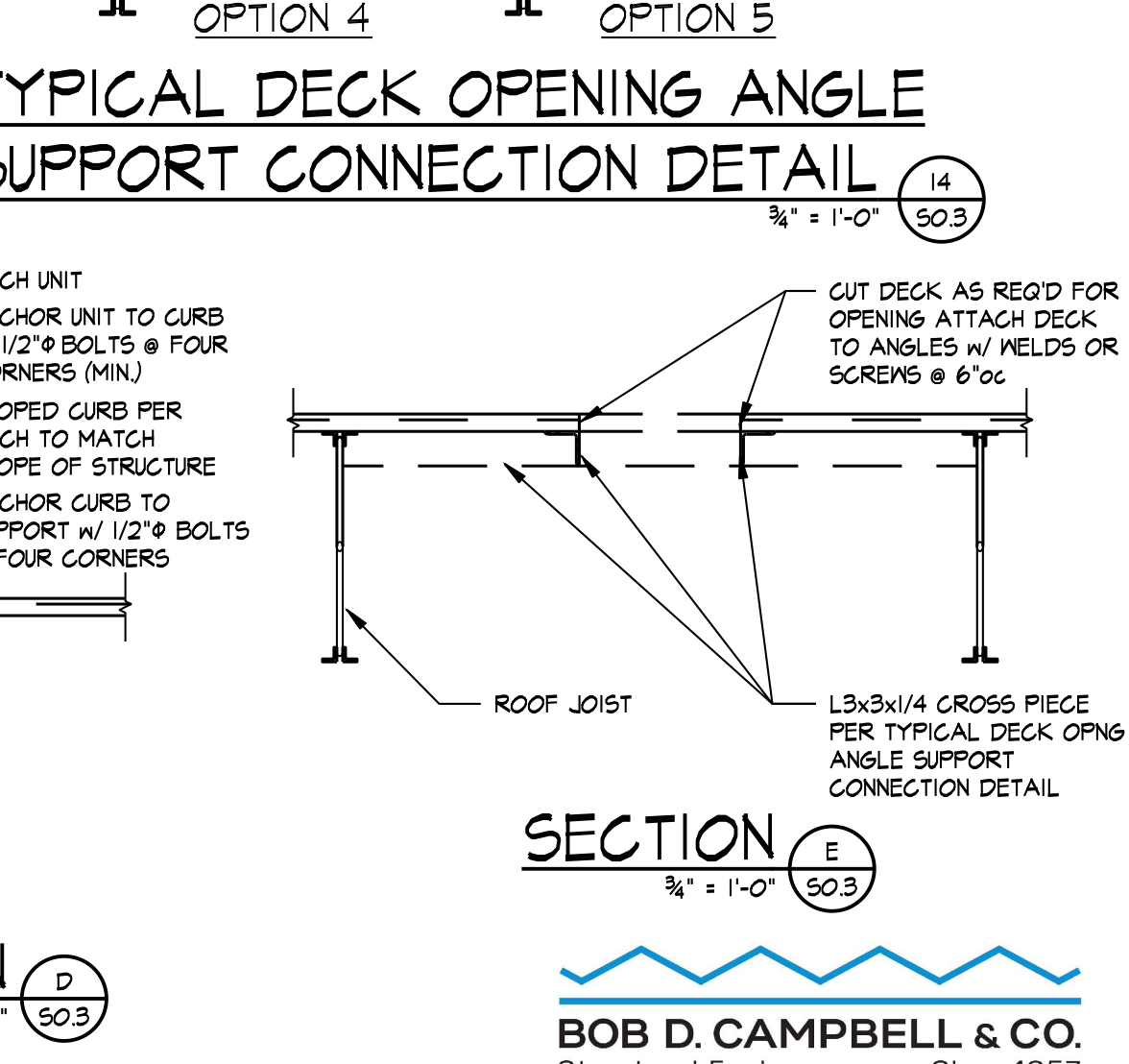
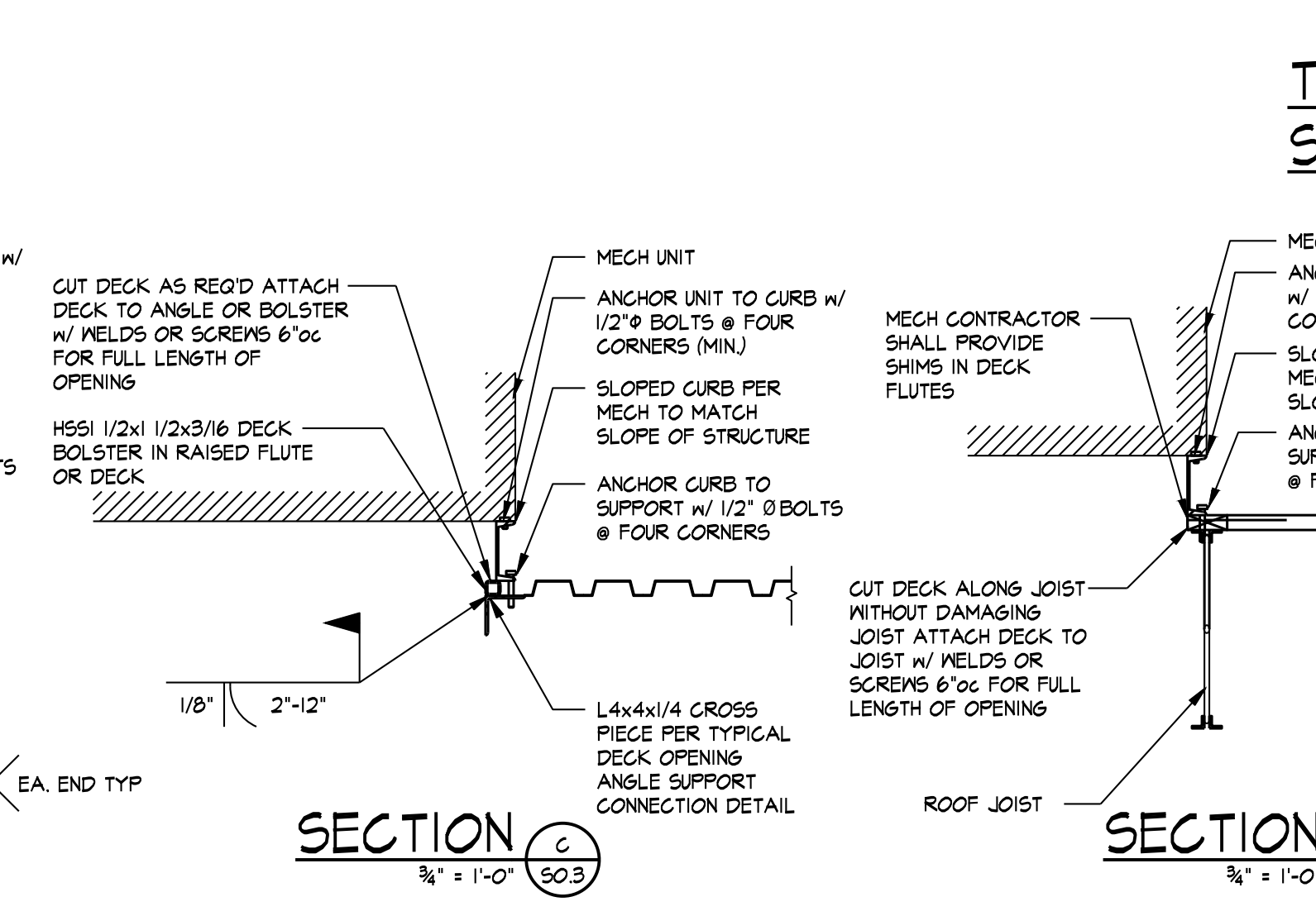
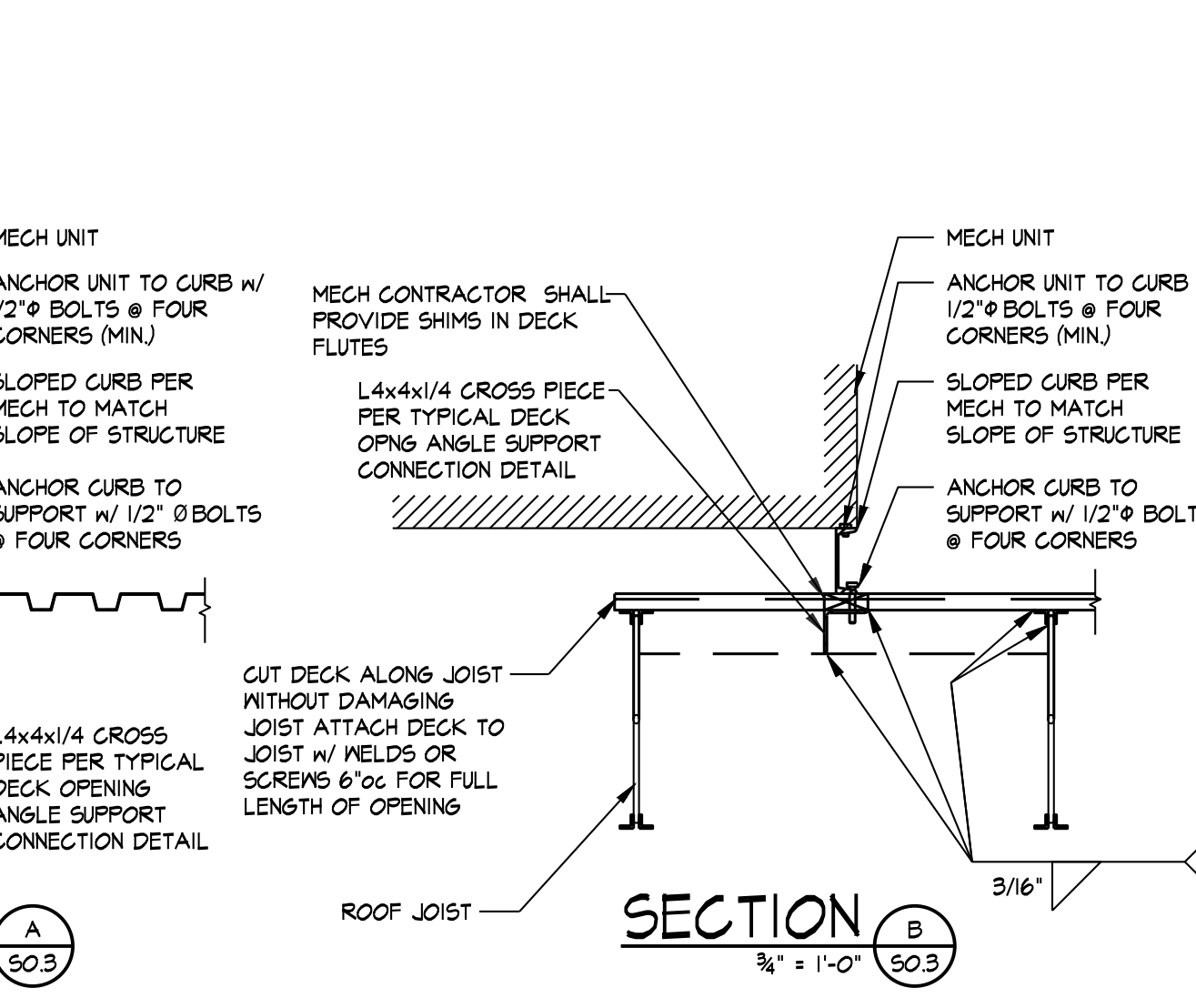
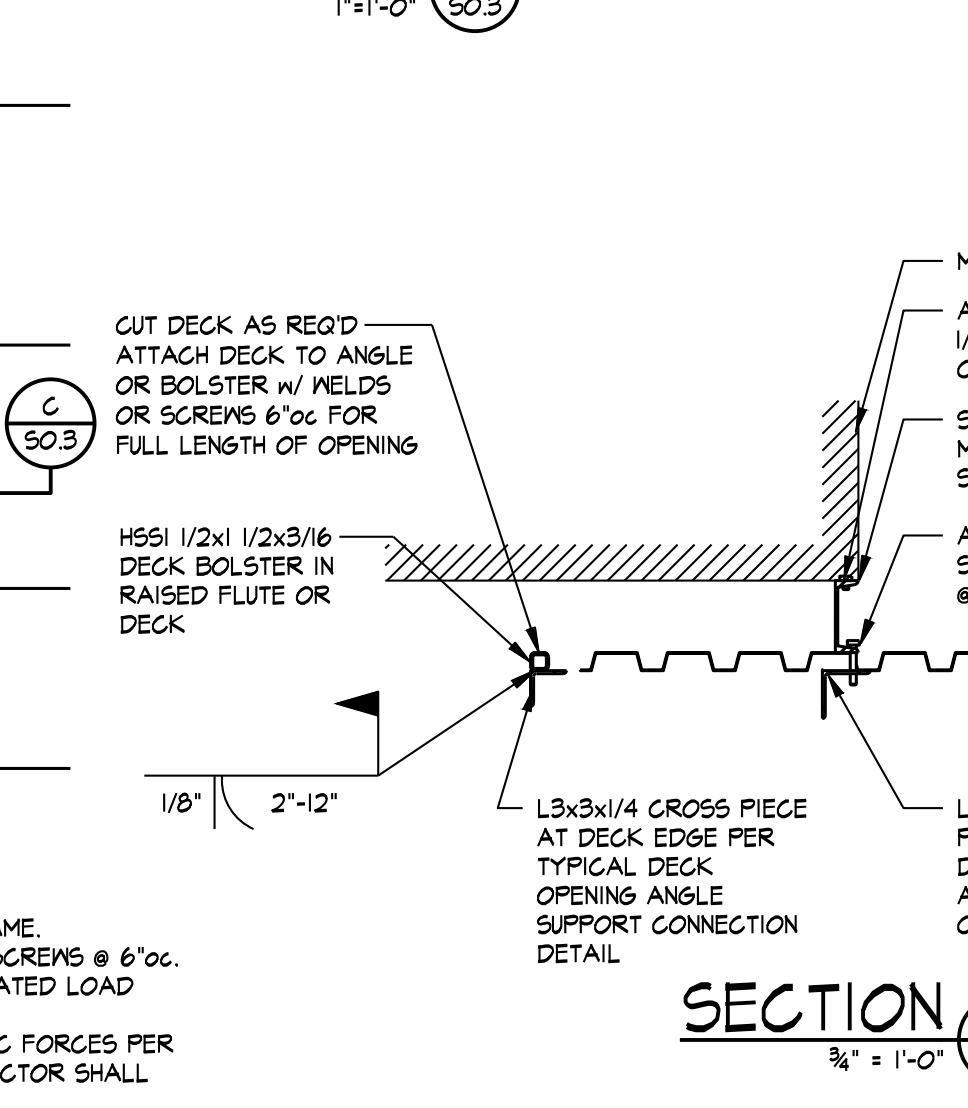
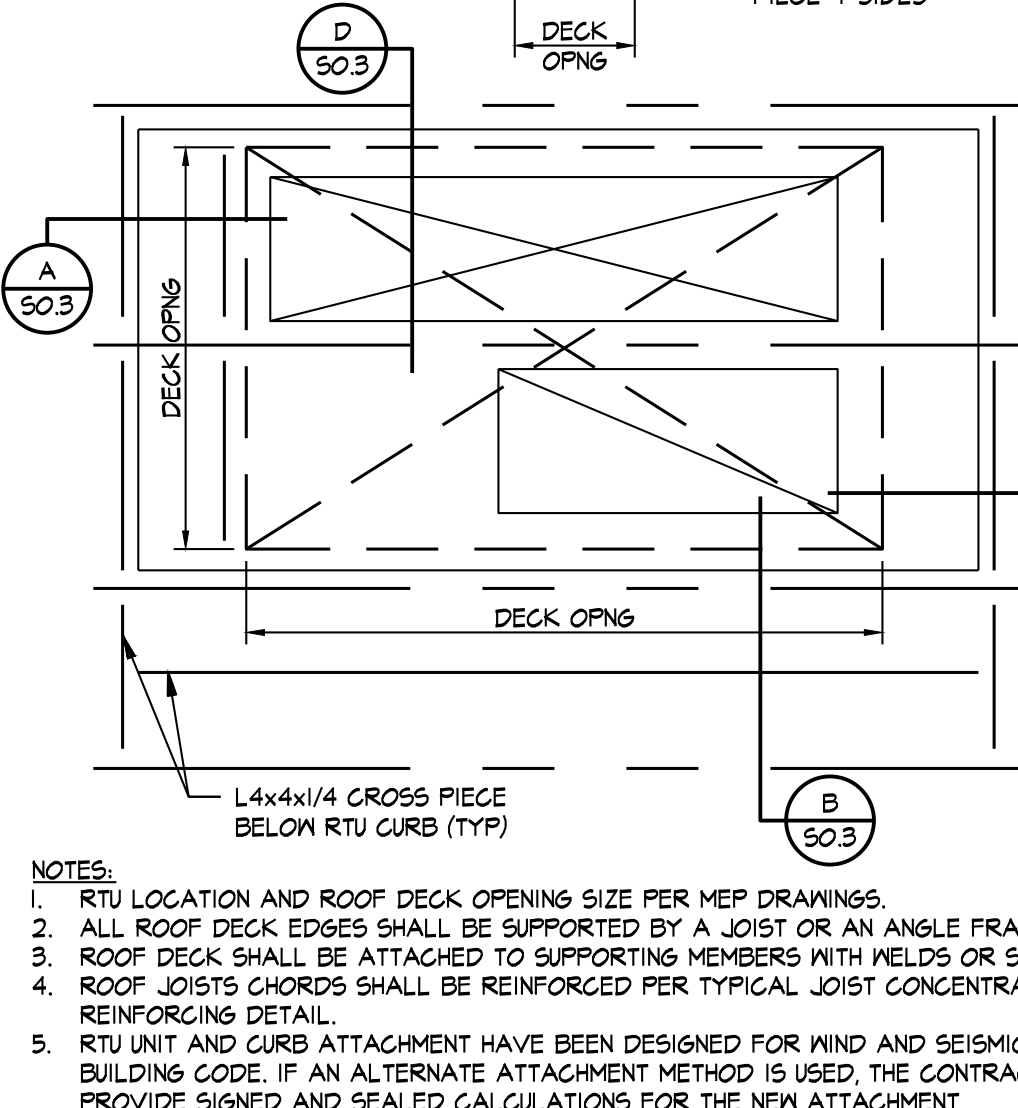
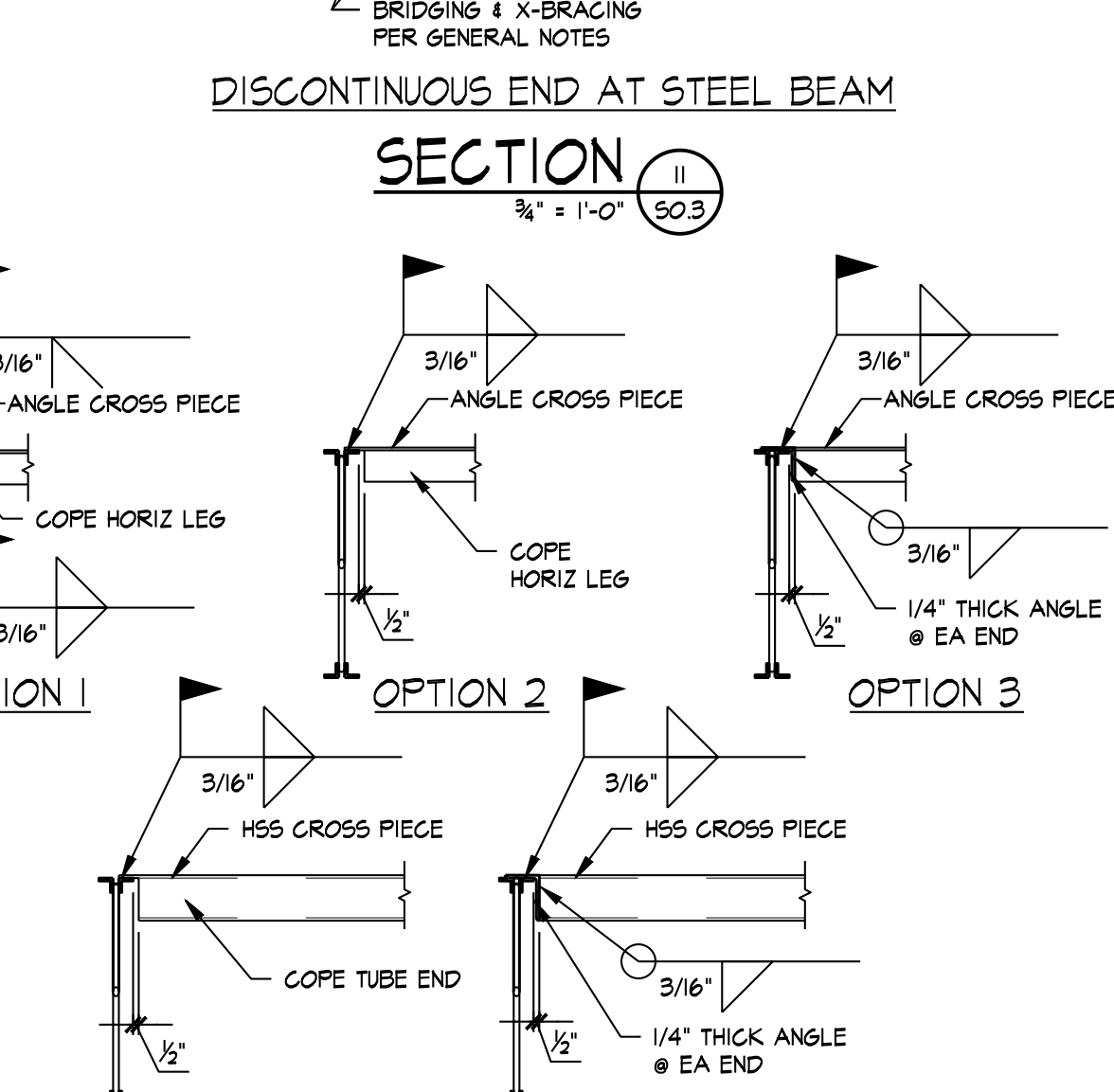
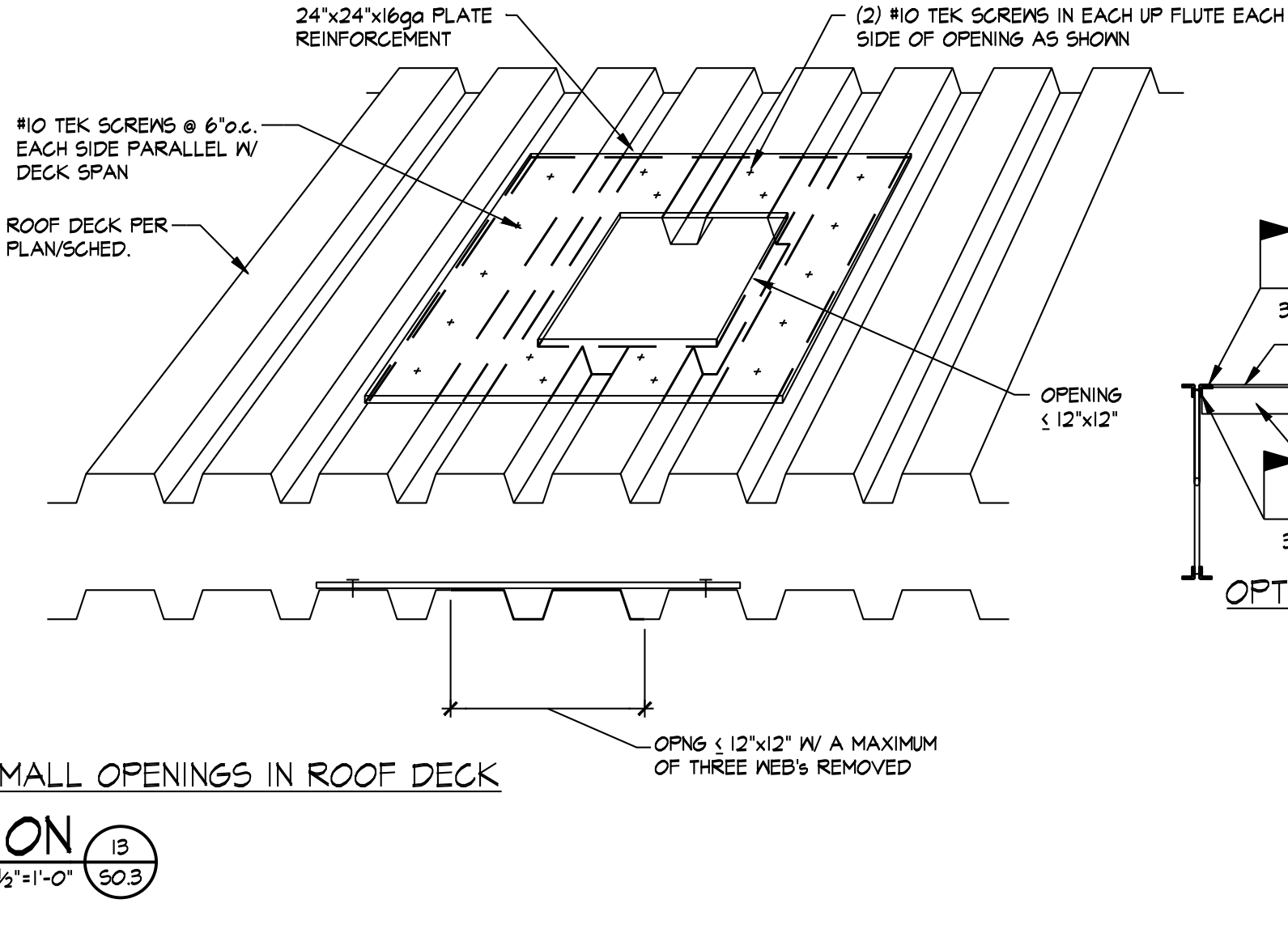
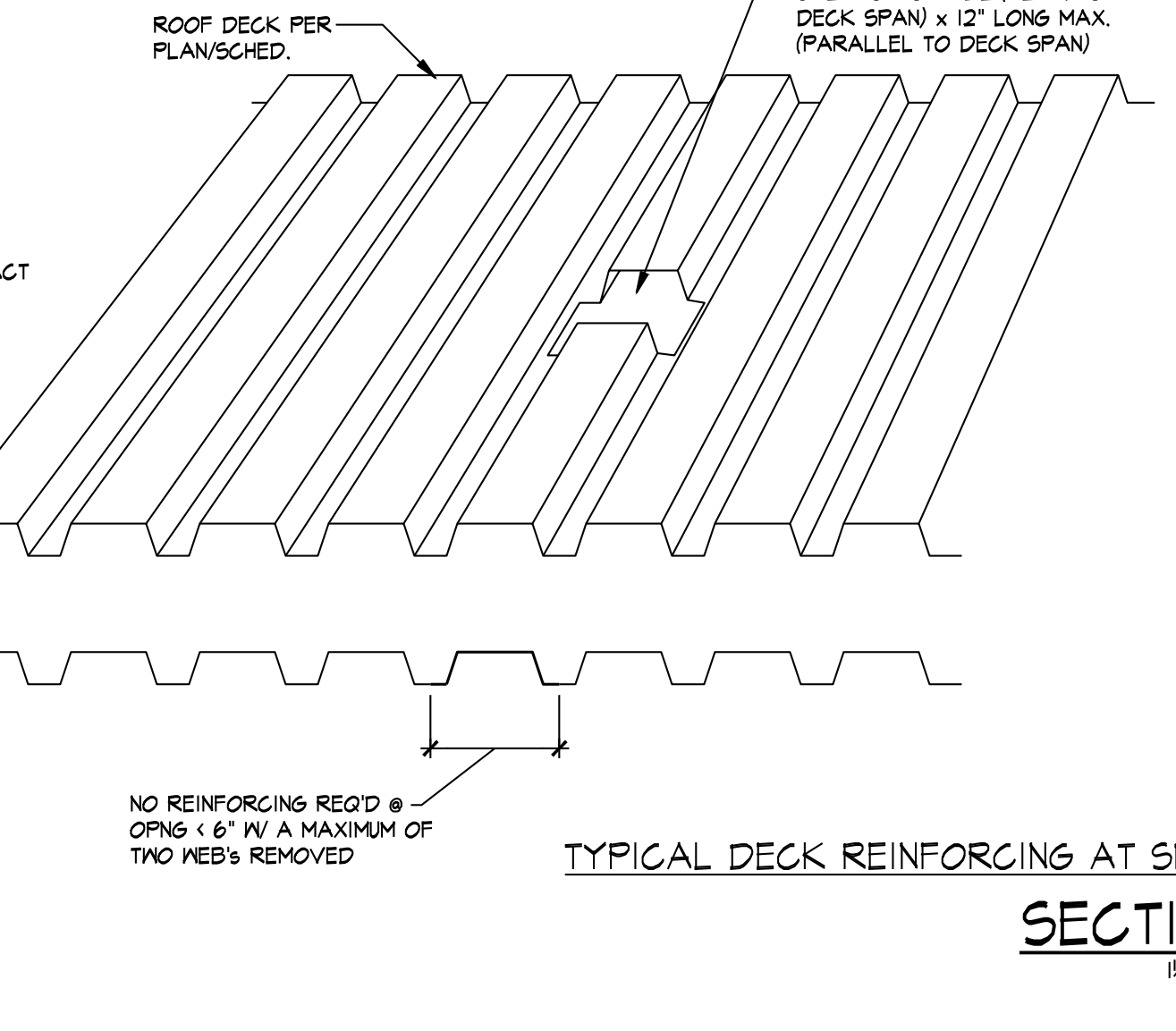
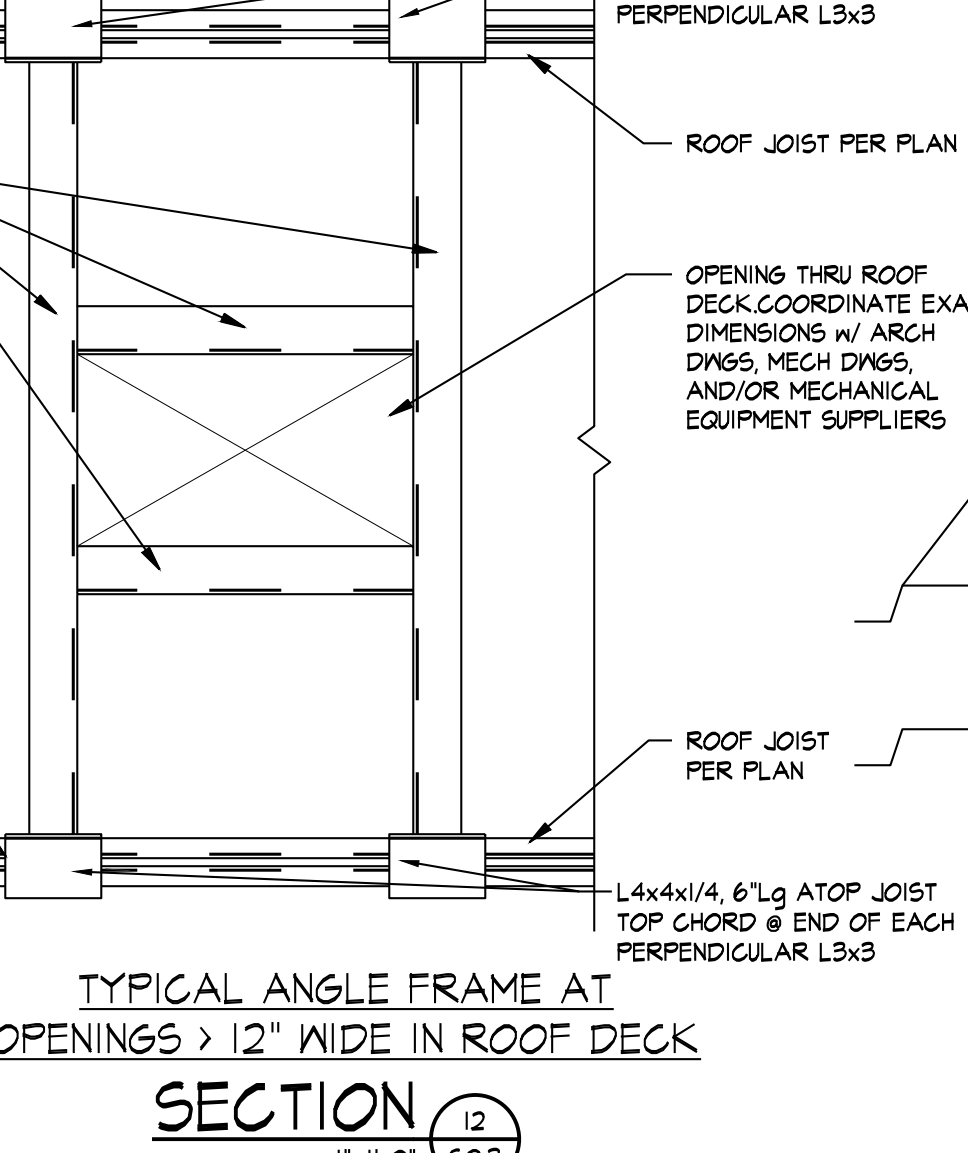
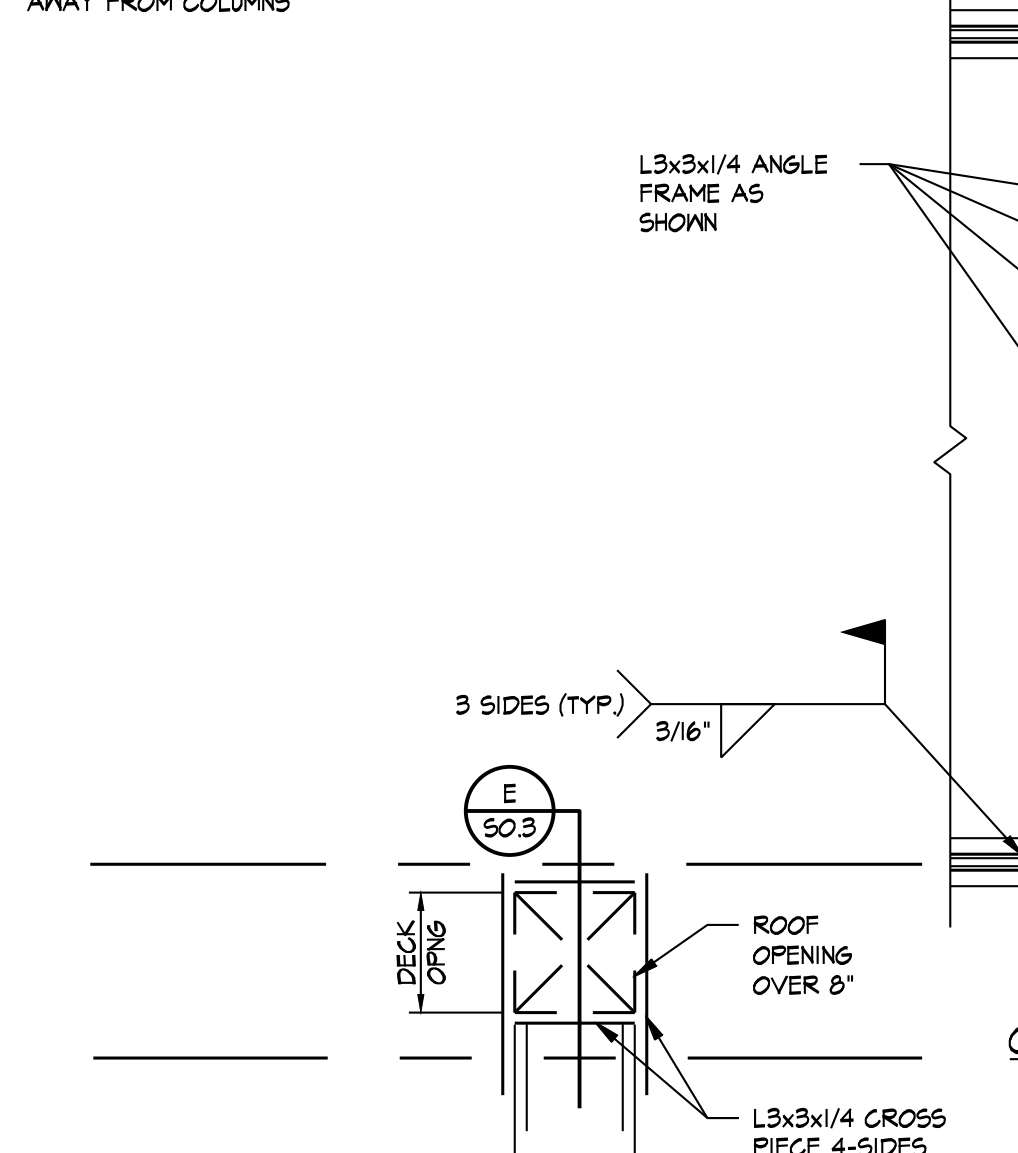
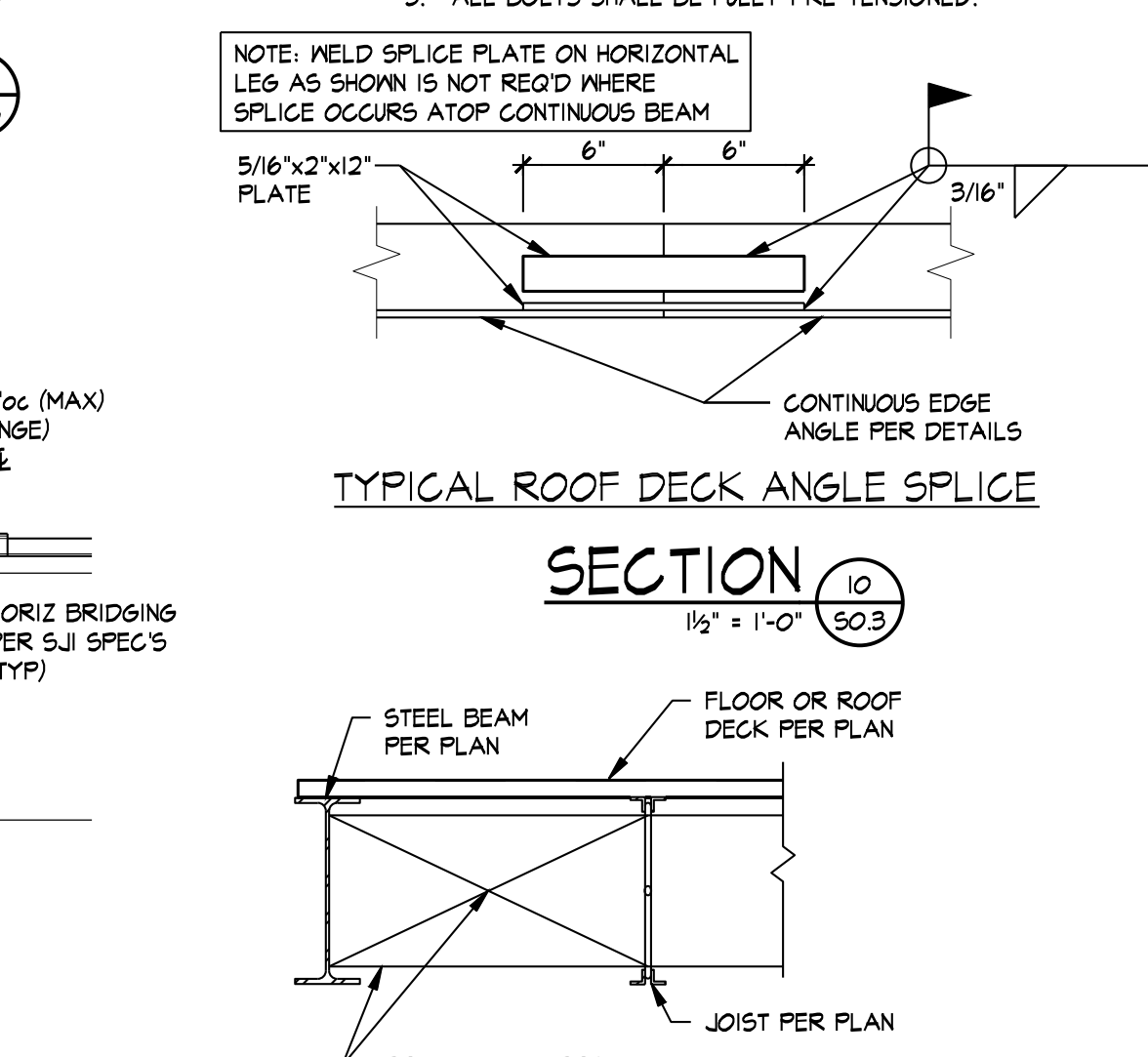
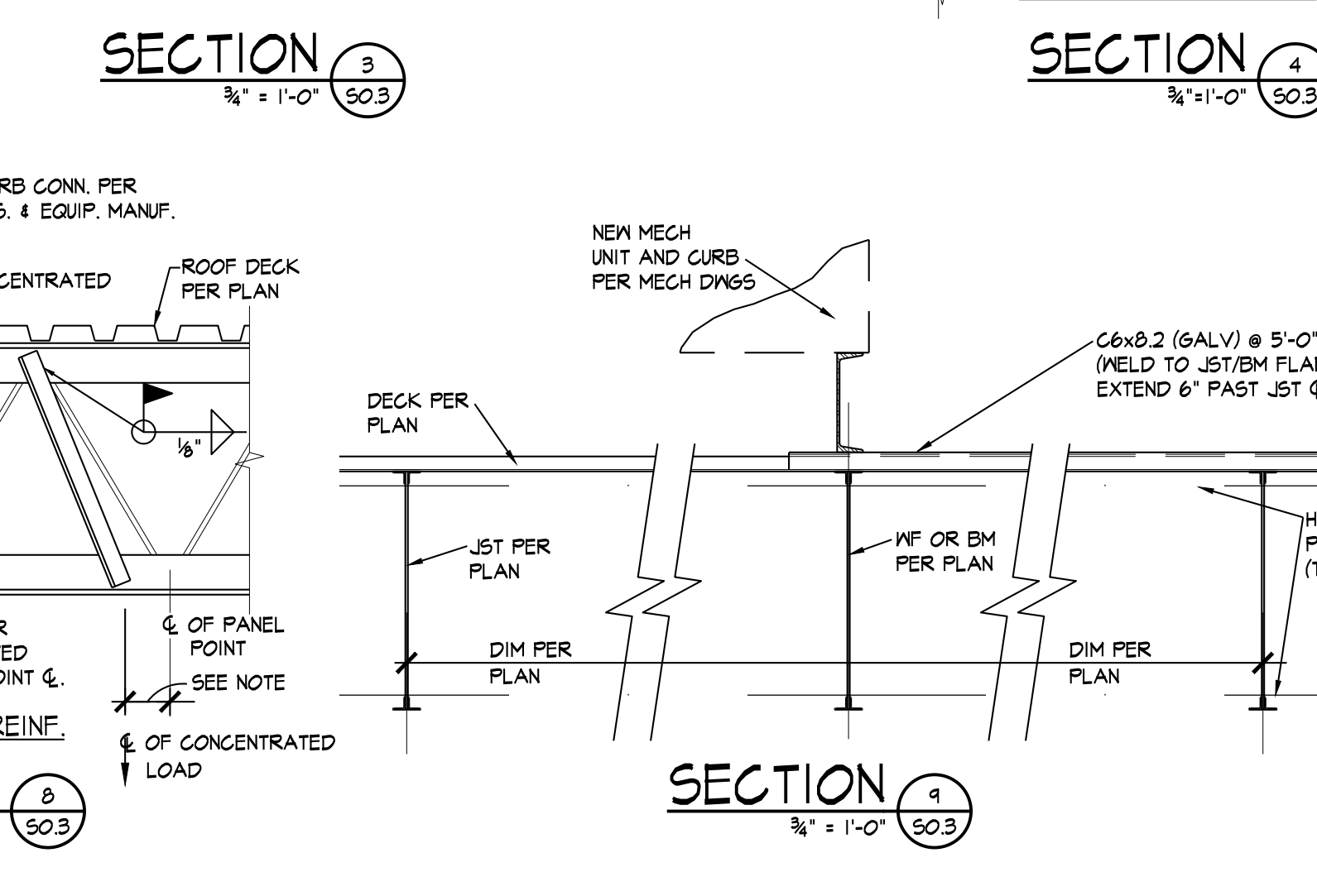
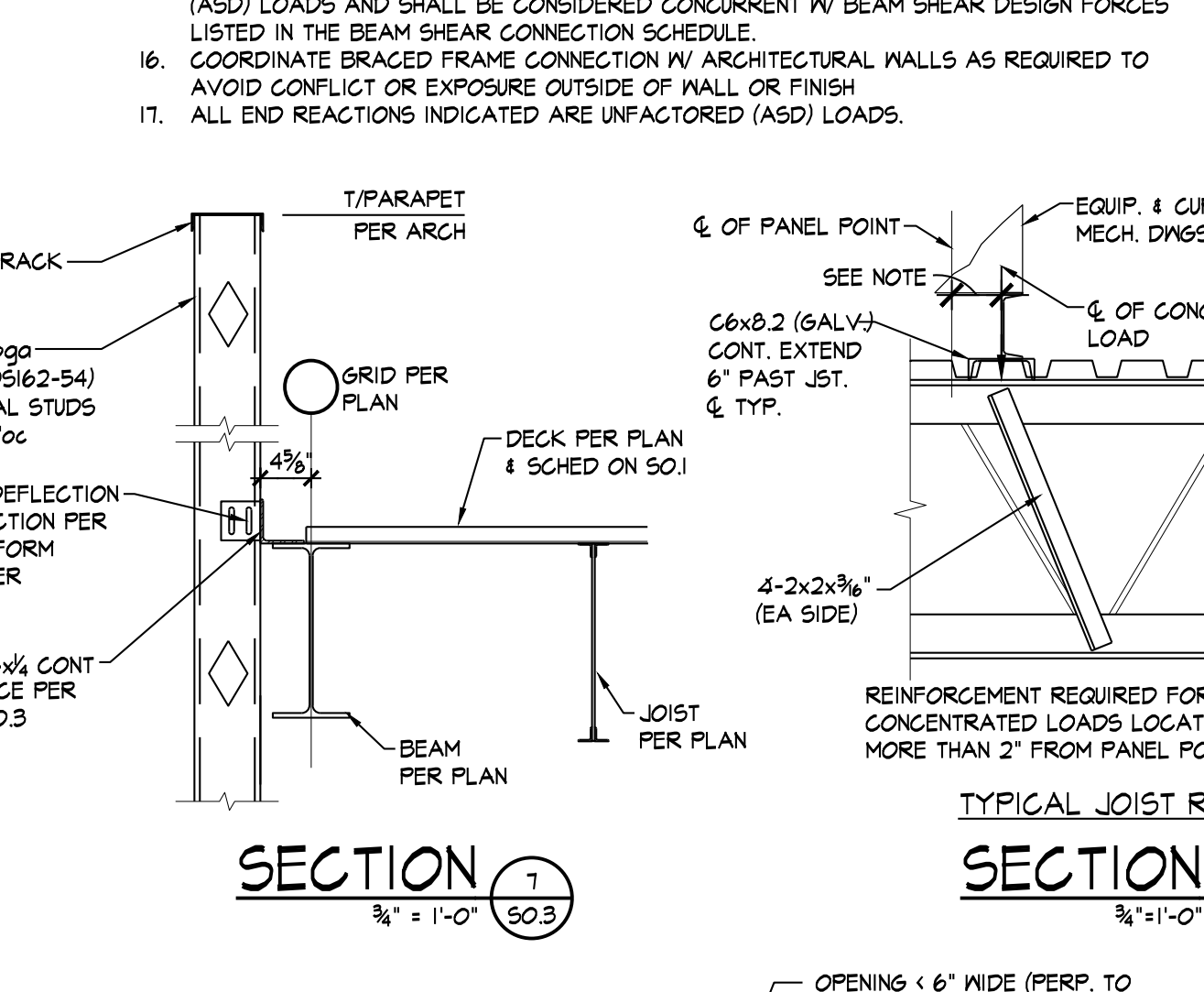
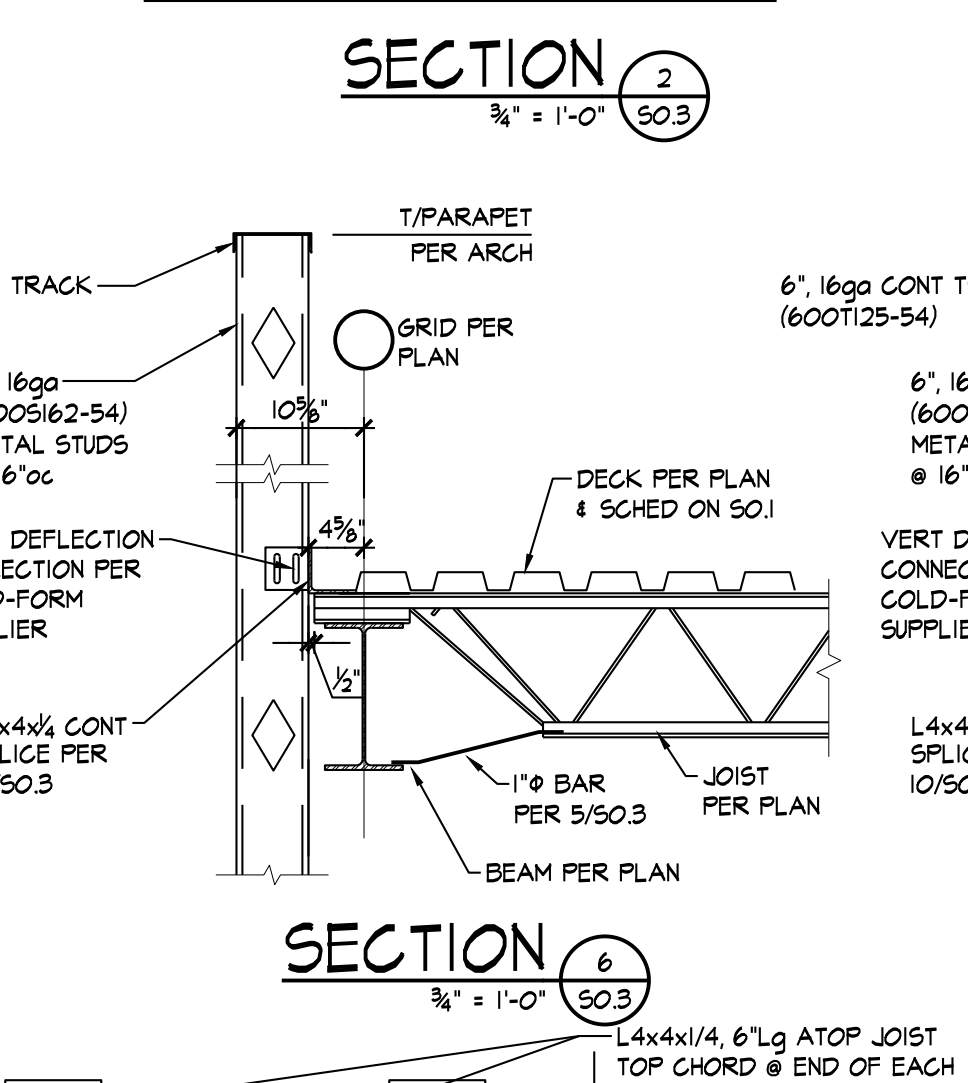
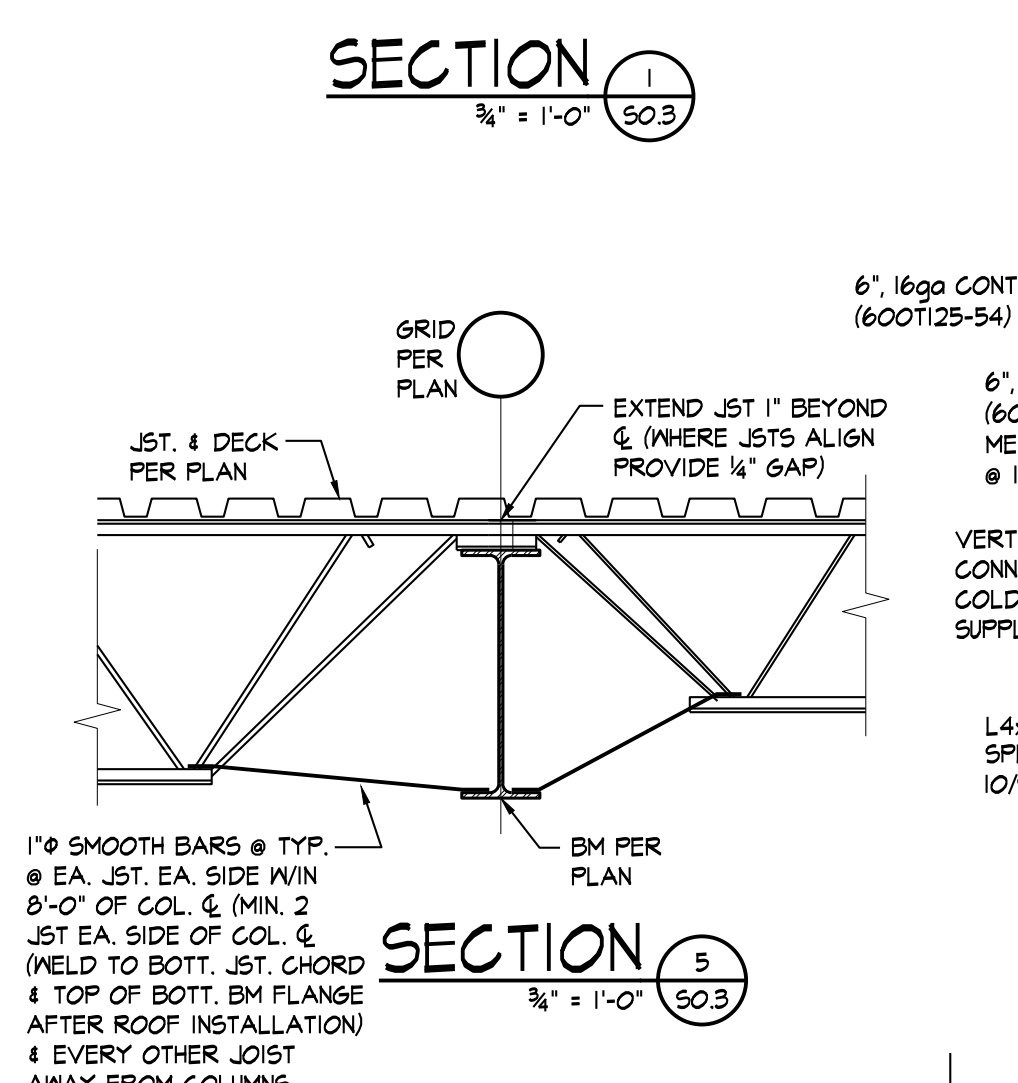
S0.2

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MINIMUM CONNECTION SCHEDULE		
BEAM SIZE	MINIMUM "N" QUANTITY 3/4" BOLTS IN WEB	MINIMUM LENGTH, "L" (IN)
W8,C8	2	6
W10,C10	3	9
W12,C12	3	9
W14,C15	3	9
W16	4	12
W18	5	15
W21	6	18
W24	6	18

SCHEDULE NOTES:
 1. ALL BOLTS SHALL BE 3/4" Ø ASTM A325.
 2. ALL BOLTS SHALL HAVE HEAVY HEX NUTS.
 3. ALL BOLTS SHALL BE FULLY PRE-TENSIONED.



NOTES:
 1. RTU LOCATION AND ROOF DECK OPENING SIZE PER MEP DRAWINGS.
 2. ALL ROOF DECK EDGES SHALL BE SUPPORTED BY A JOIST OR AN ANGLE FRAME.
 3. ROOF DECK SHALL BE ATTACHED TO SUPPORTING MEMBERS WITH WELDS OR SCREWS @ 6" OC.
 4. ROOF JOISTS CHORDS SHALL BE REINFORCED PER TYPICAL JOIST CONCENTRATED LOAD REINFORCING DETAIL.
 5. RTU UNIT AND CURB ATTACHMENT HAVE BEEN DESIGNED FOR WIND AND SEISMIC FORCES PER BUILDING CODE. IF AN ALTERNATE ATTACHMENT METHOD IS USED, THE CONTRACTOR SHALL PROVIDE SIGNED AND SEALED CALCULATIONS FOR THE NEW ATTACHMENT.

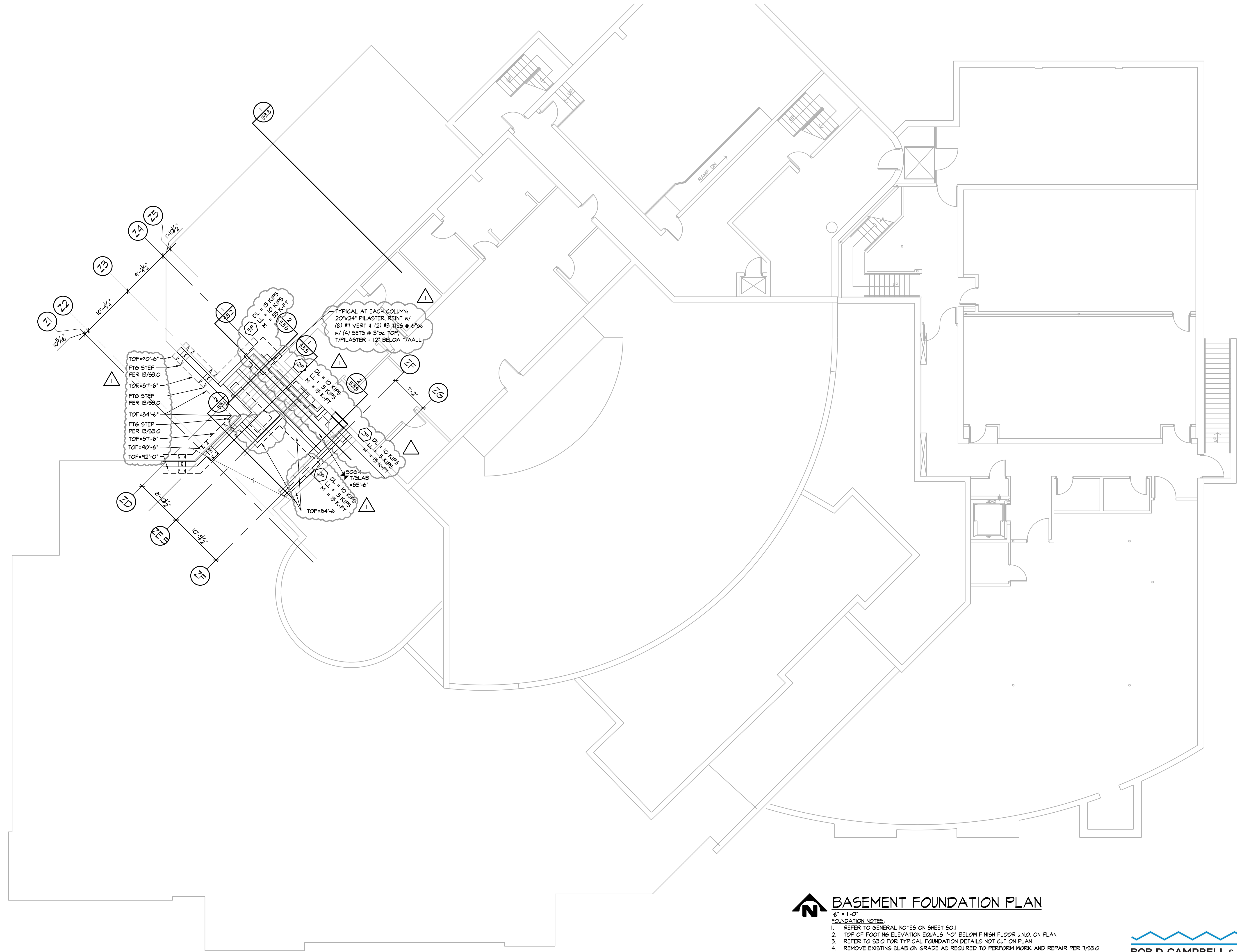
RTU FRAMING DETAILS
 3/4" = 1'-0"

RTU FRAMING DETAILS
 3/4" = 1'-0"

RTU FRAMING DETAILS
 3/4" = 1'-0"

RTU FRAMING DETAILS
 3/4" = 1'-0"

STEEL CONNECTION NOTES:
 1. REFER TO GENERAL NOTES ON SHEET S0.1.
 2. CONNECTIONS SHOWN IN THESE DETAILS ARE MINIMUM REQUIREMENTS.
 3. FABRICATOR SHALL BE RESPONSIBLE FOR THE ENGINEERING, DESIGNING, AND DETAILING OF EACH CONNECTION FOR LOADS SHOWN ON THE DRAWINGS IN ACCORDANCE WITH THE SPECIFICATIONS AND THE STRUCTURAL GENERAL NOTES.
 4. SUGGESTED CONNECTION DETAILS ARE SHOWN. FINAL CONNECTION CONFIGURATION AND DESIGN SHALL BE COMPLETED BY THE CONNECTION ENGINEER. CONNECTION DESIGN SHALL INCLUDE COLUMN OR BEAM CONTINUITY PLATES, WEB STIFFENERS, AND/OR DOUBLER PLATES AS REQUIRED FOR THE FORCES INDICATED.
 5. FABRICATOR MAY OPT TO USE OTHER AISC APPROVED CONNECTIONS IN LIEU OF THESE SHOWN HEREIN TO MEET END REACTION REQUIREMENTS (i.e. DOUBLE ANGLE CONNECTION).
 6. CONNECTION DETAILING SHALL COMPLY WITH THE STANDARD DETAILS SHOWN IN THE LATEST EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION.
 7. ALL BOLTS SHALL BE 3/4" Ø ASTM A325 MINIMUM.
 8. ALL BOLTS SHALL HAVE HEAVY HEX NUTS.
 9. ALL BOLTS SHALL BE FULLY PRE-TENSIONED.
 10. BOLT SPACING AND EDGE DISTANCES SHALL BE ADJUSTED PER AISC MANUAL FOR BOLTS LARGER THAN 3/4" DIAMETER. AISC MANUAL FOR BOLTS LARGER THAN 3/4" DIAMETER. CLIP ANGLES MAY BE SHOP WELDED TO BEAM WEB PER AISC.
 11. FOR BEAMS WITH AXIAL LOADS PER DRAWINGS, BOLTS AND CONNECTIONS SHALL BE SLIP-CRITICAL PER AISC GUIDELINES. INCREASE NUMBER OF BOLTS AND/OR PROVIDE EXTENDED SHEAR PLATE CONNECTION IN AN ADDITIONAL COLUMN OF BOLTS TO ACCOMMODATE COMBINED FORCES.
 12. PROVIDE ASTM A490 BOLTS IF REQUIRED TO MEET END REACTION LOAD REQUIREMENTS.
 13. REFER TO ELEVATIONS ON SHEET S4.0 FOR BRACE FORCES. REFER TO PLANS FOR ADDITIONAL BEAM AXIAL FORCES, BRACE AND BEAM FORCES INDICATED ARE UNFACTORED (ASD) LOADS AND SHALL BE CONSIDERED CONCURRENT IN BEAM SHEAR DESIGN FORCES LISTED IN THE BEAM SHEAR CONNECTION SCHEDULE.
 14. COORDINATE BRACED FRAME CONNECTION IN ARCHITECTURAL WALLS AS REQUIRED TO AVOID CONFLICT OR EXPOSURE OUTSIDE OF WALL OR FINISH.
 15. ALL END REACTIONS INDICATED ARE UNFACTORED (ASD) LOADS.



BASEMENT FOUNDATION PLAN

1/8" = 1'-0"

FOUNDATION NOTES:

1. REFER TO GENERAL NOTES ON SHEET S0.1
2. TOP OF FOOTING ELEVATION EQUALS 1'-0" BELOW FINISH FLOOR U.N.O. ON PLAN
3. REFER TO S0.0 FOR TYPICAL FOUNDATION DETAILS NOT CUT ON PLAN
4. REMOVE EXISTING SLAB ON GRADE AS REQUIRED TO PERFORM WORK AND REPAIR PER 1/53.0
5. REFER TO SCHEDULES ON S0.1
6. PROVIDE (6) 15X3'-6" (3T, 3B) DRILL & EPOXY 12" INTO EXISTING FOOTING & PROJECT INTO NEW FOOTING. LOCATE 4" CLEAR FROM TOP/BOTTOM OF EXISTING FOOTING

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1 Addendum 1 10/15/26

DATE: 9-18-2026

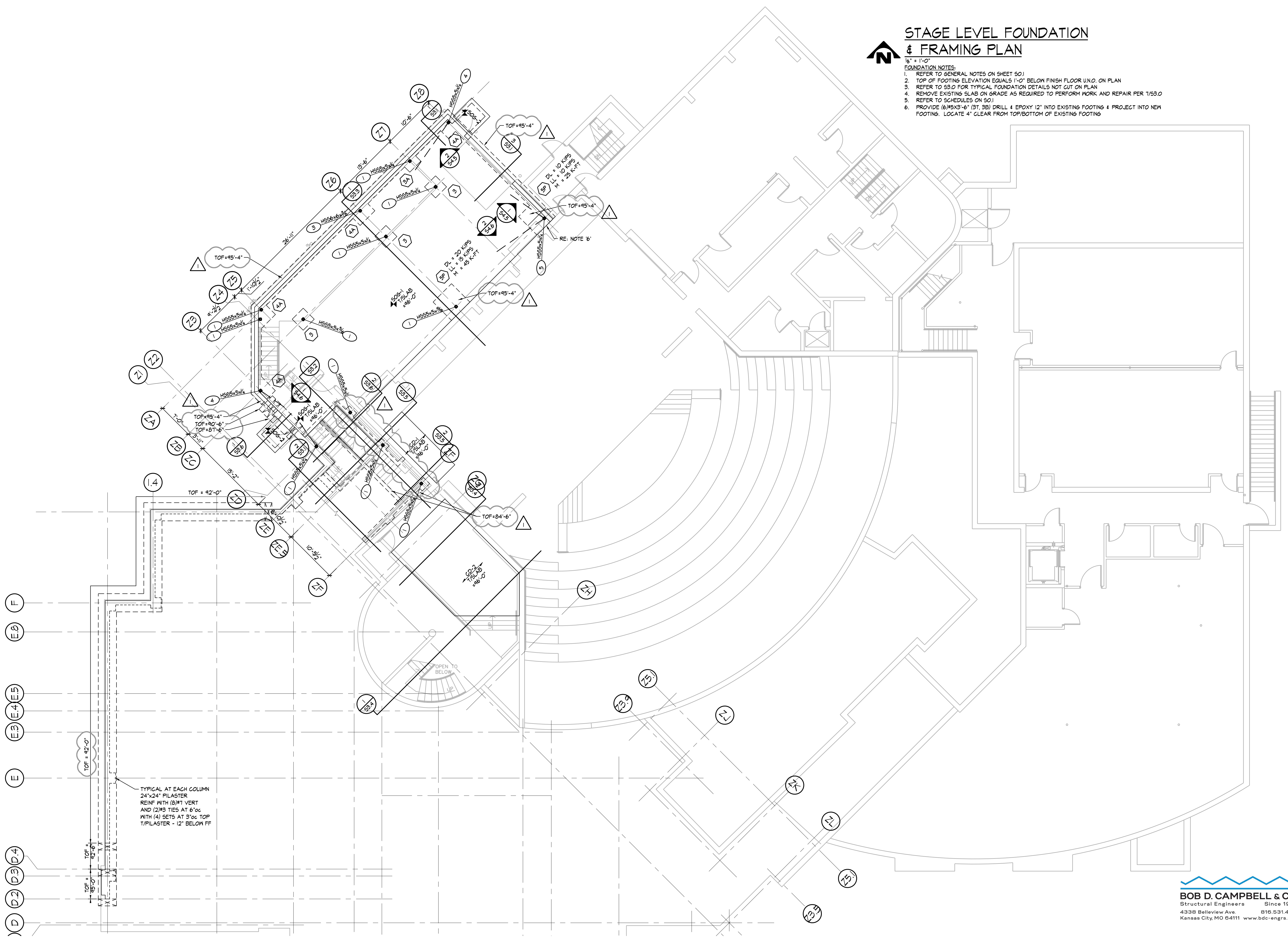
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**STAGE LEVEL FOUNDATION
& FRAMING PLAN**

- 1/8" = 1'-0"
- FOUNDATION NOTES:
1. REFER TO GENERAL NOTES ON SHEET 50.1
 2. TOP OF FOOTING ELEVATION EQUALS 1'-0" BELOW FINISH FLOOR U.N.O. ON PLAN
 3. REFER TO 53.0 FOR TYPICAL FOUNDATION DETAILS NOT CUT ON PLAN
 4. REMOVE EXISTING SLAB ON GRADE AS REQUIRED TO PERFORM WORK AND REPAIR PER 1/53.0
 5. REFER TO SCHEDULES ON 50.1
 6. PROVIDE (6) #5X3'-6" (3T, 3B) DRILL & EPOXY 12" INTO EXISTING FOOTING & PROJECT INTO NEW FOOTING. LOCATE 4" CLEAR FROM TOP/BOTTOM OF EXISTING FOOTING

TYPICAL AT EACH COLUMN
24"x24" PILASTER
REINF WITH (8) #1 VERT
AND (2) #3 TIES AT 6" OC
WITH (4) SETS AT 3' OC TOP
T/PILASTER - 12" BELOW FF

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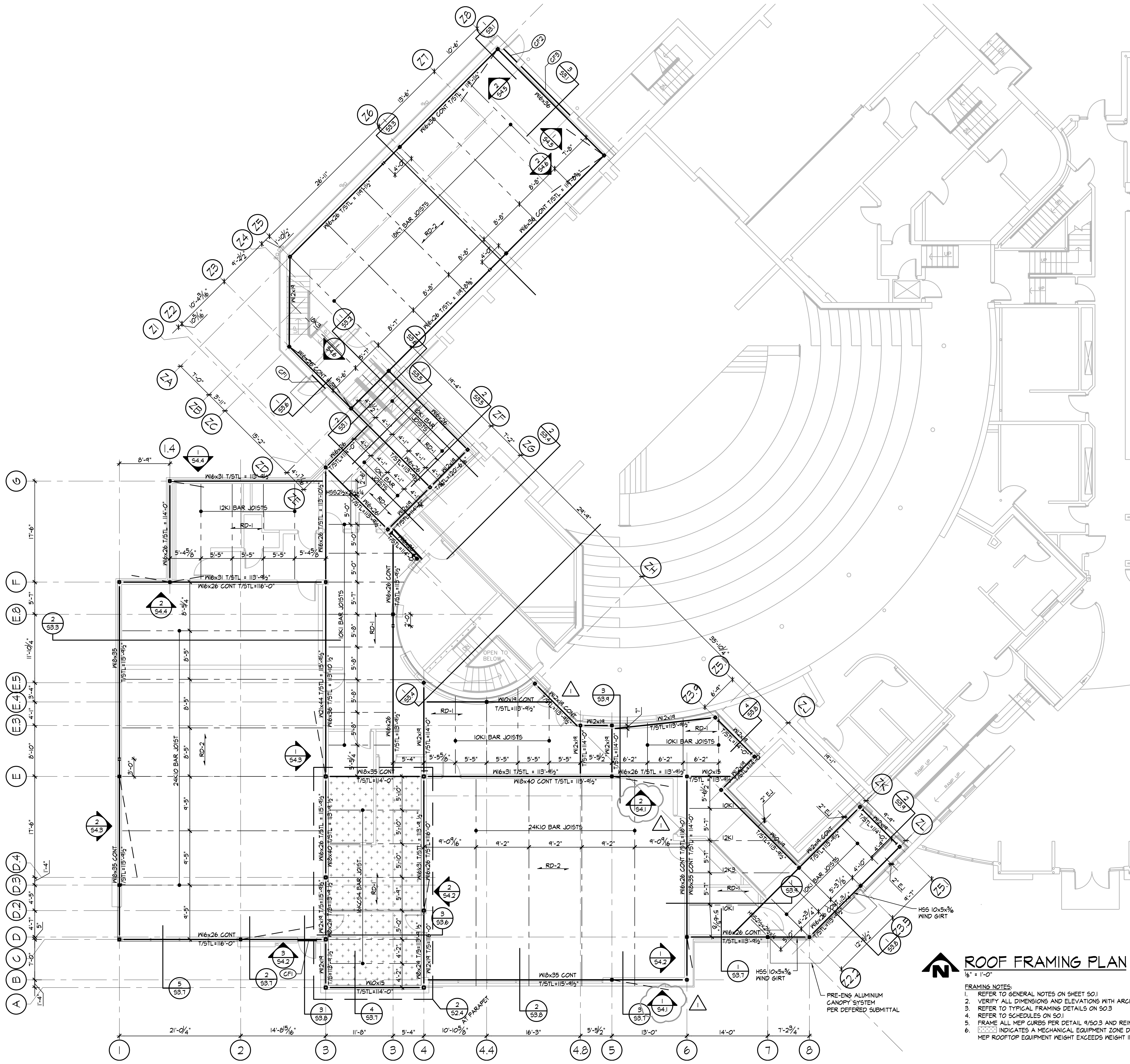


- FOUNDATION NOTES:**
1. REFER TO GENERAL NOTES ON SHEET 501.
 2. TOP OF FOOTING ELEVATION EQUALS 1'-0" BELOW FINISH FLOOR U.O.O. ON PLAN
 3. REFER TO S3.0 FOR TYPICAL FOUNDATION DETAILS NOT CUT ON PLAN
 4. REMOVE EXISTING SLAB ON GRADE AS REQUIRED TO PERFORM WORK AND REPAIR PER 1/5.0
 5. REFER TO SCHEDULES ON 501
 6. PROVIDE (6)5X3'-6" (3T, 3B) DRILL & EPOXY 12" INTO EXISTING FOOTING & PROJECT INTO NEW FOOTING. LOCATE 4' CLEAR FROM TOP/BOTTOM OF EXISTING FOOTING

- FRAMING NOTES:**
1. REFER TO GENERAL NOTES ON SHEET SO.1
 2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS
 3. REFER TO TYPICAL FRAMING DETAILS ON SO.3
 4. REFER TO SCHEDULES ON SO.1
 5. FRAME ALL MEP CURBS PER DETAIL 9/50.3 AND REINFORCE BAR JOISTS FOR POINT LOADS PER 8/50.3
 6.  INDICATES A MECHANICAL EQUIPMENT ZONE DESIGNED FOR A 500psf EQUIPMENT WEIGHT. NOTIFY EOR IF MEP ROOFTOP EQUIPMENT WEIGHT EXCEEDS WEIGHT INDICATED OR IF SIZE OR LOCATION CHANGES

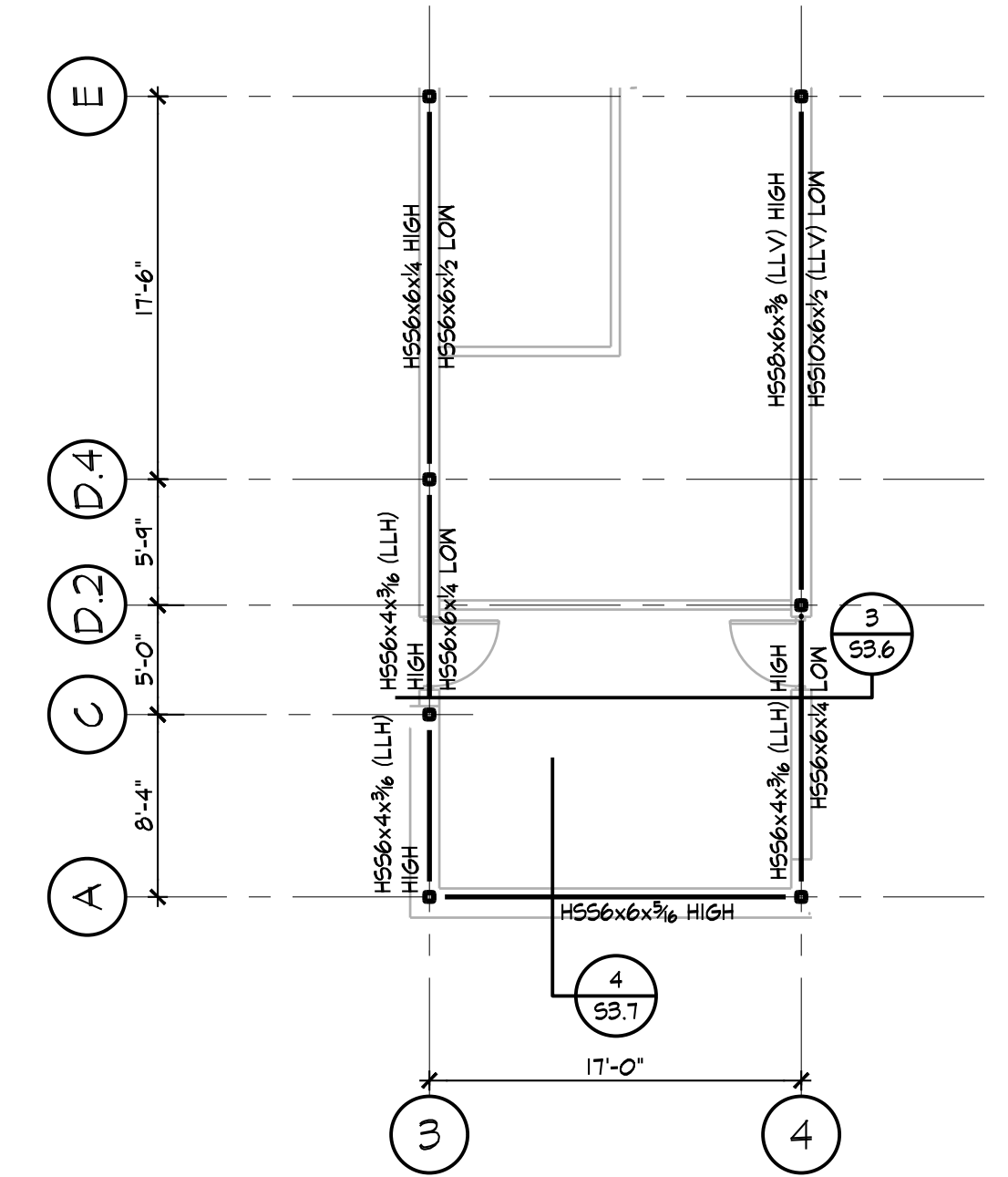


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HIGH ROOF FRAMING PLAN
2 52.4 1/8" = 1'-0"

NOTES:
1. T/STL AT HIGH HSS = 122'-0"
2. B/STL AT LOW HSS = 118'-0"



ROOF FRAMING PLAN
1/8" = 1'-0"

FRAMING NOTES:
1. REFER TO GENERAL NOTES ON SHEET 50.1
2. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS AND EXISTING CONDITIONS
3. REFER TO TYPICAL FRAMING DETAILS ON 50.3
4. REFER TO SCHEDULES ON 50.1
5. FRAME ALL MEP CURBS PER DETAIL 9/50.3 AND REINFORCE BAR JOISTS FOR POINT LOADS PER 8/50.3
6. [Hatched Area] INDICATES A MECHANICAL EQUIPMENT ZONE DESIGNED FOR A 50psf EQUIPMENT WEIGHT. NOTIFY EOR IF MEP ROOFTOP EQUIPMENT WEIGHT EXCEEDS WEIGHT INDICATED OR IF SIZE OR LOCATION CHANGES

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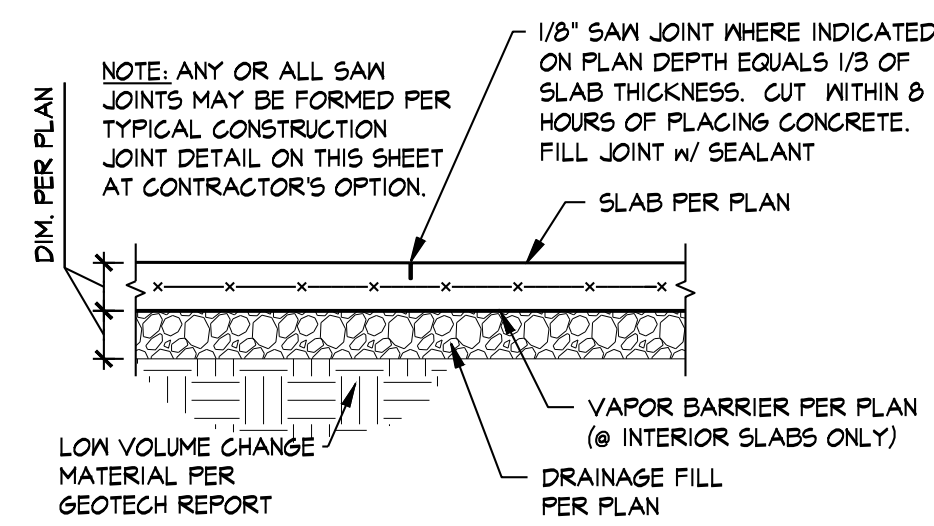
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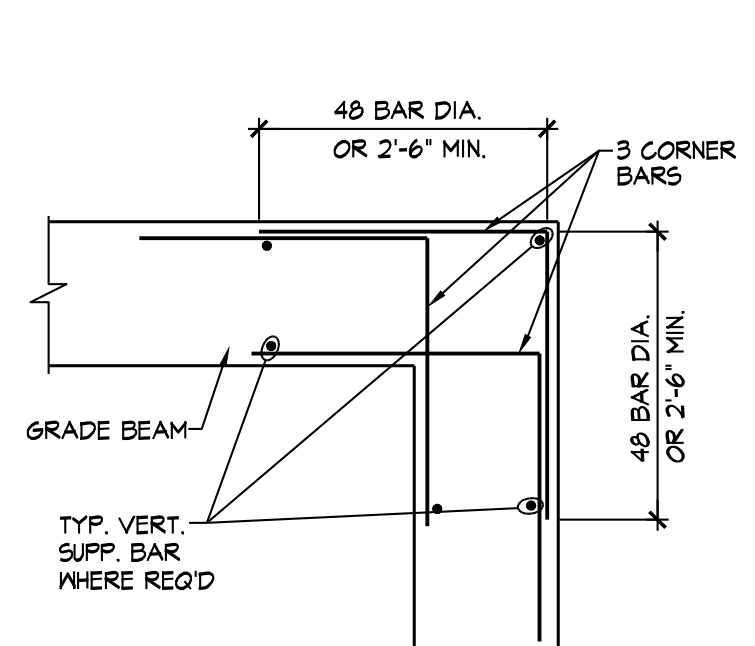
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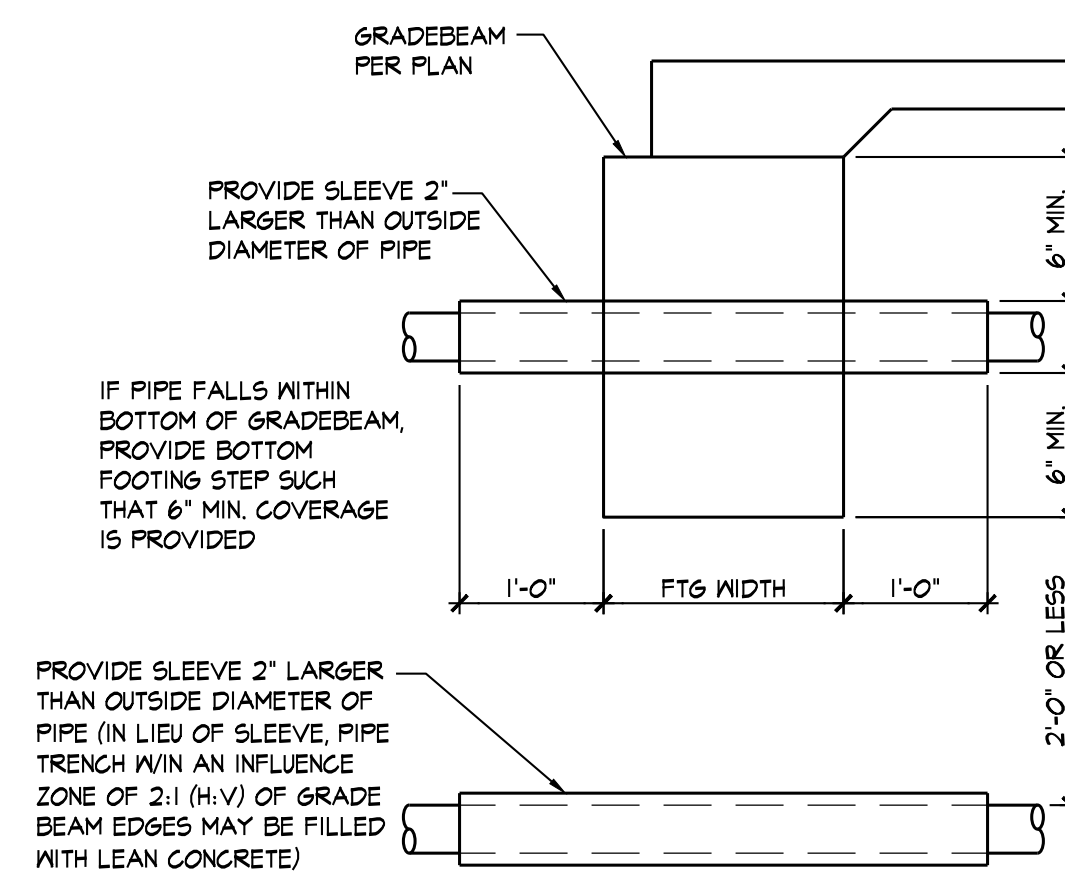


TYPICAL SAW JOINT
NOTED "SJ" ON PLAN

SECTION 1
3/4" = 1'-0" (53.0)

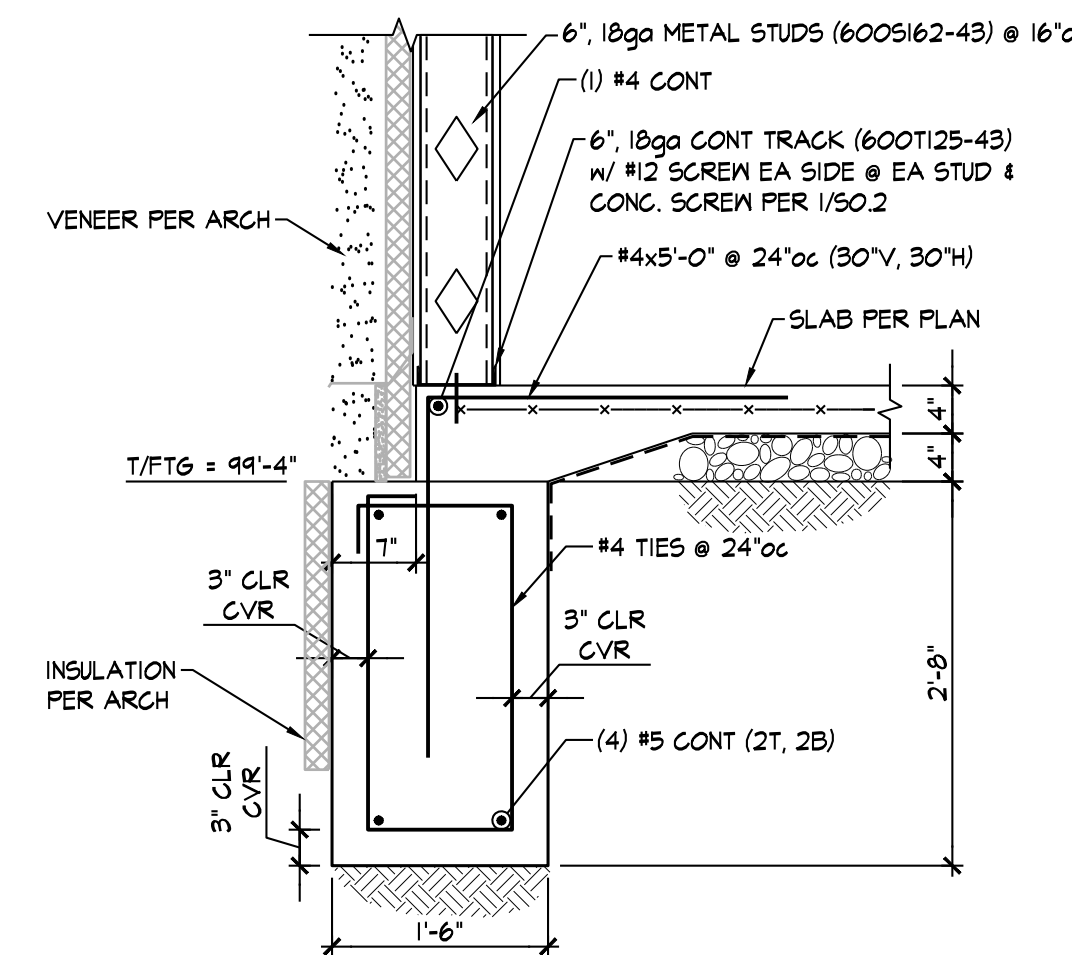


TYP. CORNER BARS

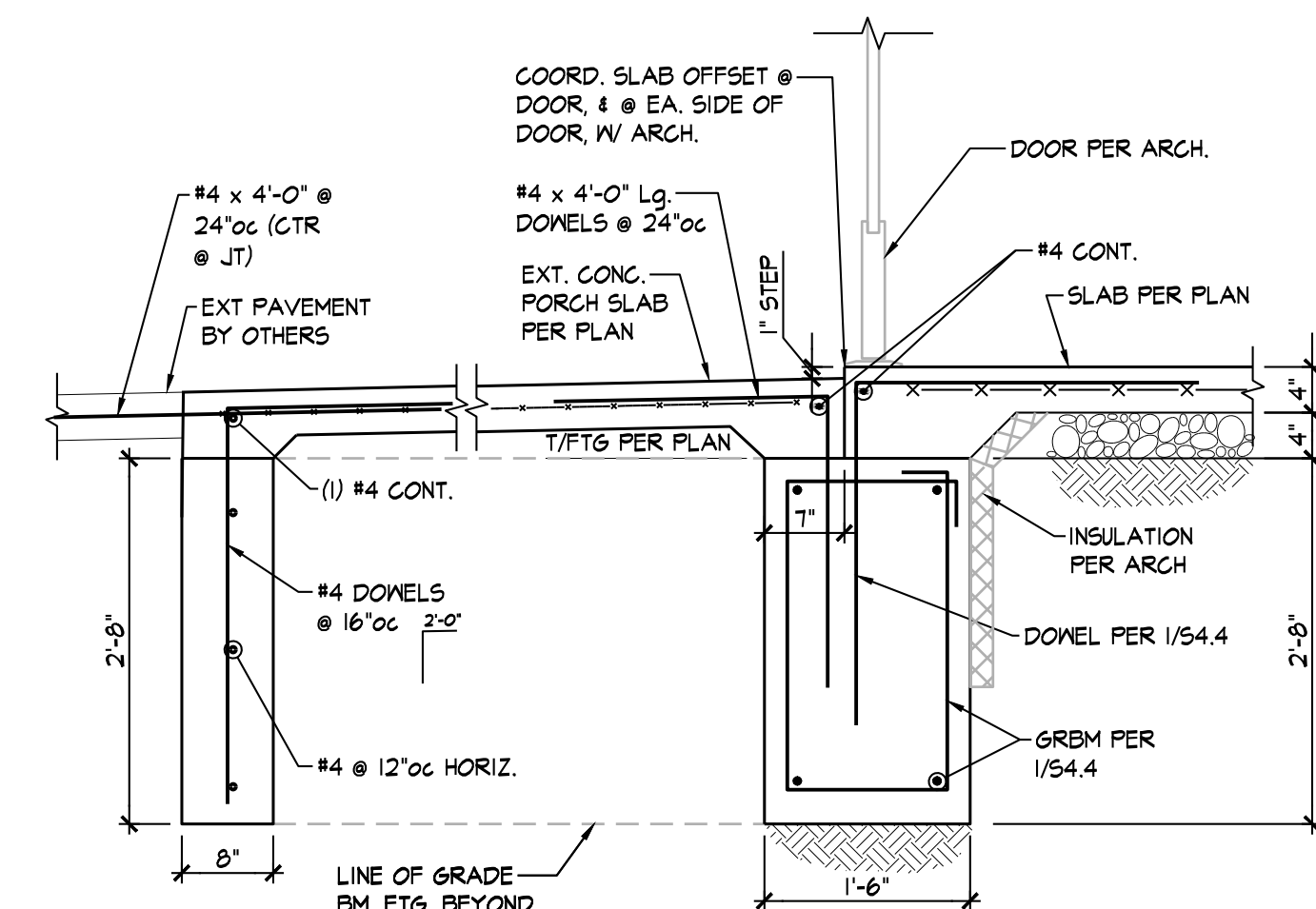


TYPICAL GRADEBEAM SLEEVE

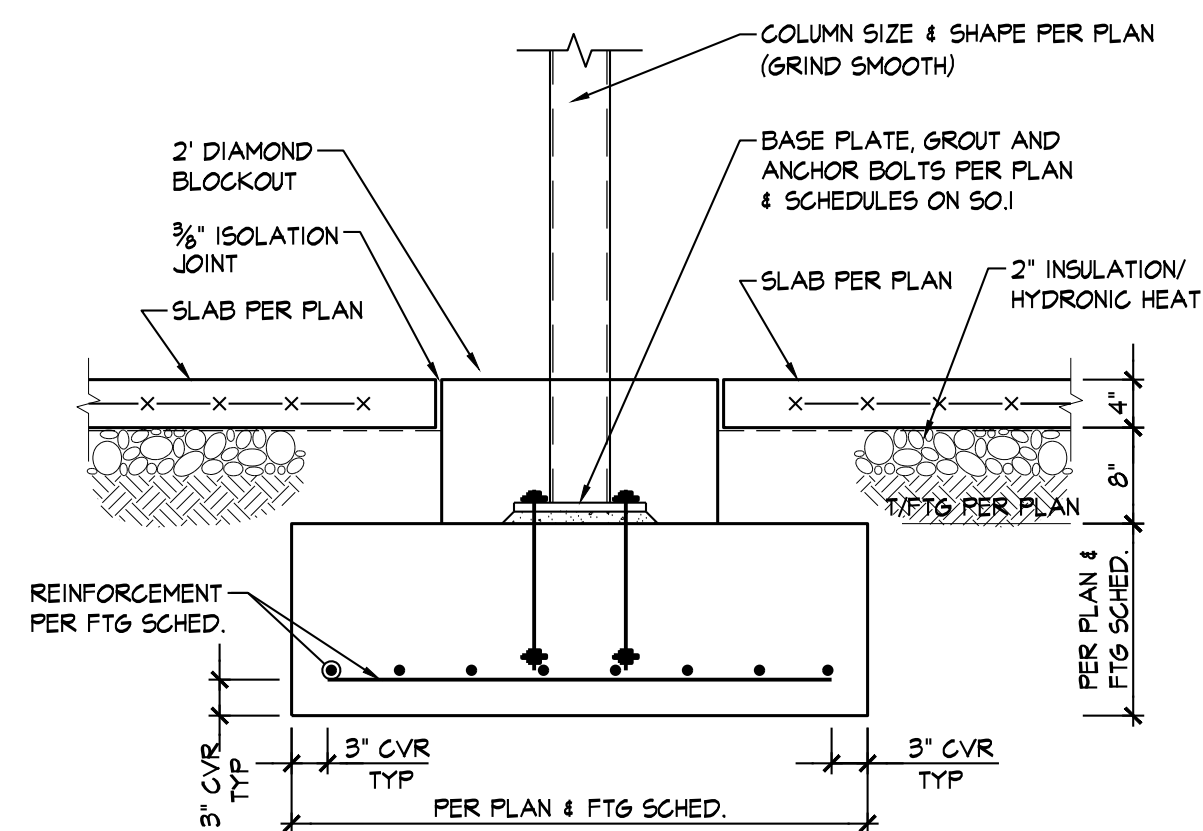
SECTION 3
3/4" = 1'-0" (53.0)



SECTION 4
3/4" = 1'-0" (53.0)

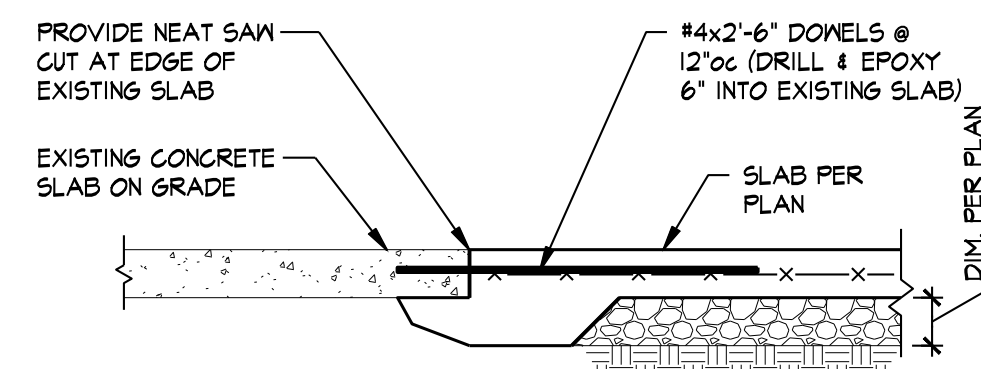


SECTION 5
3/4" = 1'-0" (53.0)



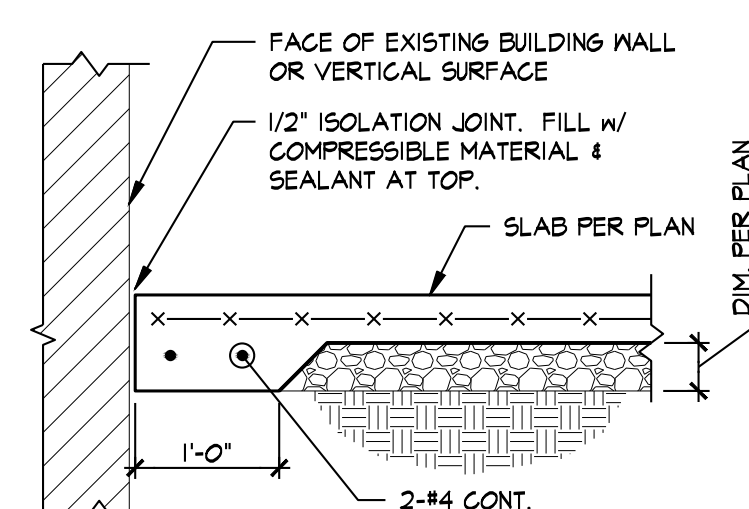
TYPICAL STEEL COLUMN SPREAD FOOTING

SECTION 6
3/4" = 1'-0" (53.0)



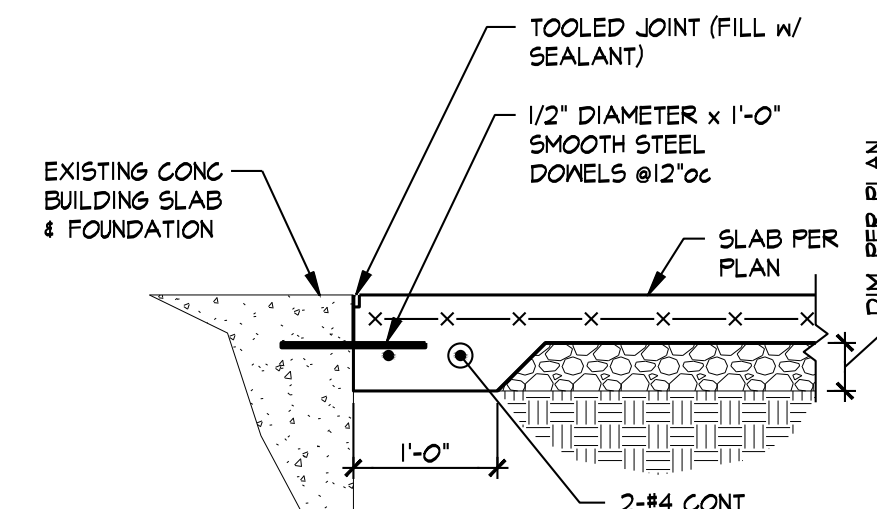
TYPICAL AT NEW-TO-EXISTING
SLAB ON GRADE

SECTION 7
3/4" = 1'-0" (53.0)



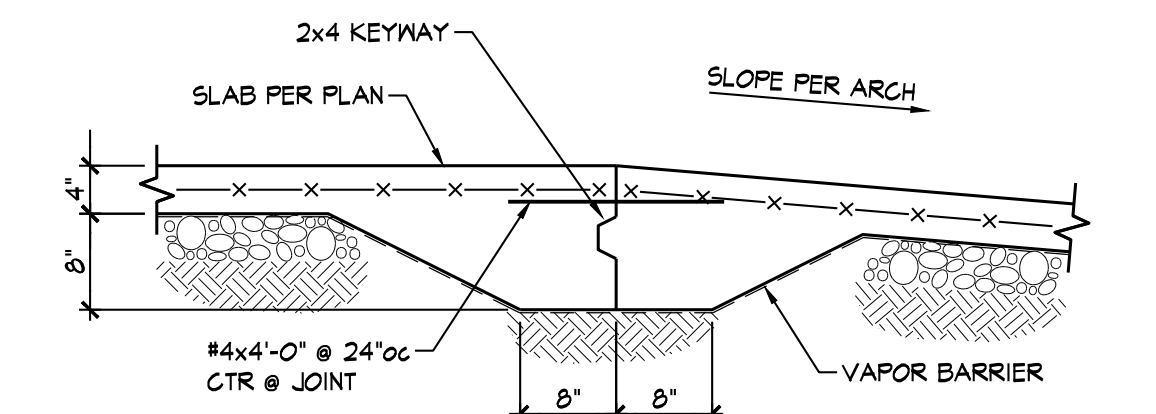
TYPICAL SLAB EDGE DETAIL AGAINST EXISTING
BUILDING WALL OR VERTICAL SURFACE

SECTION 8
3/4" = 1'-0" (53.0)

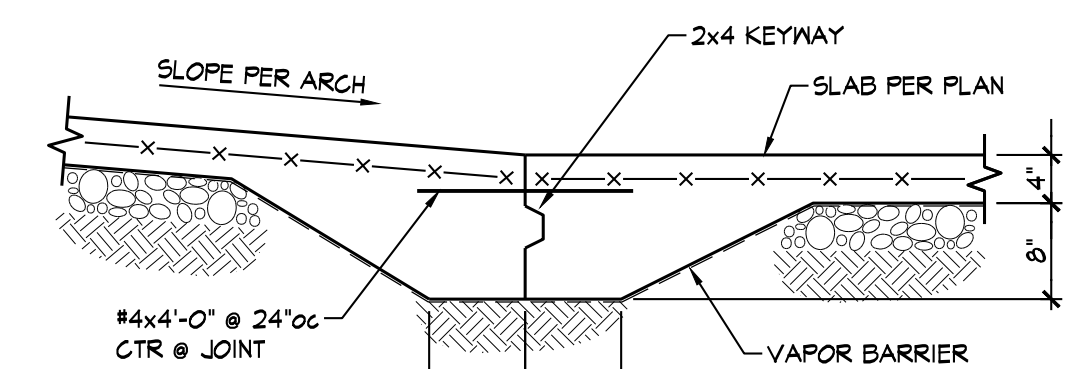


TYPICAL SLAB EDGE DETAIL AT
EXISTING BUILDING SLAB THRESHOLD

SECTION 9
3/4" = 1'-0" (53.0)

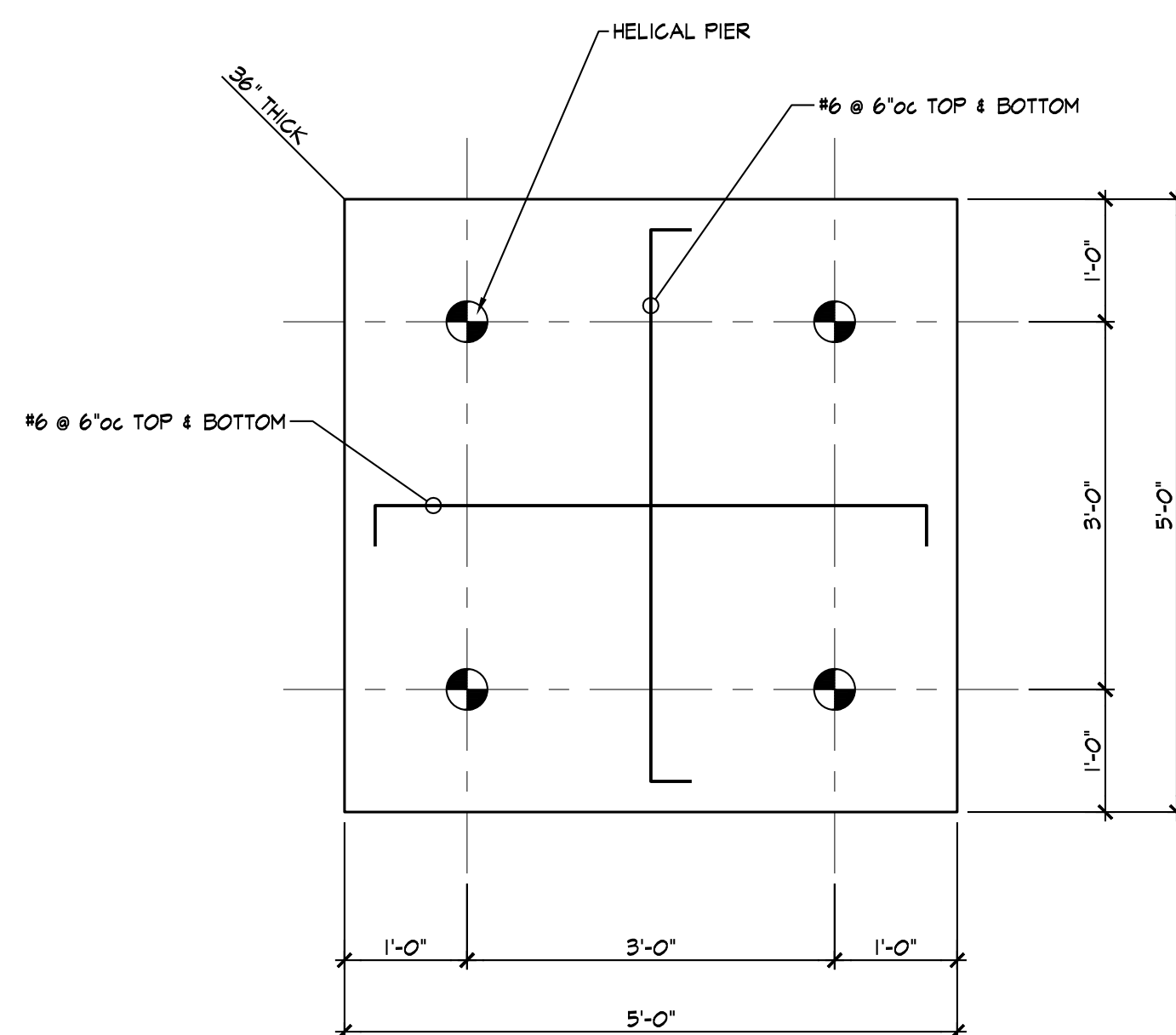


TOP OF RAMP



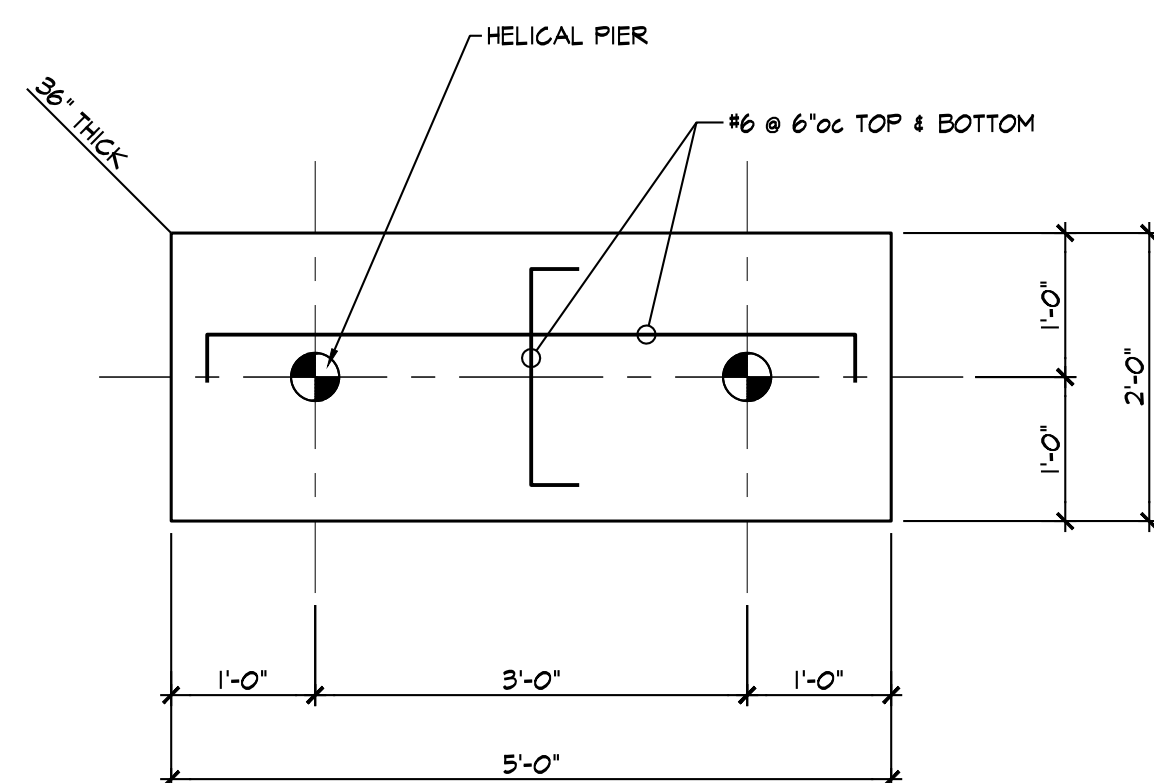
BOTTOM OF RAMP

SECTION 10
3/4" = 1'-0" (53.0)



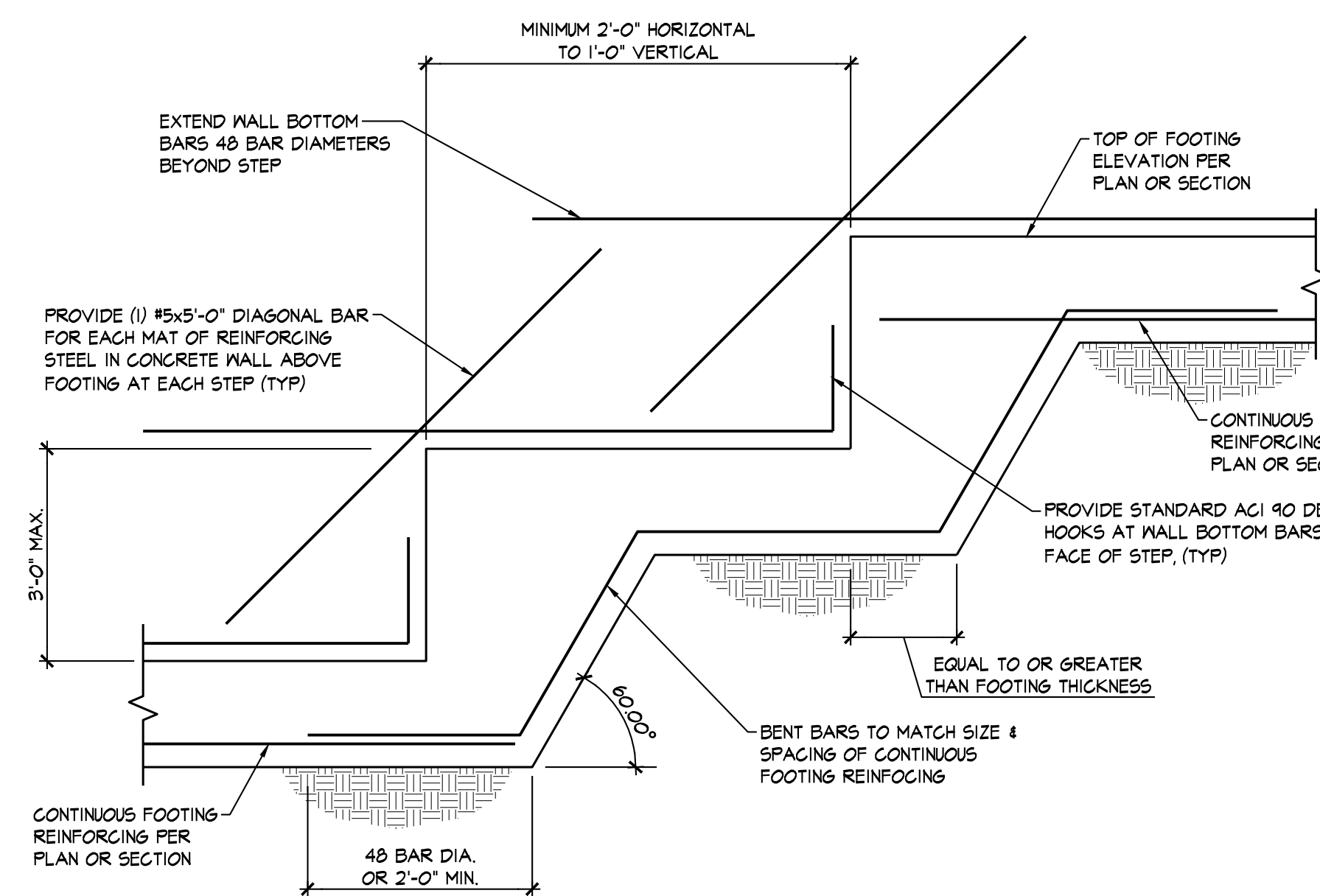
PILE CAP PLAN - 5'-0"x5'-0"x3'-0"

- NOTE:
1) POUR PILE CAP MONOLITHIC W/ ADJACENT GRADE BEAM
2) PILE CAP IS CENTERED AT COLUMN ABOVE UNLESS NOTED OTHERWISE ON PLAN



PILE CAP PLAN - 2'-0"x5'-0"x3'-0"

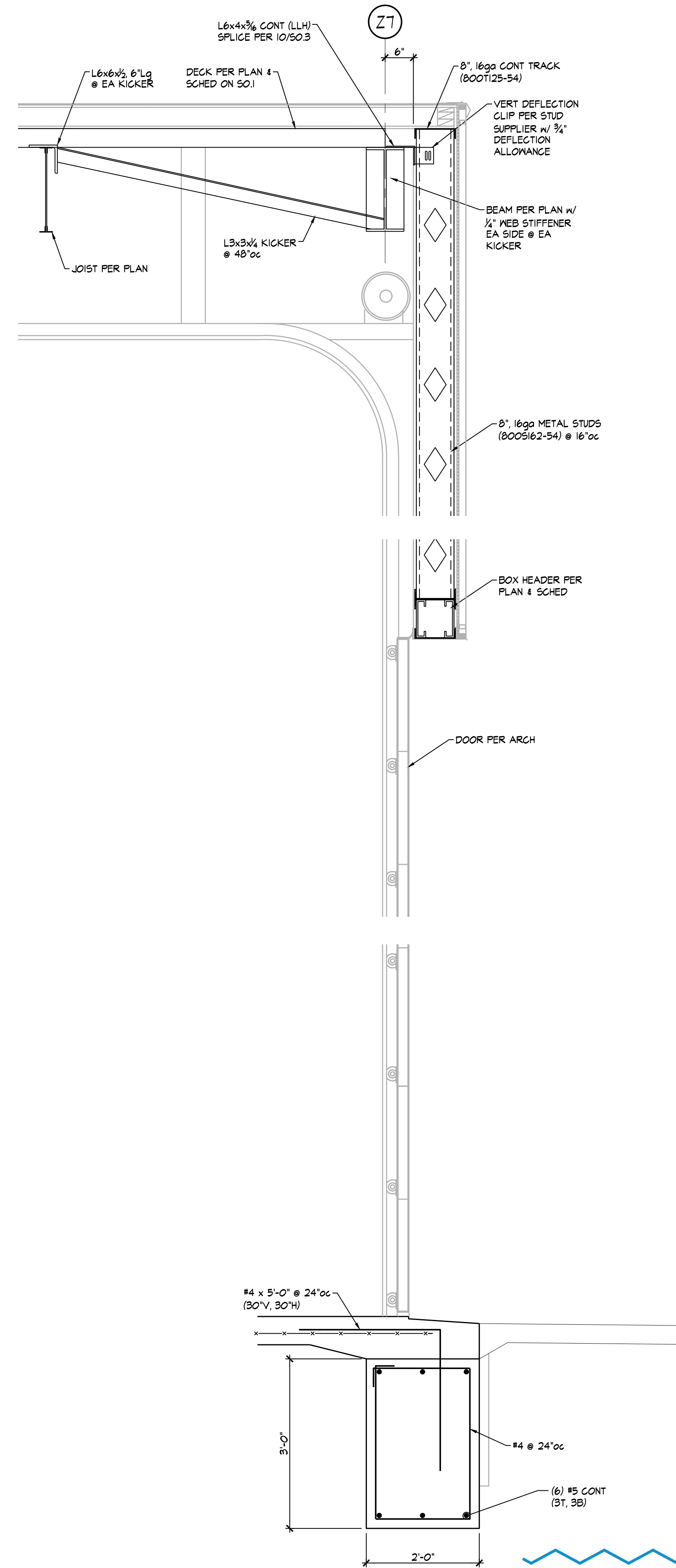
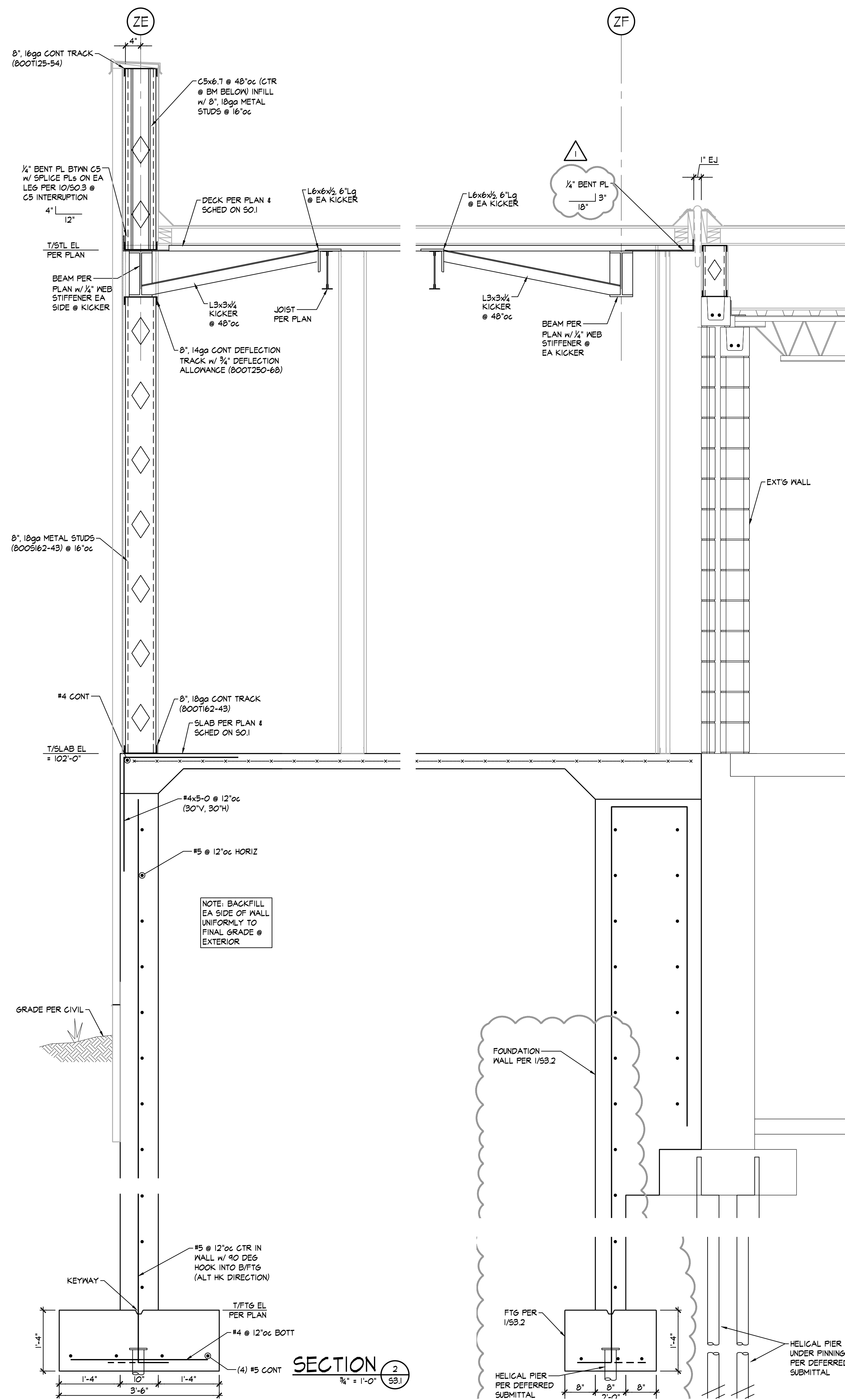
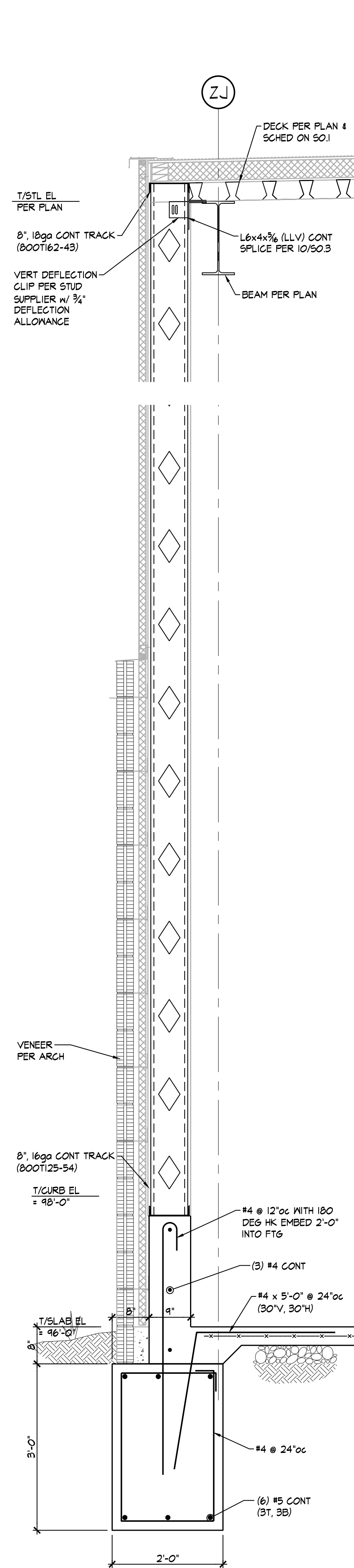
- NOTE:
1) POUR PILE CAP MONOLITHIC W/ ADJACENT GRADE BEAM
2) PILE CAP IS CENTERED AT COLUMN ABOVE UNLESS NOTED OTHERWISE ON PLAN



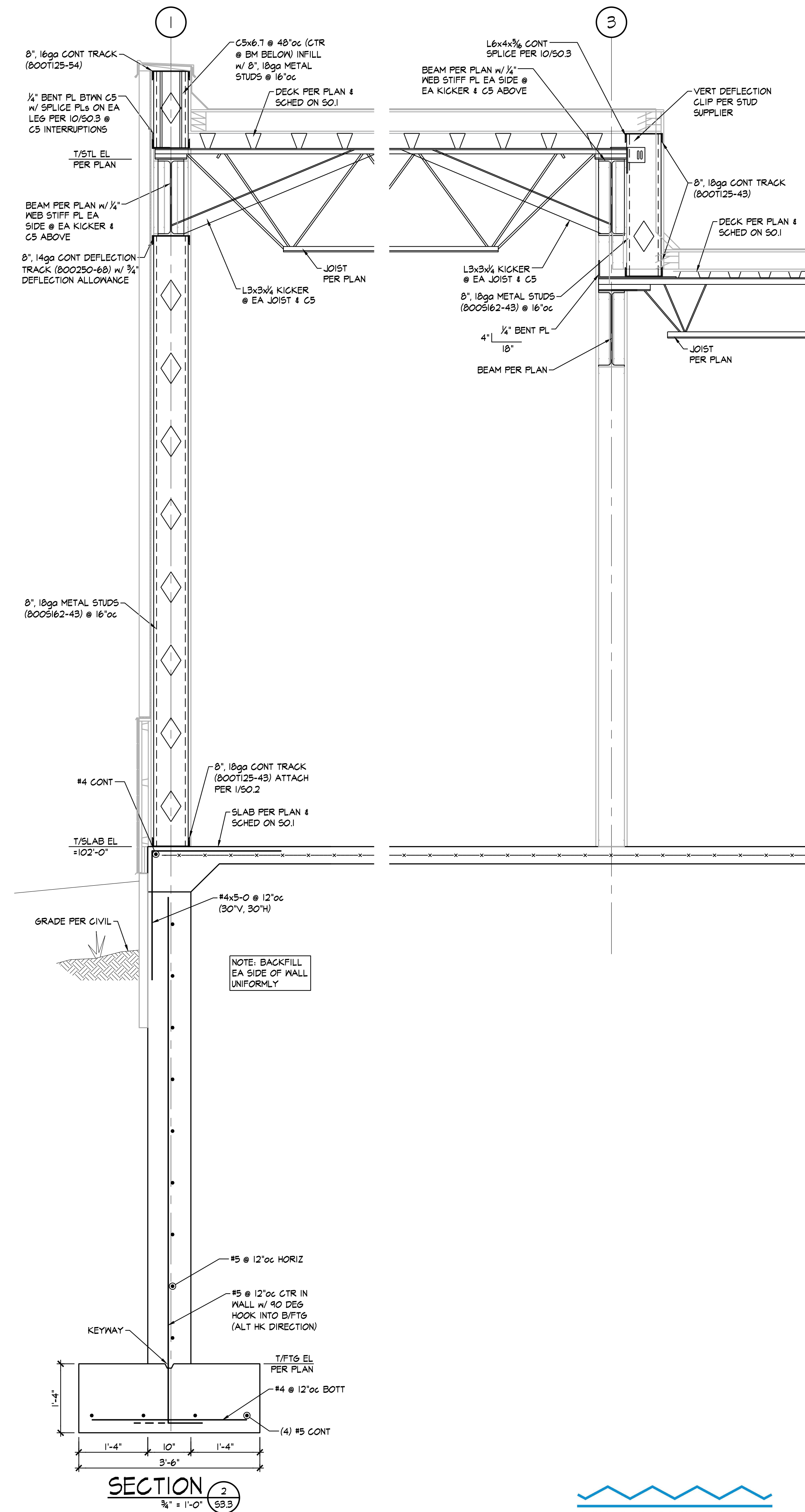
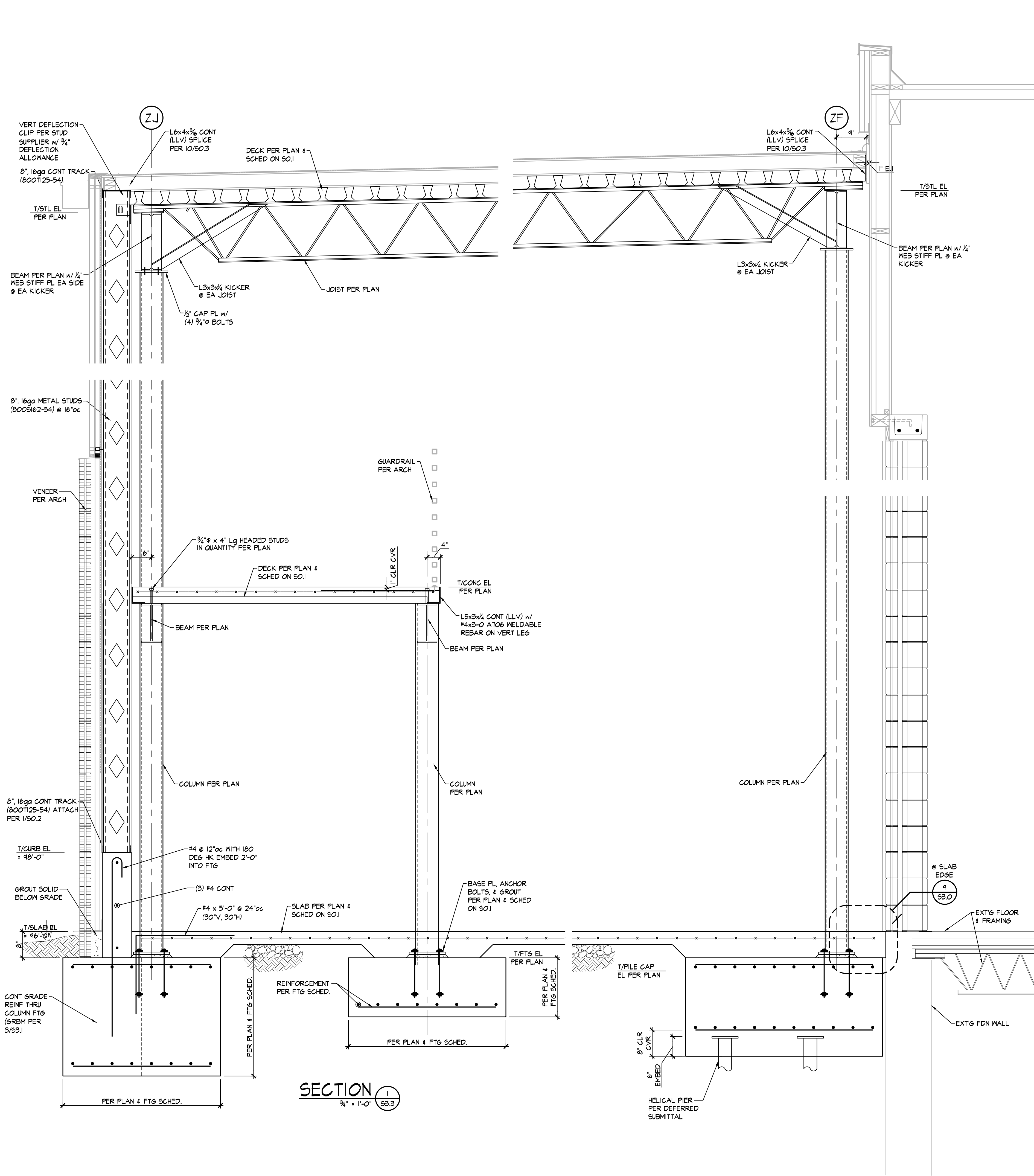
TYPICAL FOOTING STEP

SECTION 13
3/4" = 1'-0" (53.0)

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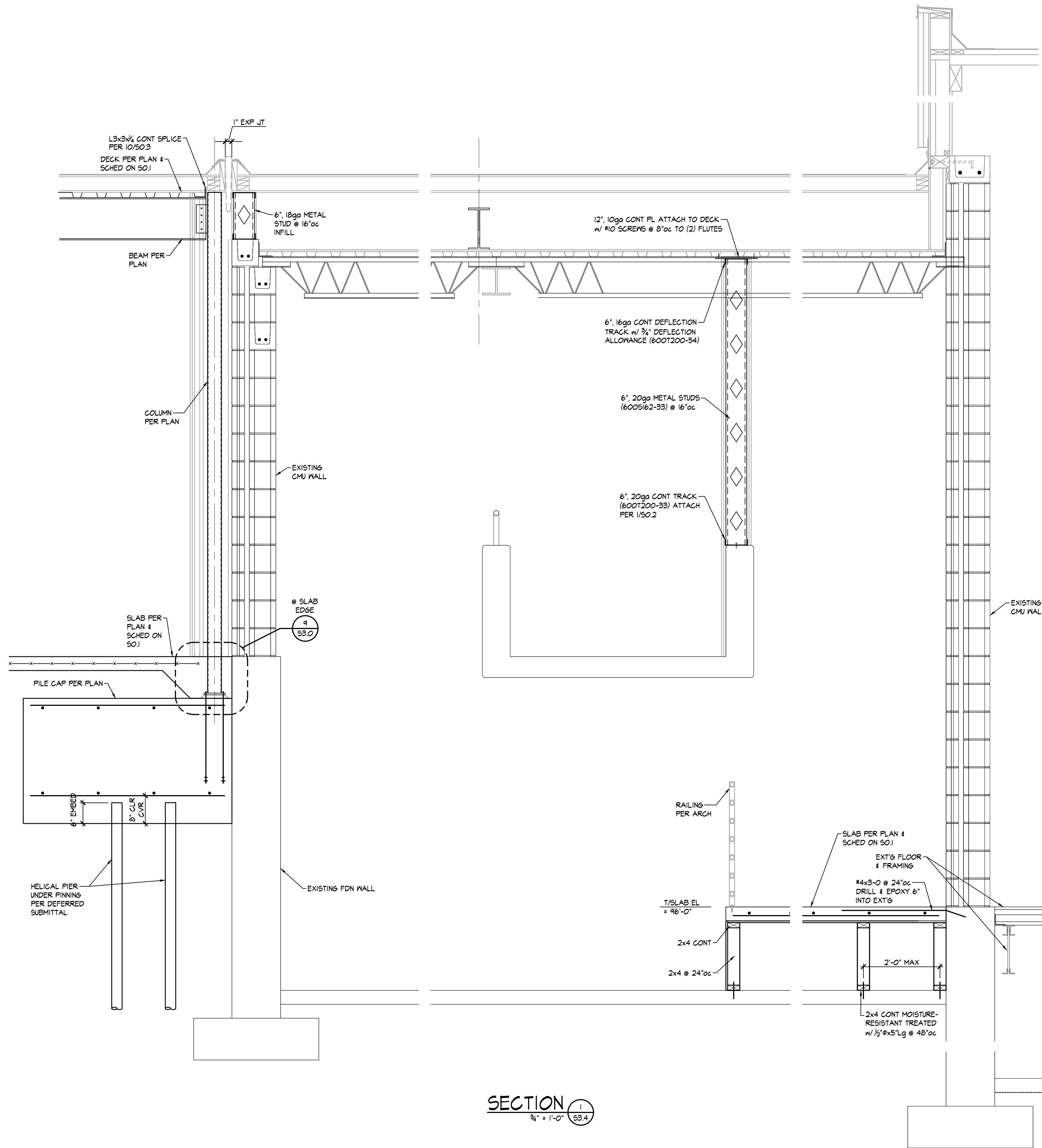




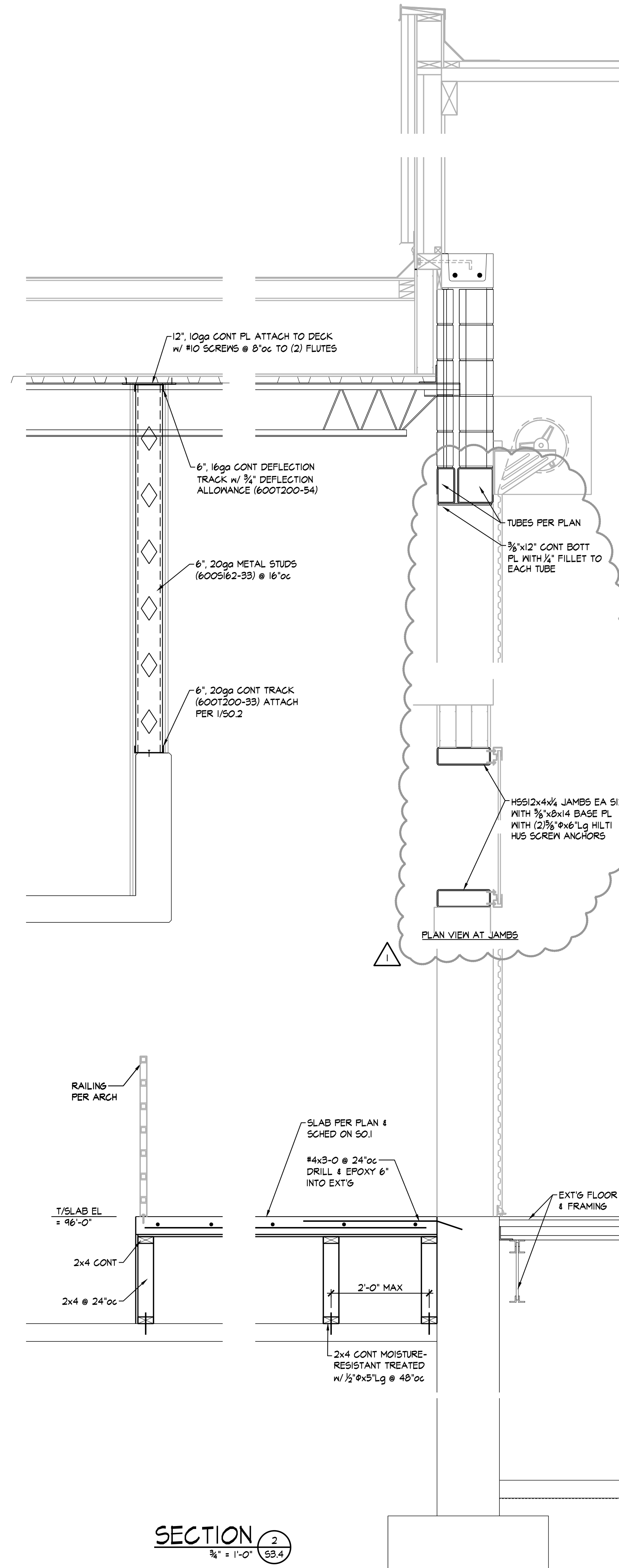
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SECTION 1
3/4" = 1'-0" S3.4



SECTION 2
3/4" = 1'-0" S3.4

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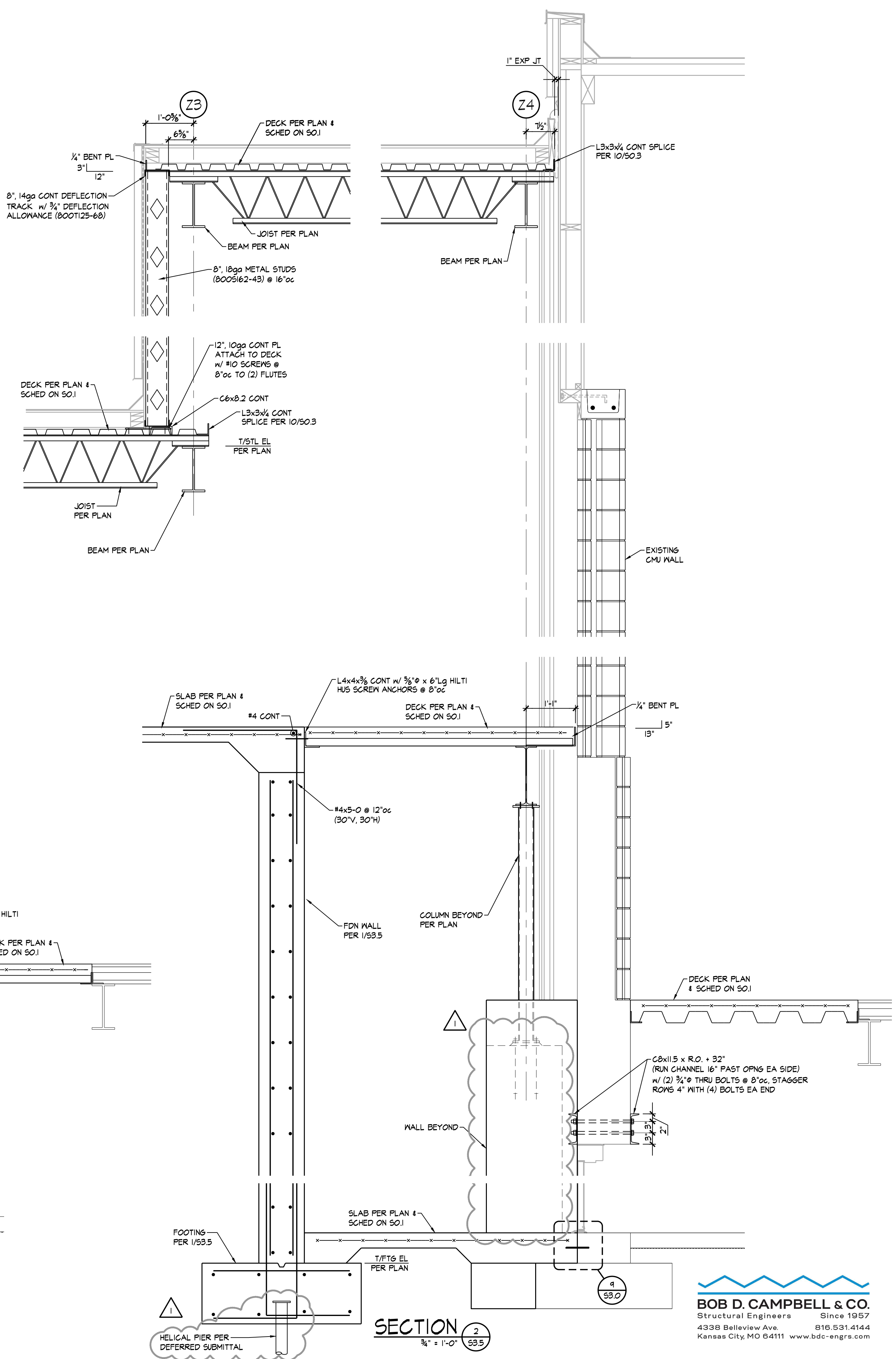
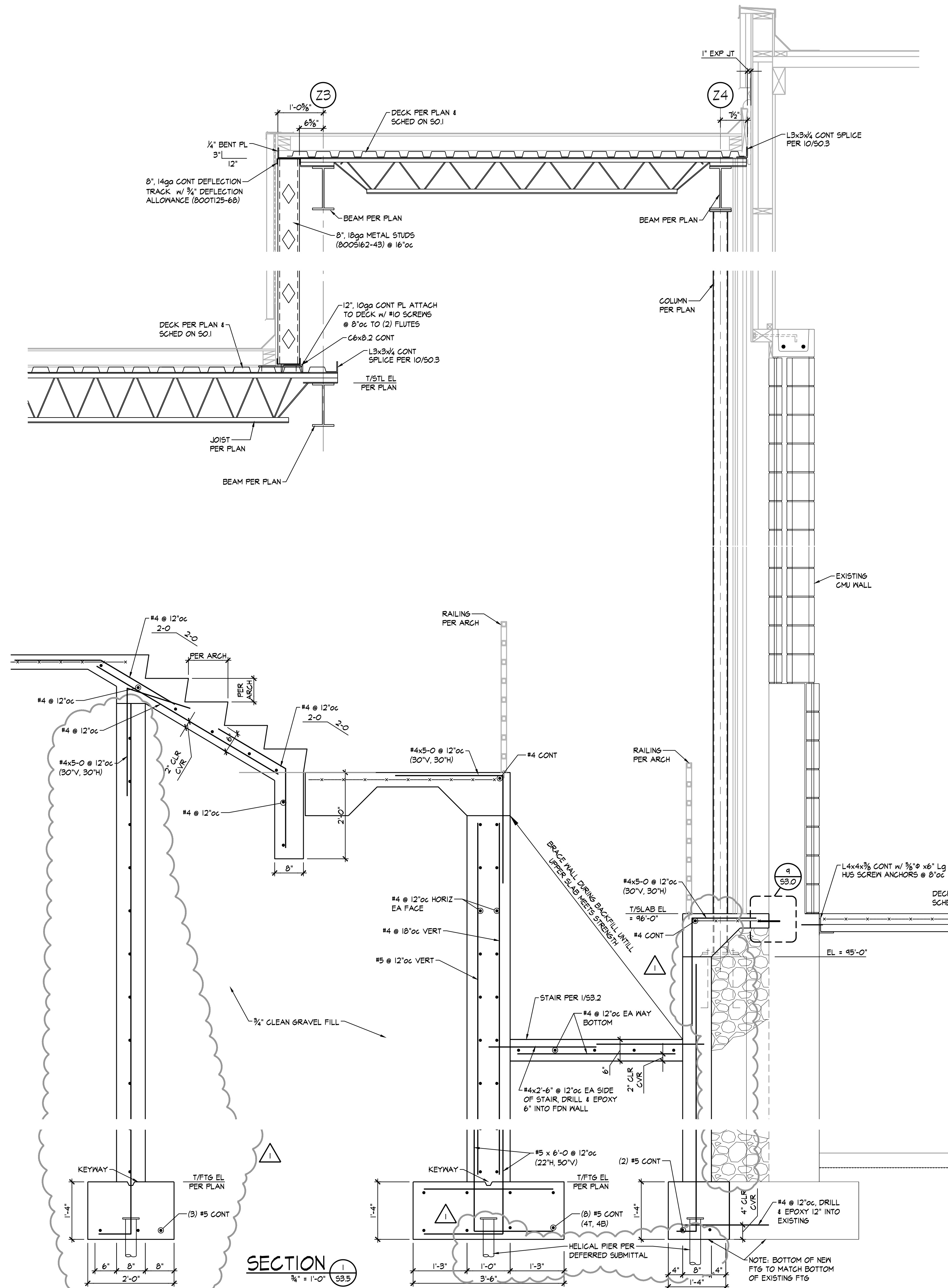
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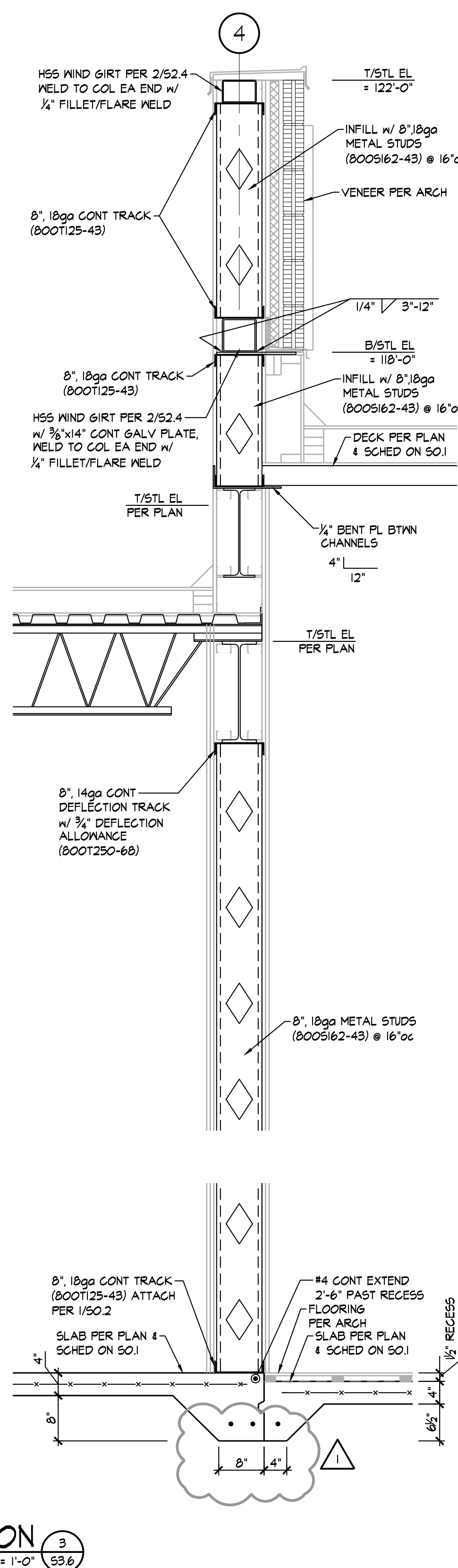
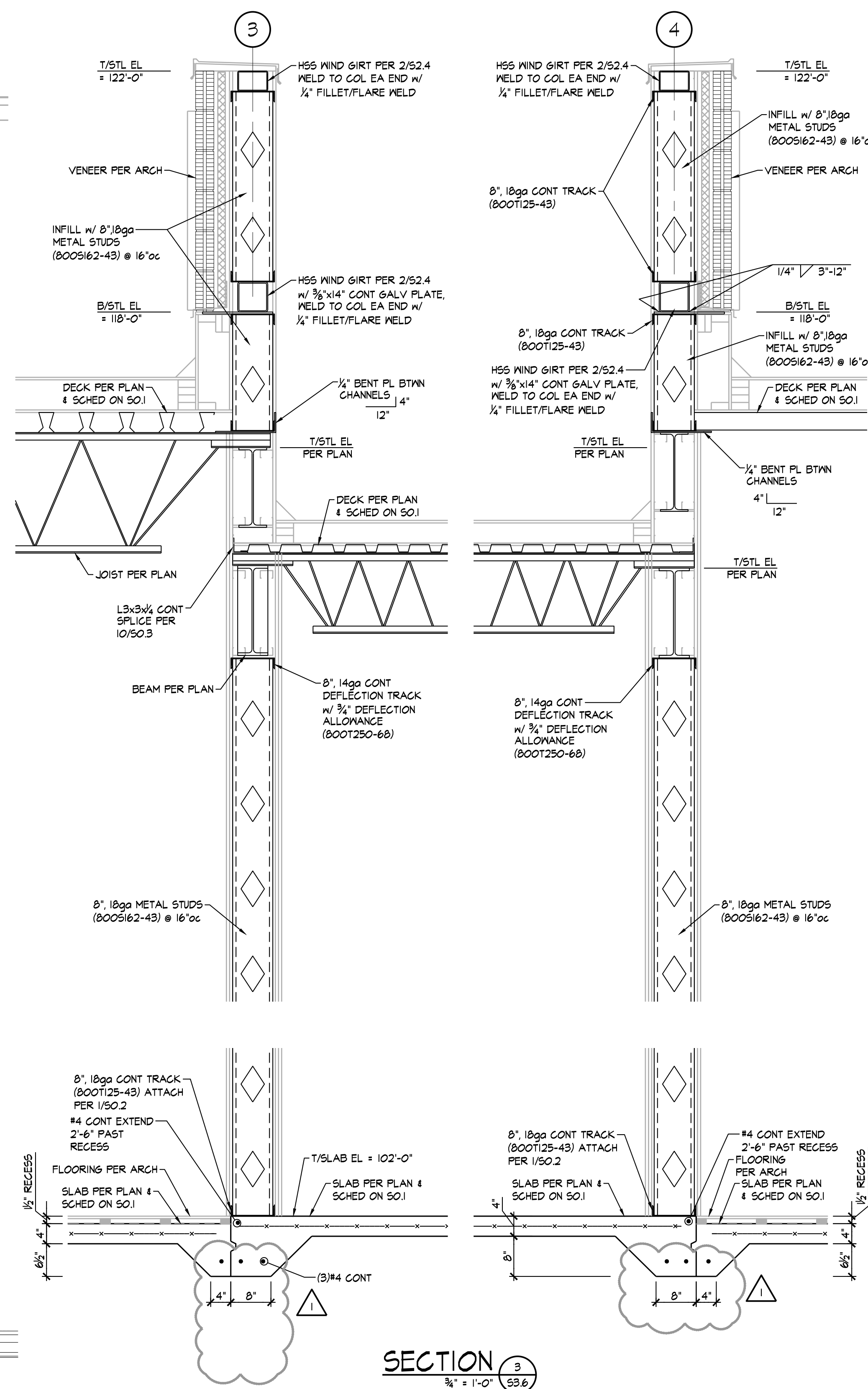
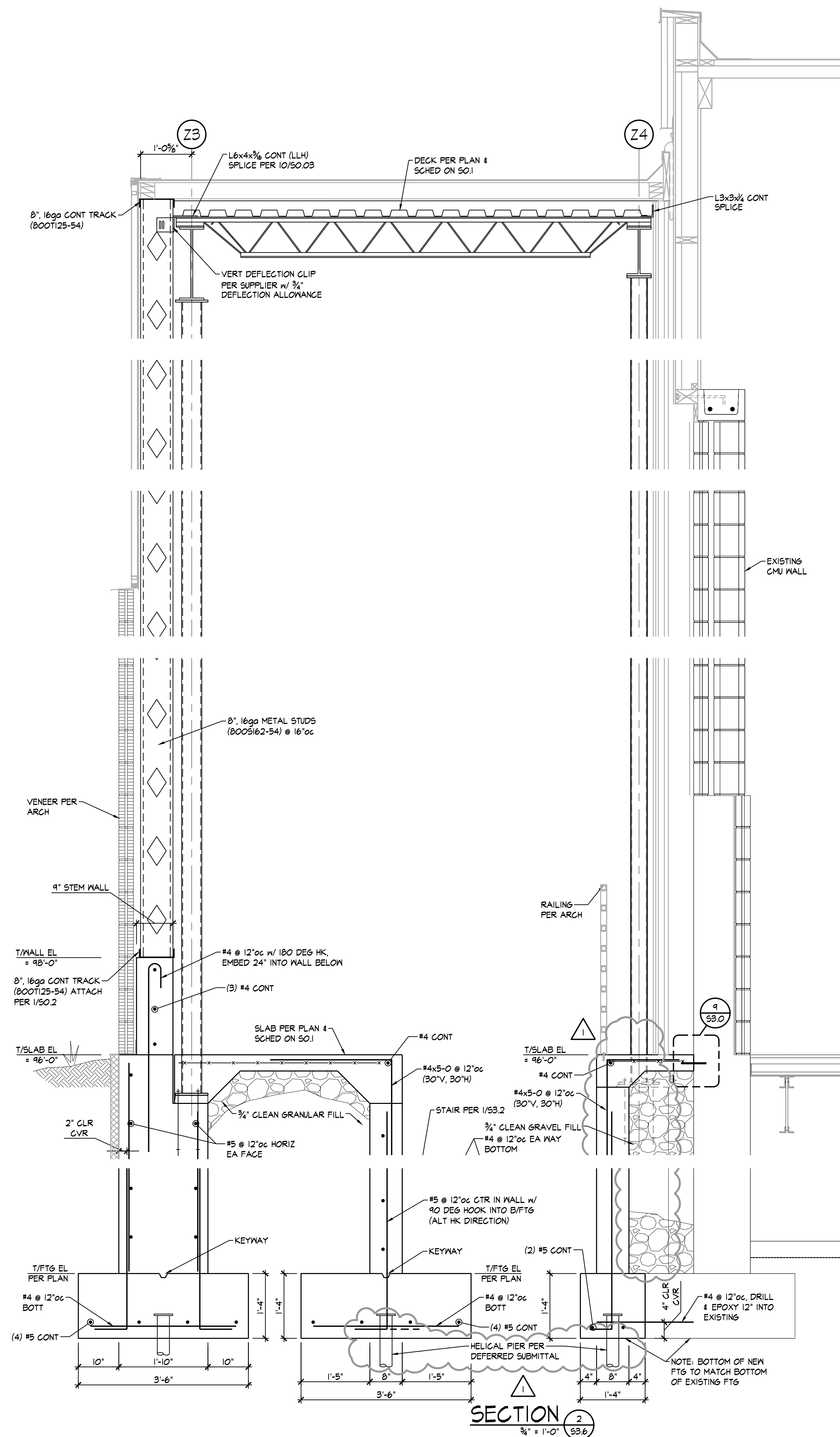
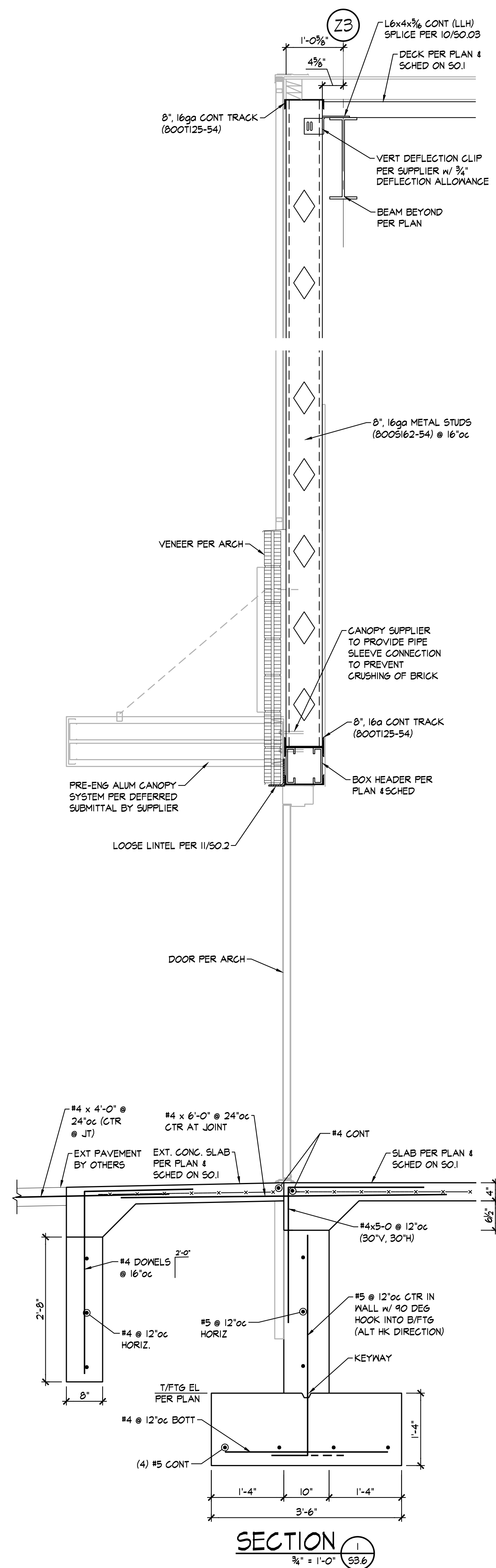
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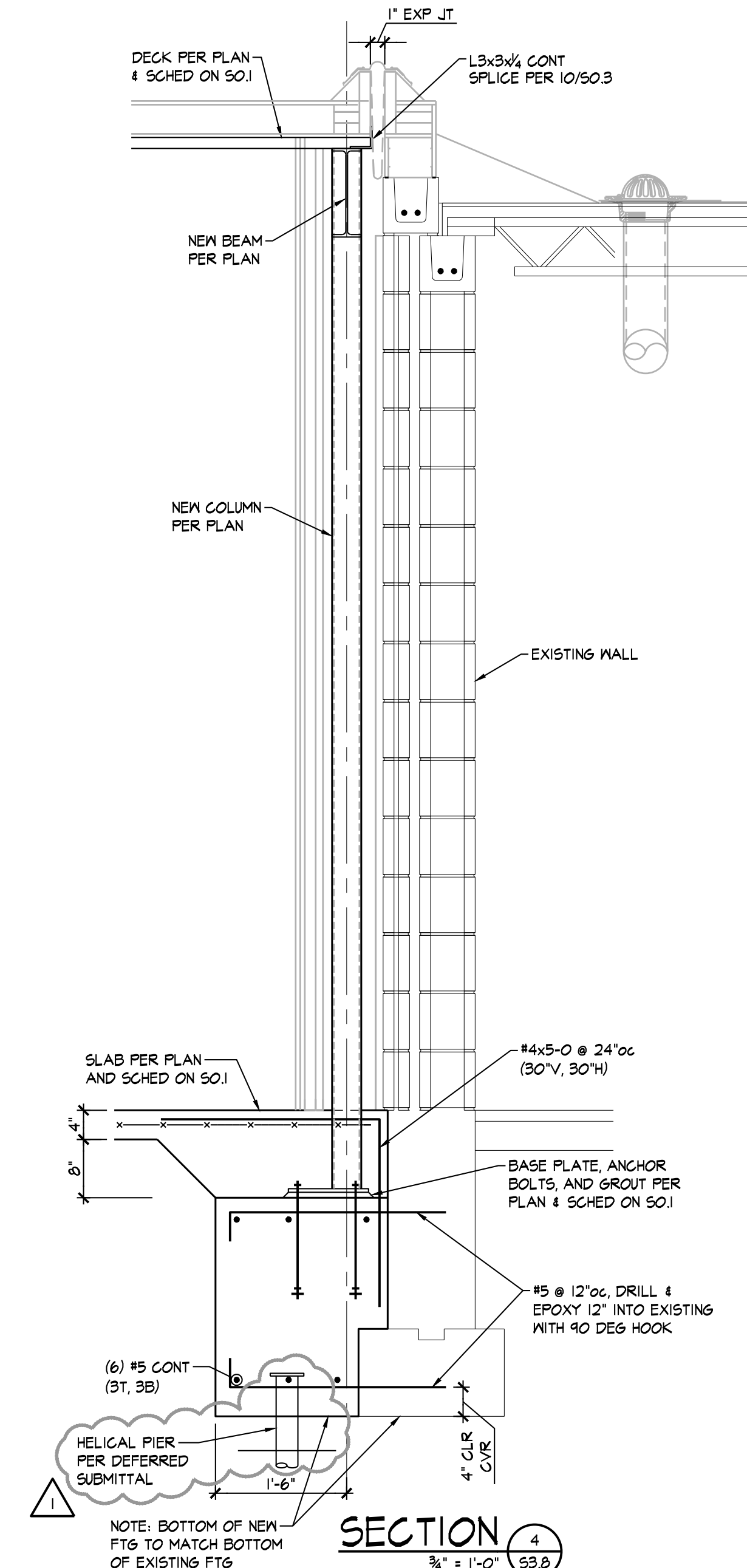
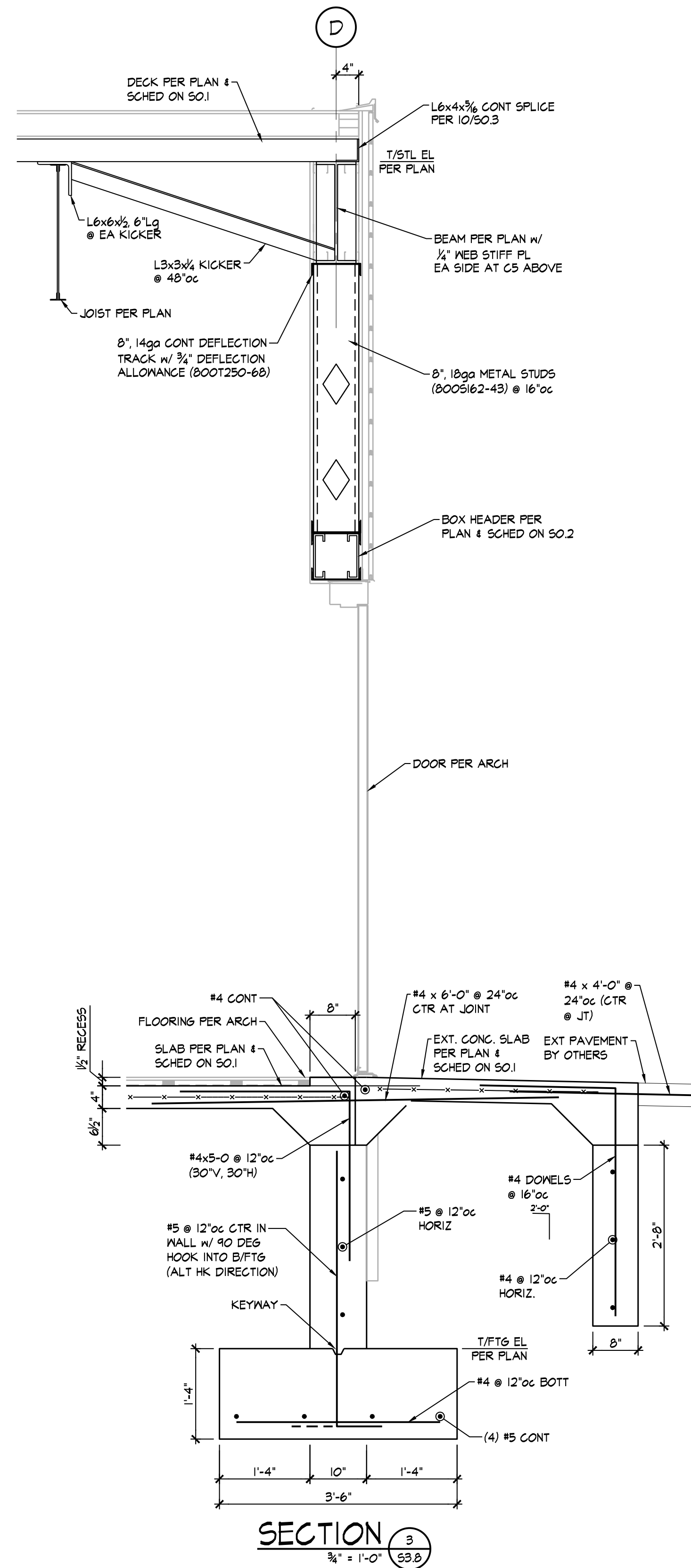
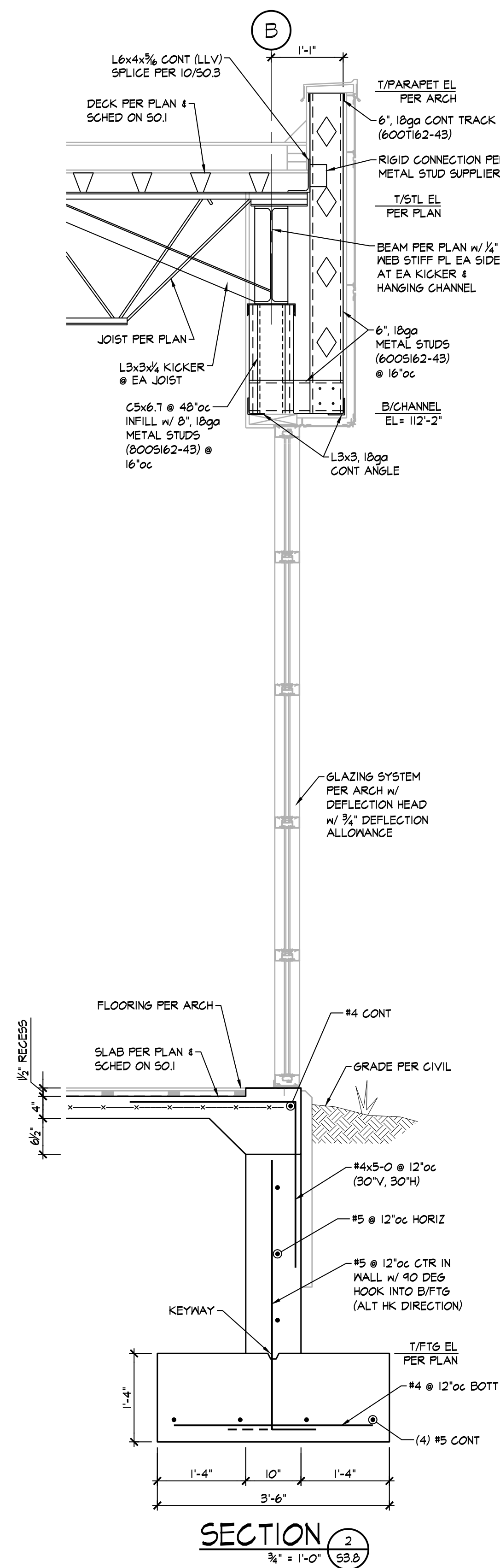
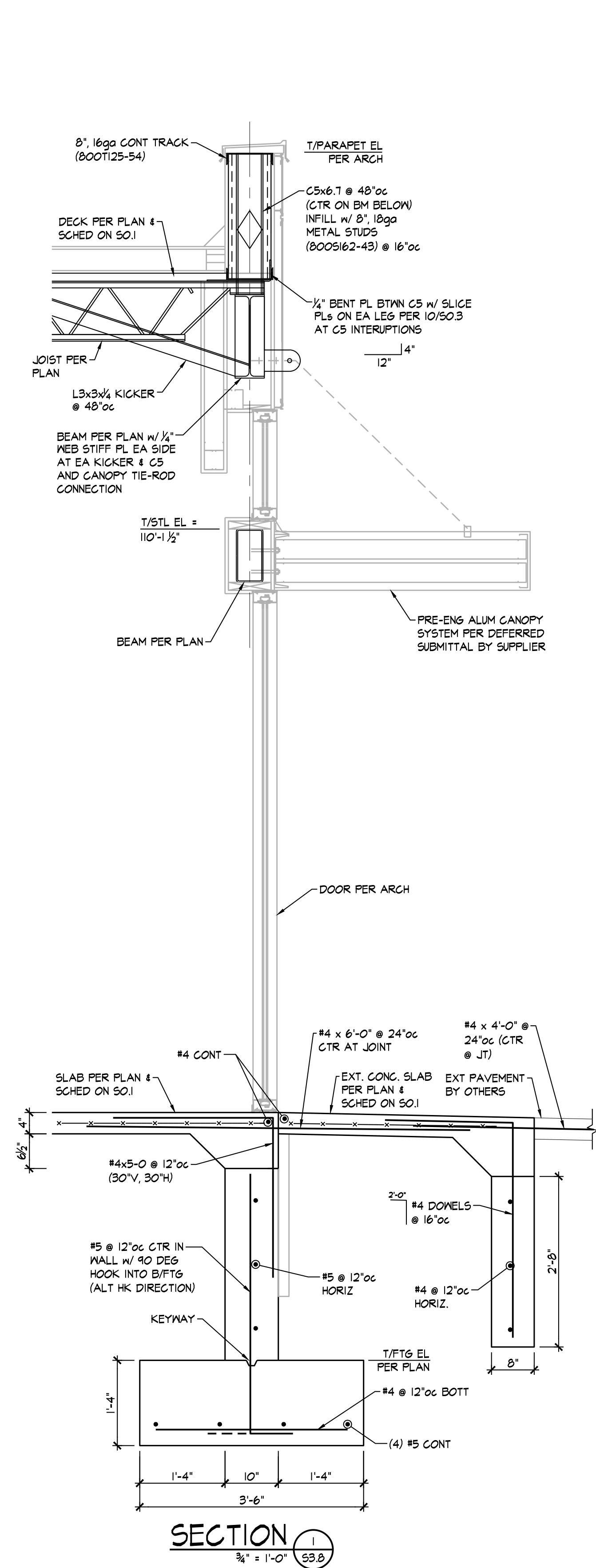
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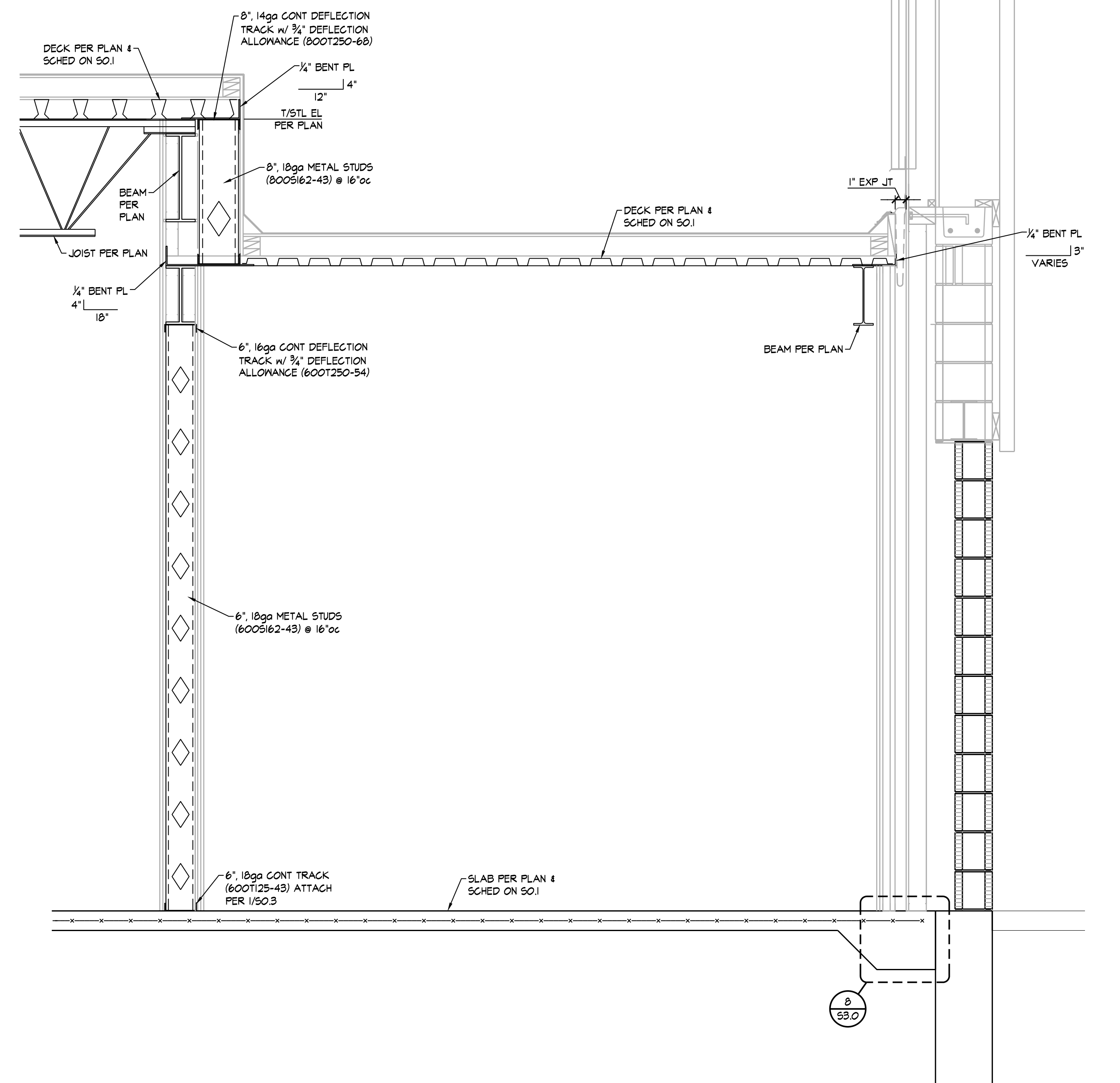
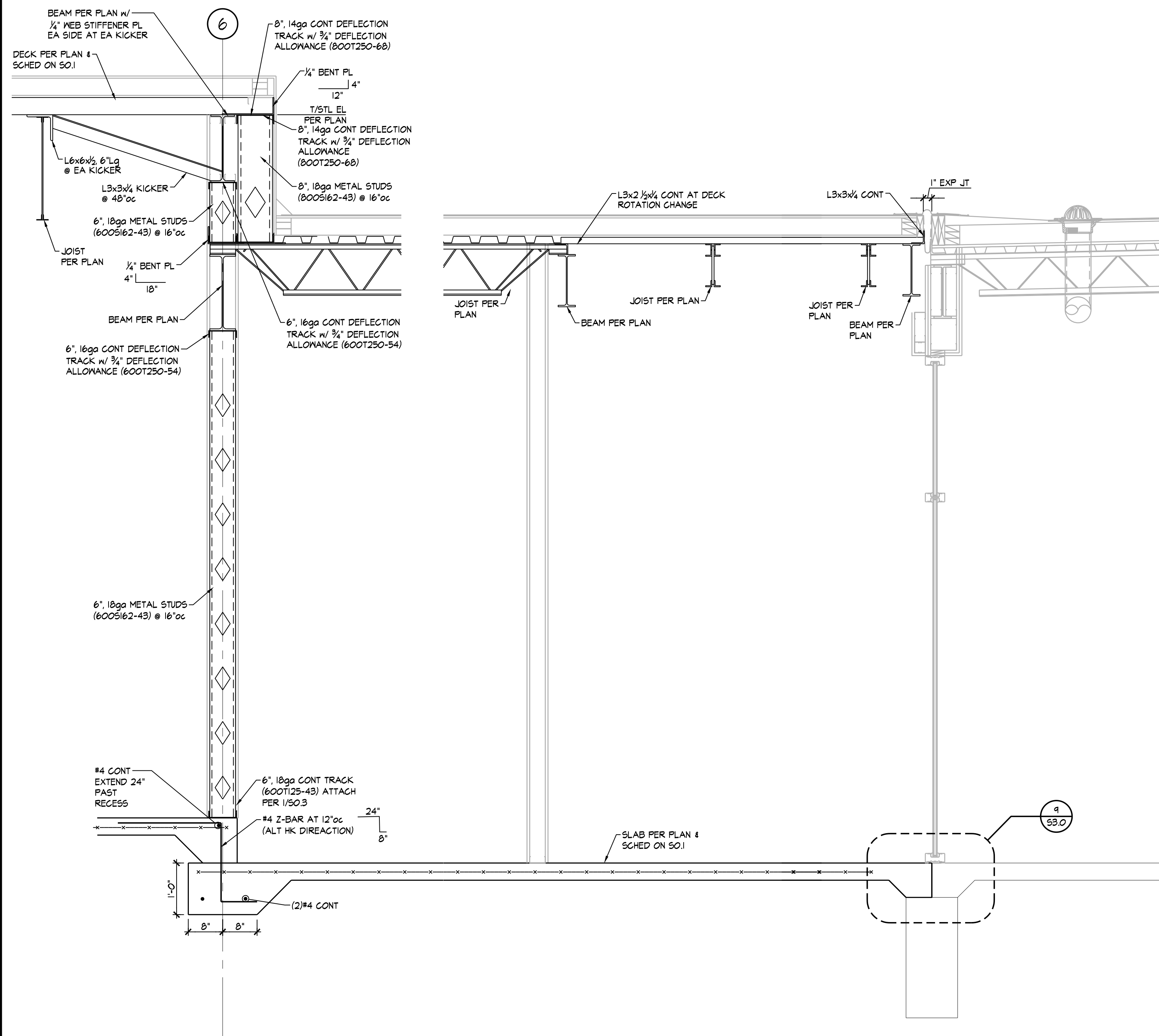
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SECTION 1
 $\frac{3}{4}" = 1'-0"$ 53.9

SECTION 2
3/4" = 1'-0" 53.9

SECTION 3
 $\frac{3}{4}" = 1'-0"$ 53.9



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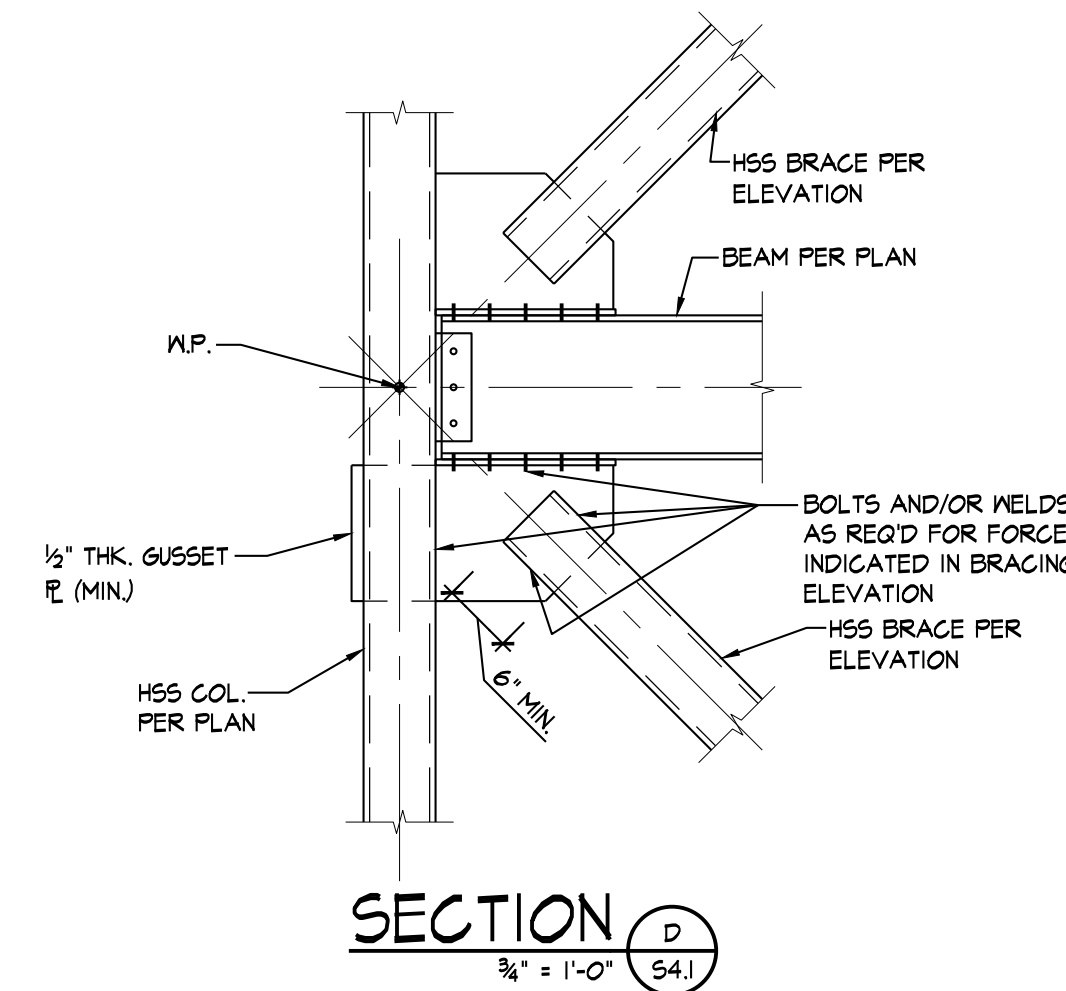
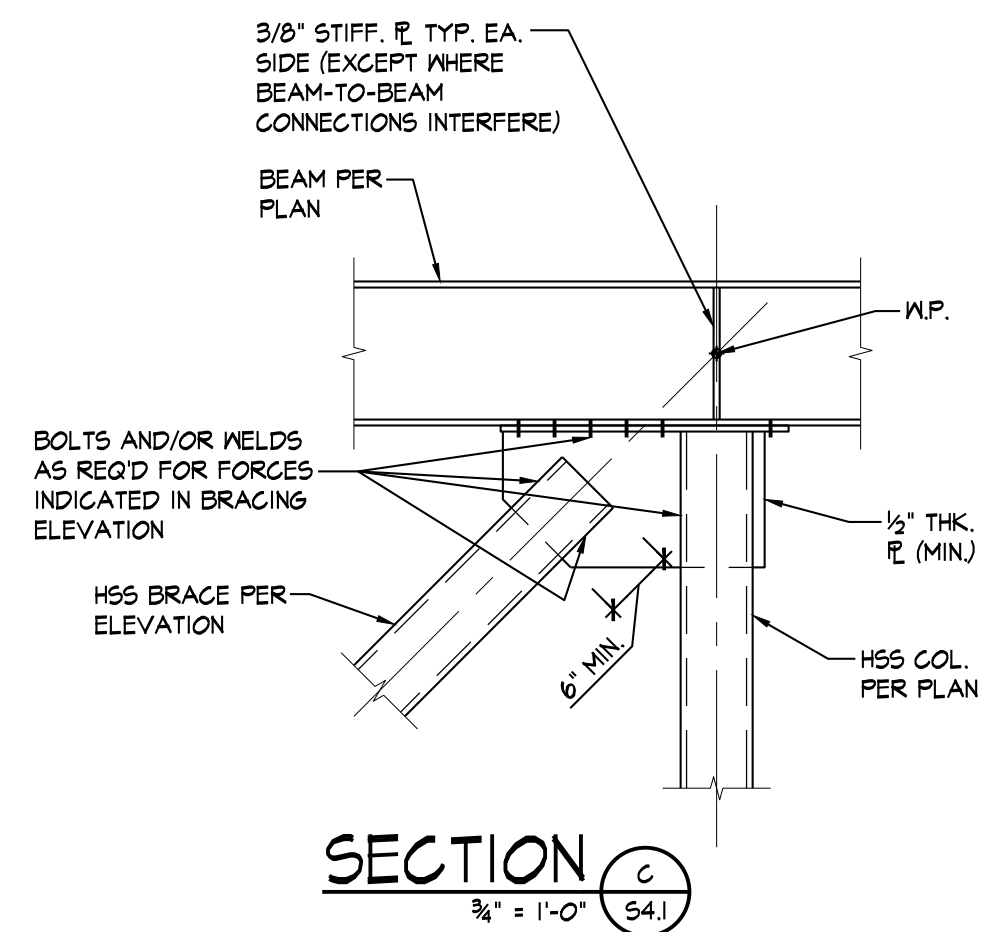
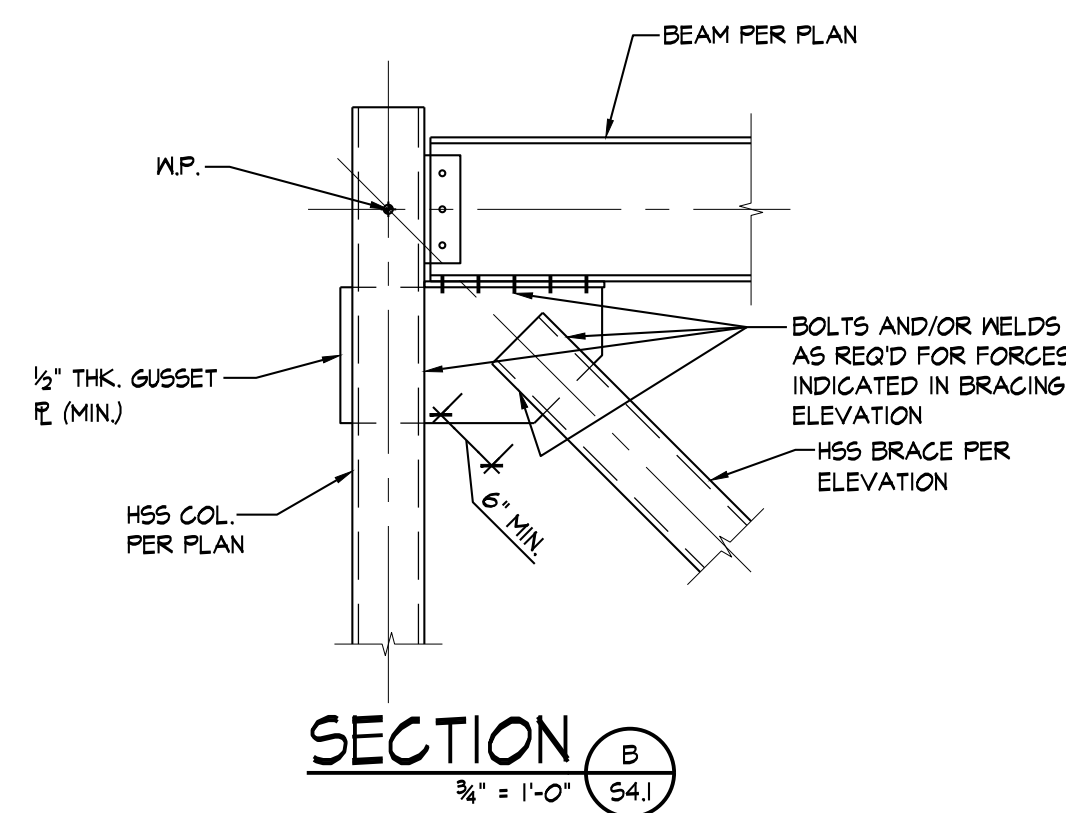
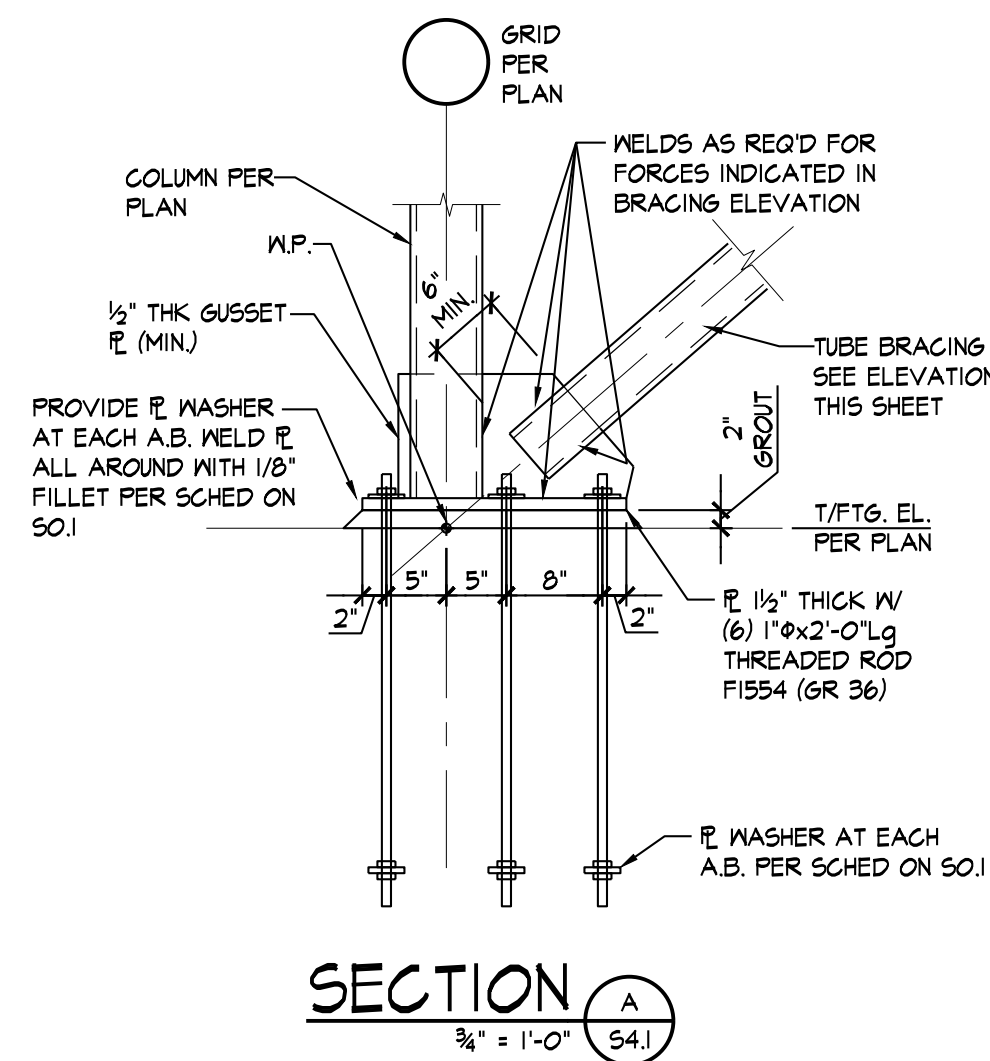
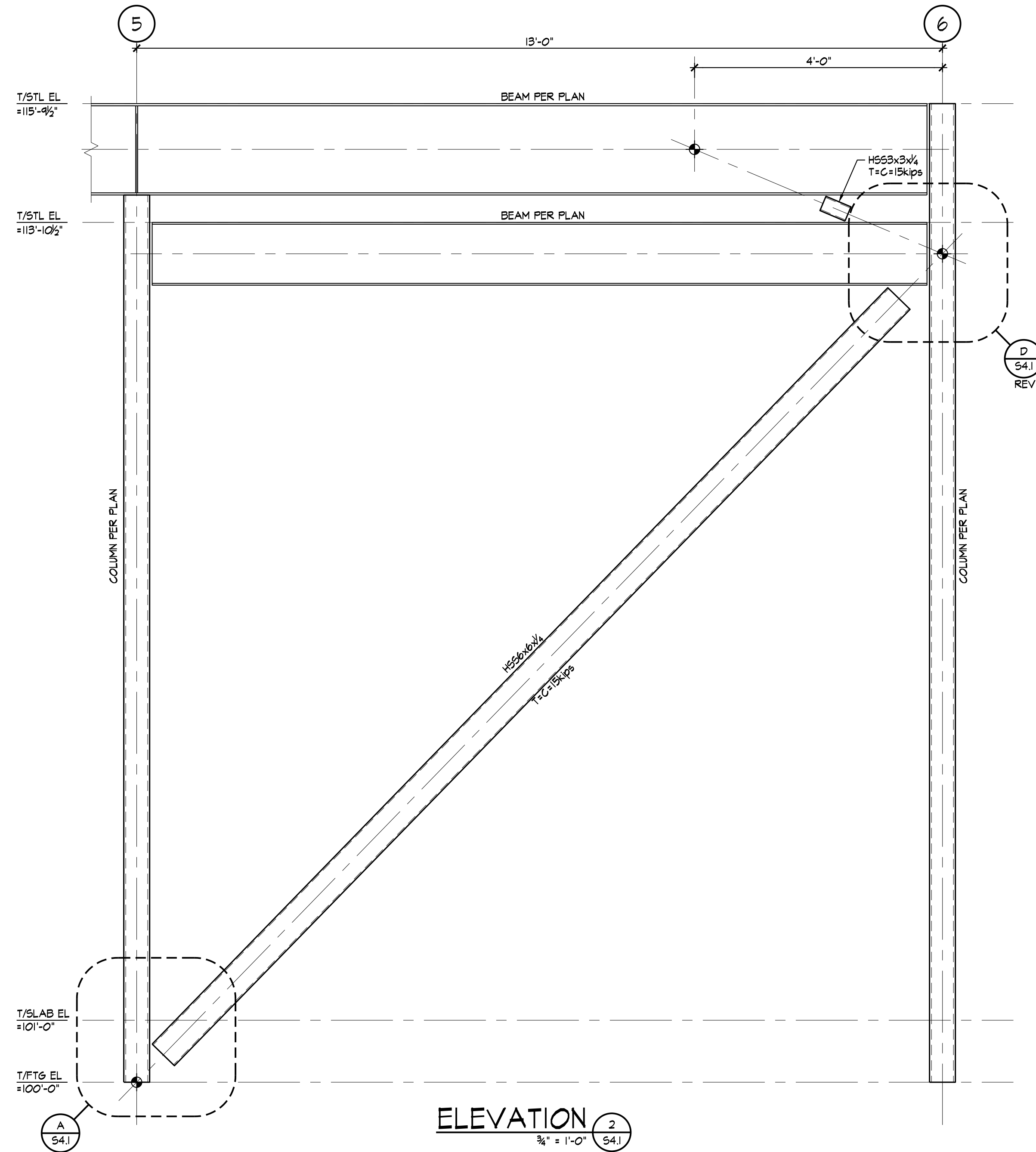
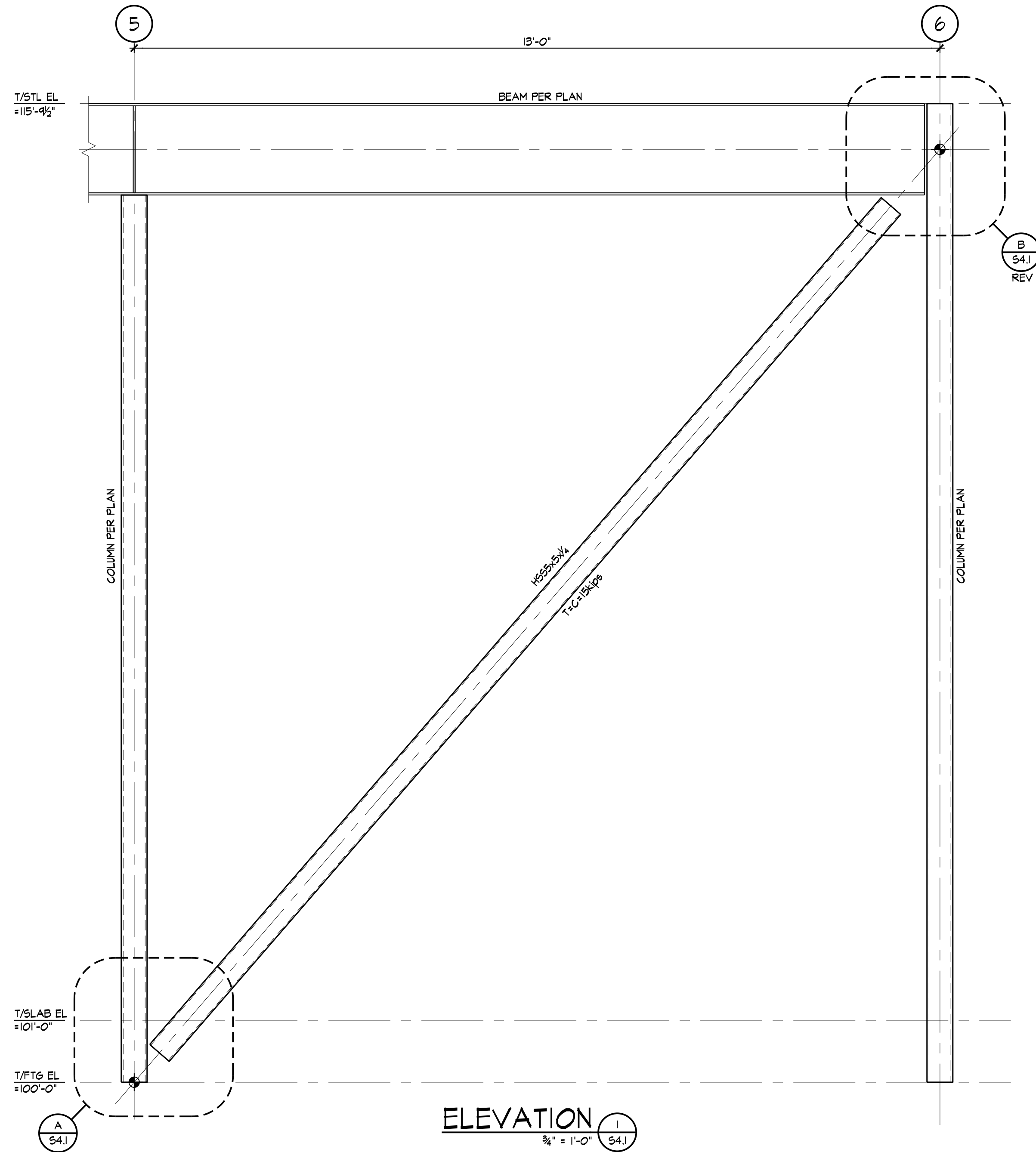
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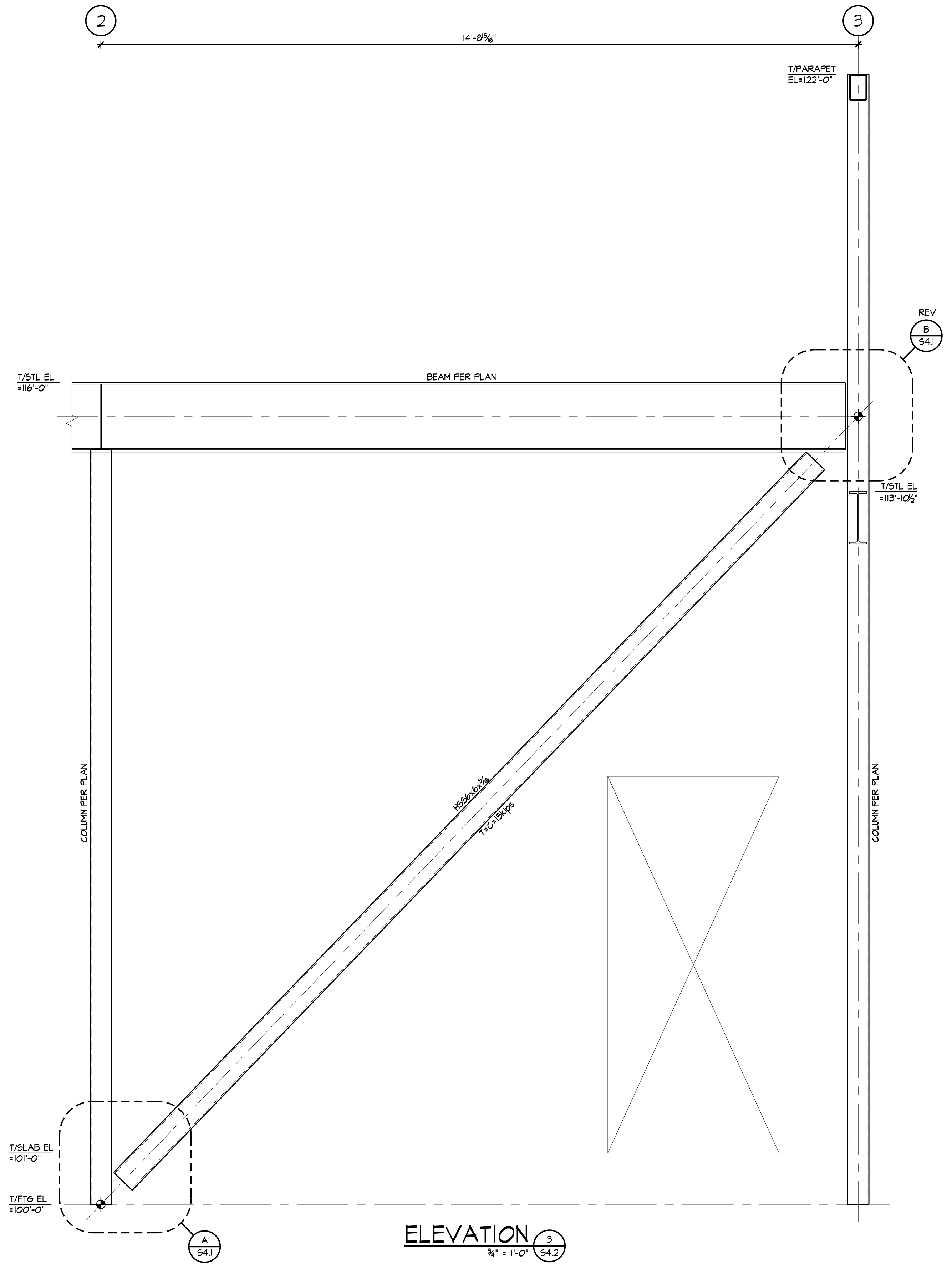
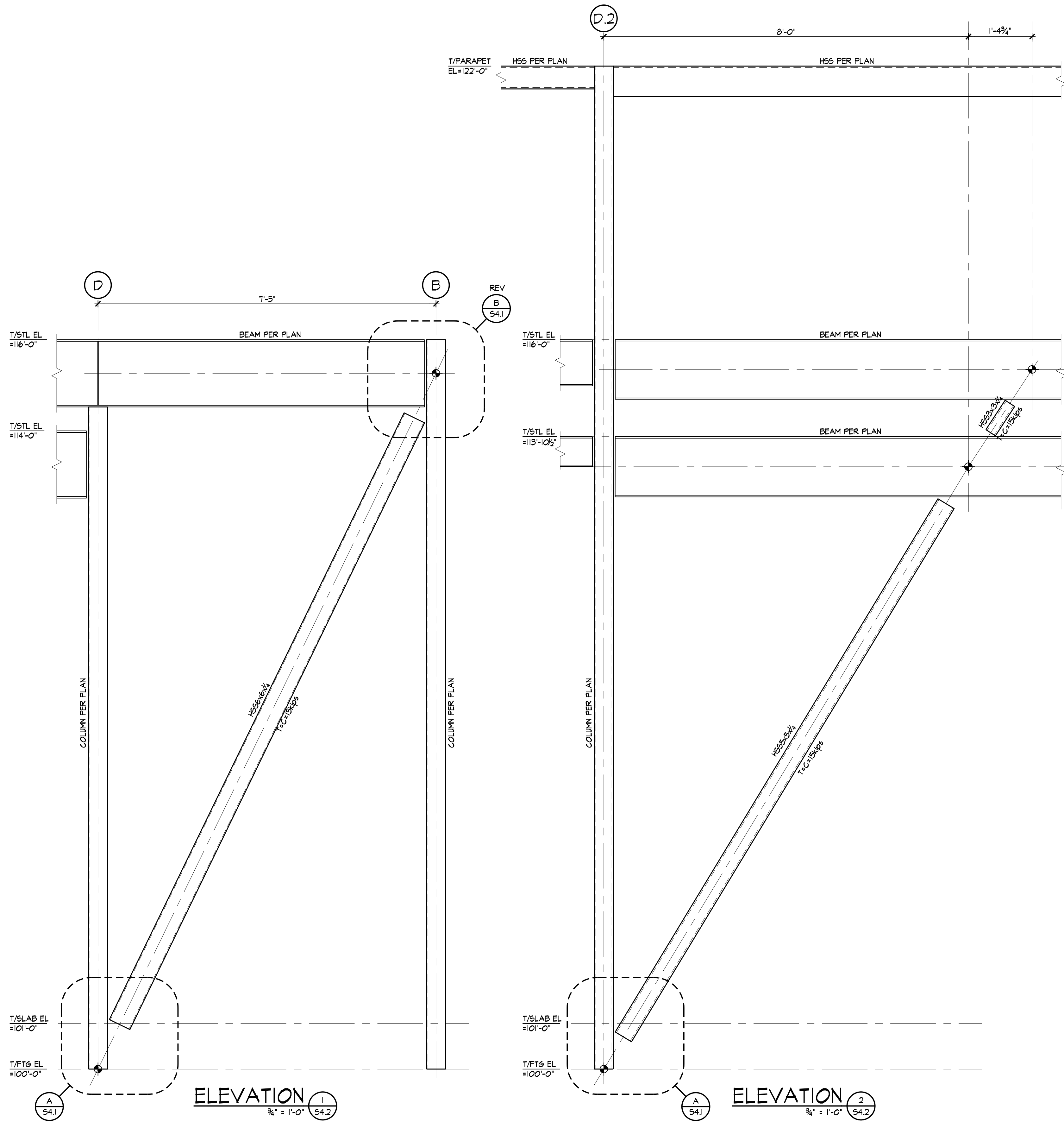
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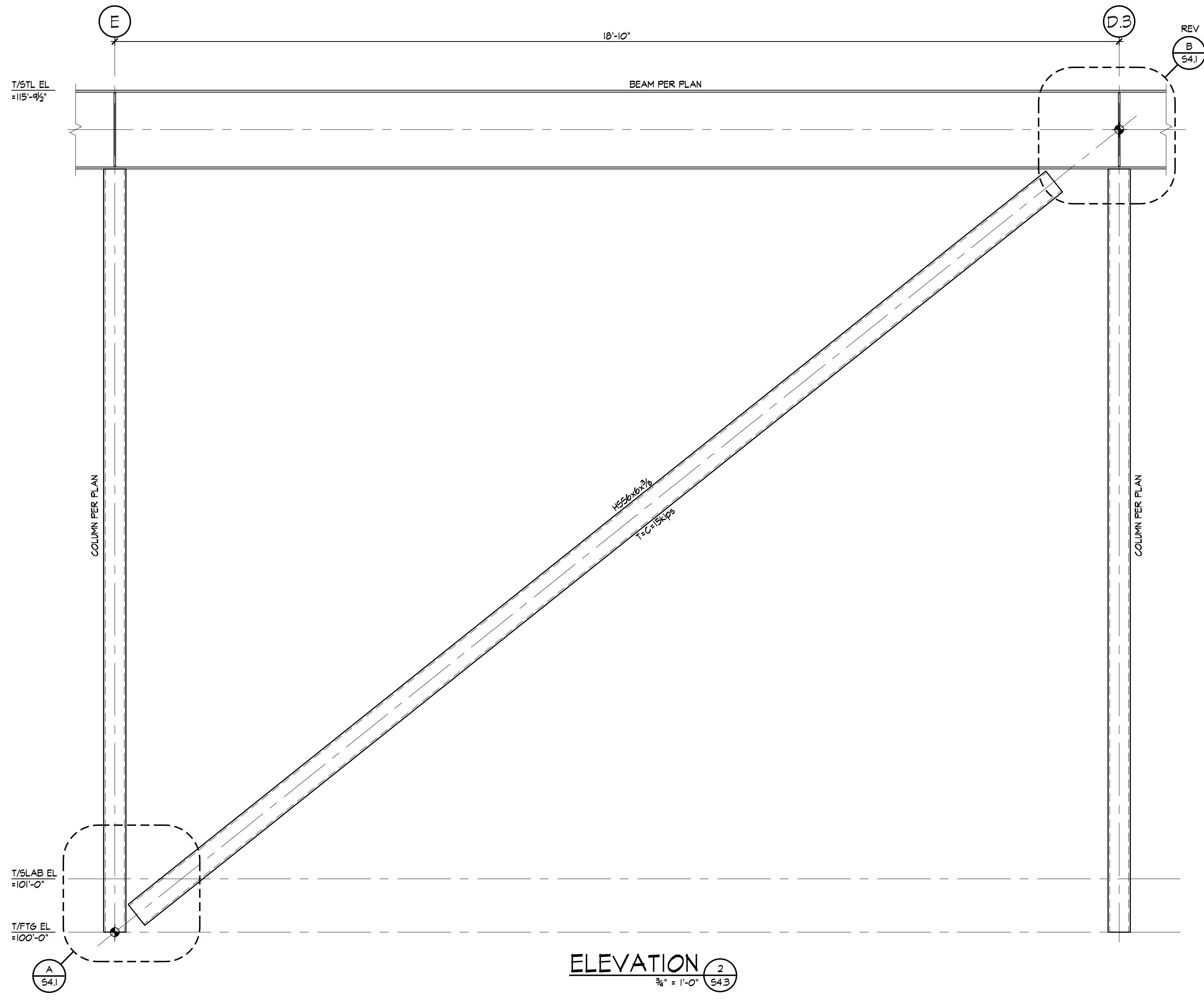
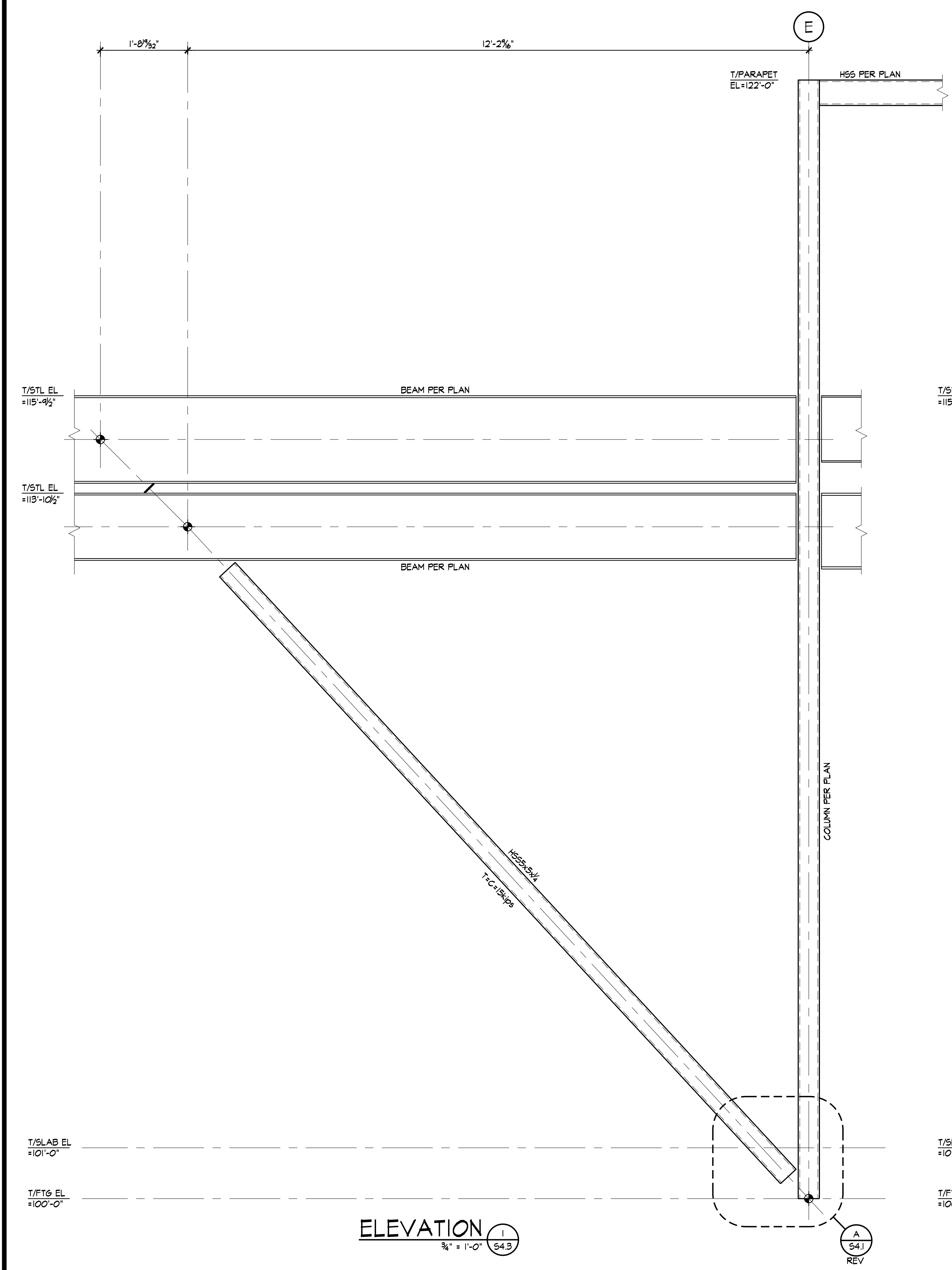
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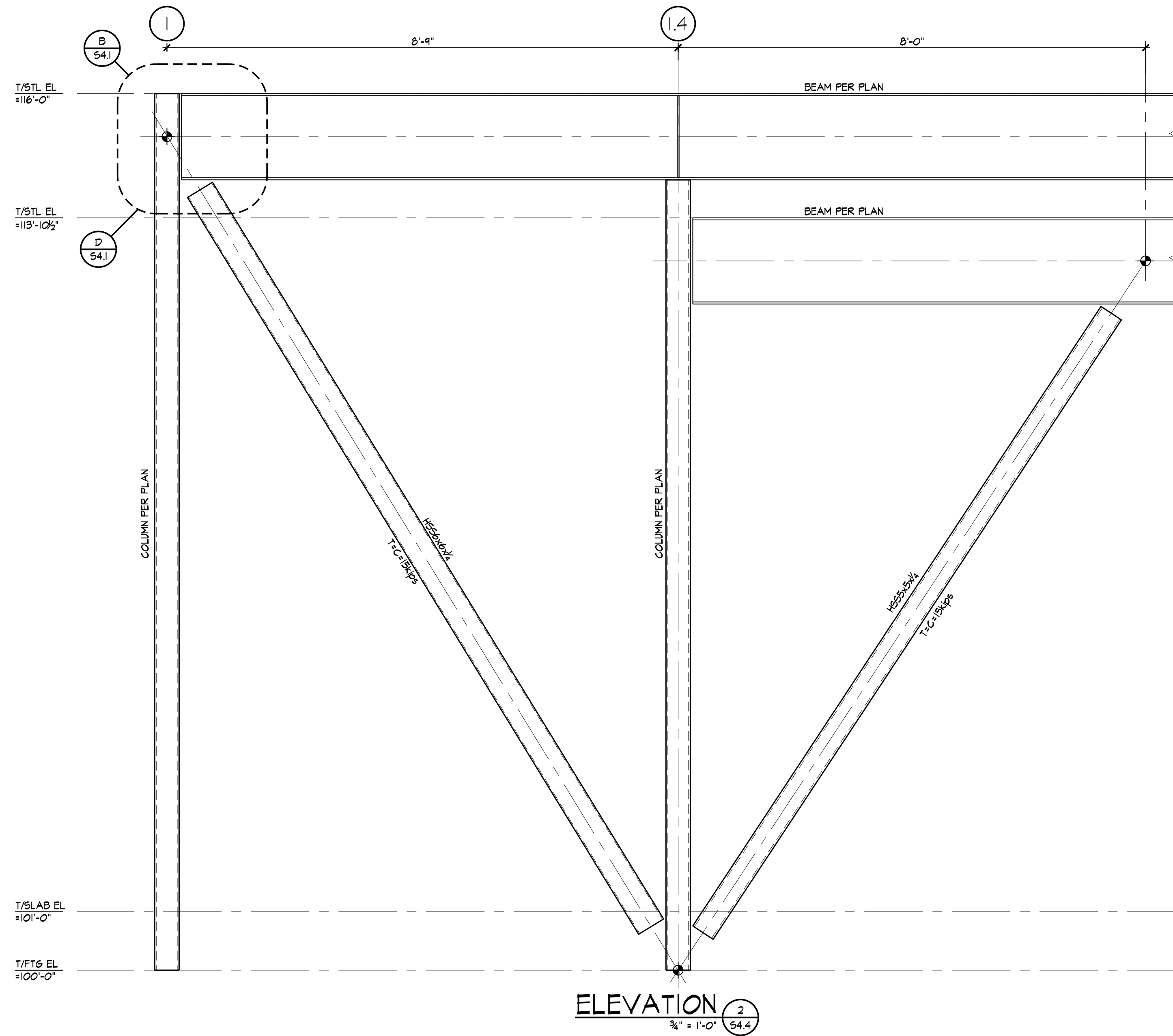
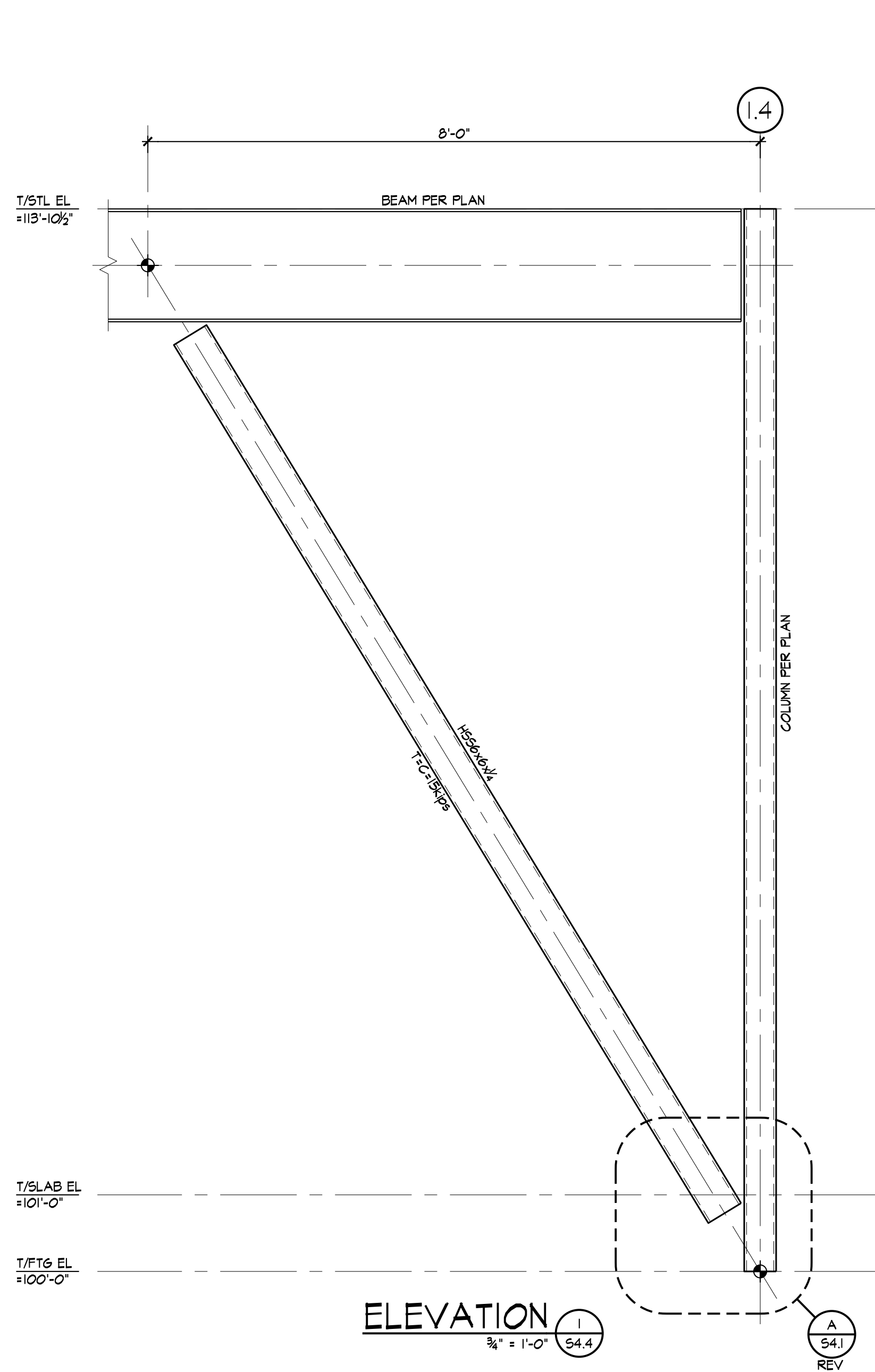
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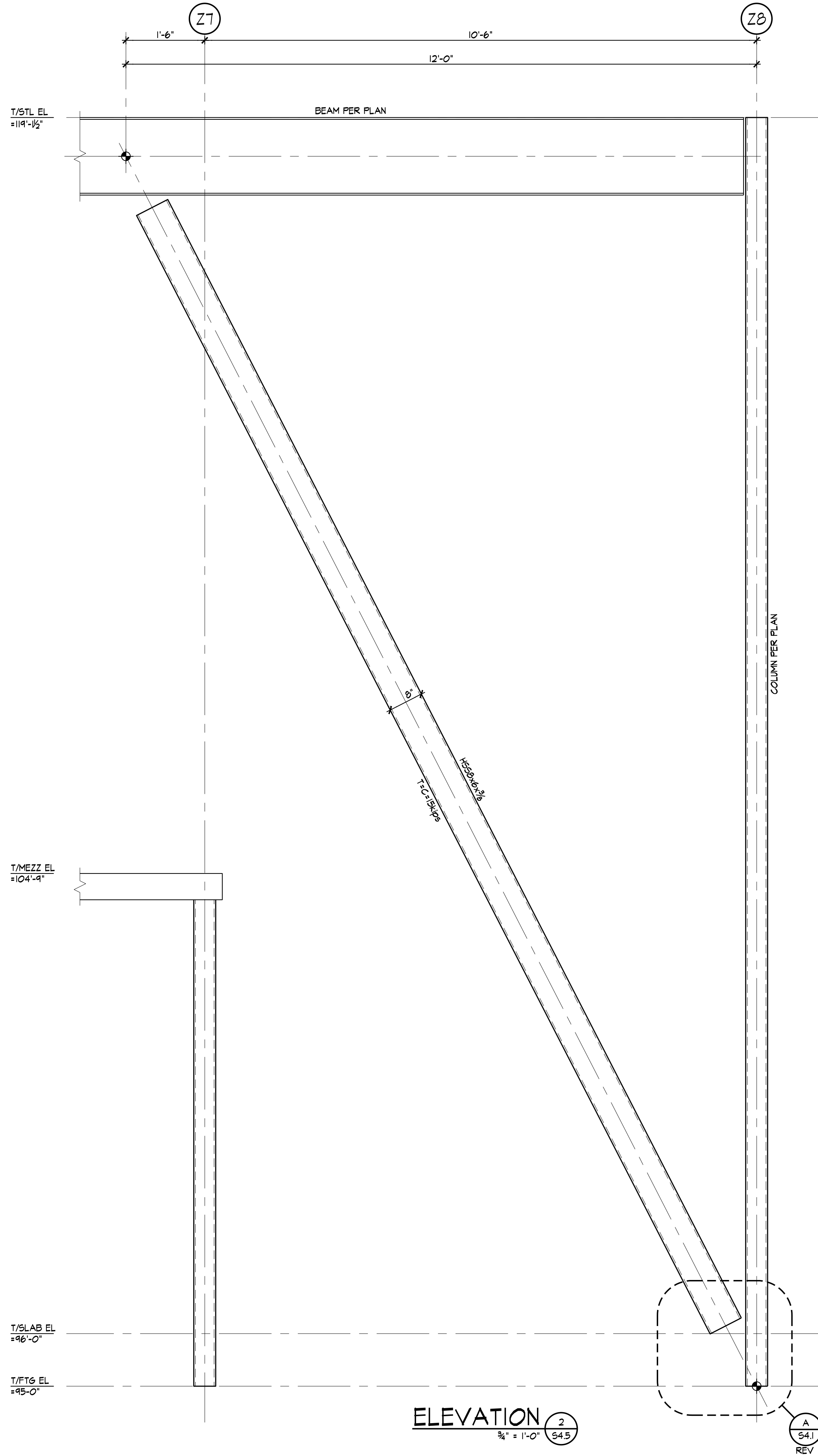
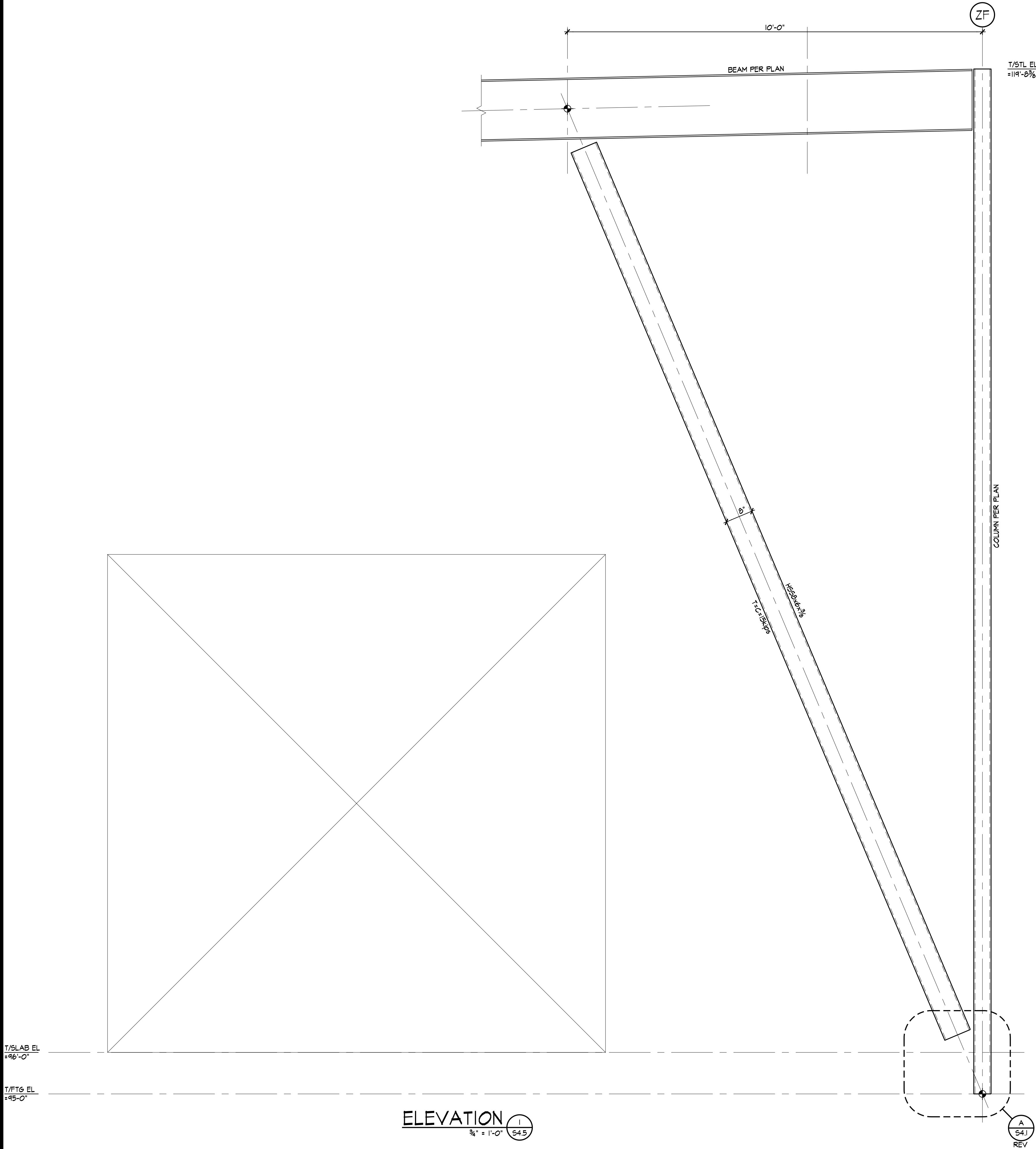
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